



CODE COMPLIANCE CERTIFICATE

SECTION 95, BUILDING ACT 2004

THE OWNER

KCDC Road Reserve
C/- Geoff Brown
P.O. Box 3942
Wellington

No.	100764
Issue date	12/04/12
Application date	20/09/10

THE BUILDING

Street Address	Near building on 86 Emerald Glen Road Paekakariki
Valuation No.	
Legal Description	
Intended Use	Currently there is a stream, after construction there will be a road over stream.
Intended Life	Indefinite, but not less than 50 years
Current Use	Stream/waterway crossing road reserve
Proposal	Retaining walls(over culvert) to support a new road. Road to connect existing Emerald Glen Road to Mackays Crossing roundabout to provide access once water fall road level closes.
Estimated Value	\$200,000

Code Compliance

The building consent authority named below is satisfied, on reasonable grounds, that –

- The building work complies with the building consent; and
- The property owner must follow and carry out the manufacturers prescribed maintenance procedures.

Signed for and on behalf of the Kapiti Coast District Council:

Signature:

Date:

12/4/12

SCOPE OF INSPECTION SHEET

Date 27/3/2012 SR or BCA 100764 Inspection No. 2

Time 11.00 AM Documents on site YES ☒ NO

Owner Name Project Description Culvert

Site Address 86. Enderab. Glen Road Restricted Building Work YES / NO

☒ Building ☐ Plumbing

Circle appropriate Type & Inspection Other

Site Found Piles PreSlab Subfloor Drainage PreWrap PreClad Weathertight PreLine PostLine Block Brick Ret.Wall ☒ Final

Engineer Supervision ☒ Yes No n/a - Name Opus

Notes Culvert Completed.
Fence on western side in
Place.

Diagram (if necessary)

Engineers P.S. & A. (P.S. & A.)
work Policy 002.

24/12

Photo's Taken - Nos. saved / scanned? Y / N

Site instruction issued Engineer. Not C.P. 0.10 P.S. & A.

Confirmation of person on site

Name LBP No.

Result: ☒ Pass

☐ Part pass

☒ Fail

Your next inspection will be
(Guidance notes for inspection on the rear of this sheet)

Agreed, Sign

Officer Name Ian King Sign

Information entered into data base ☐

Initial

Date 27/3/2012

SCOPE OF INSPECTION

Inspections are made in conjunction with, and with the cooperation of, the owner/owner's agent to ensure that the building work complies with the provisions of the Building Act and NZ Building Code. The inspection is general in nature and involves an audit of the following aspects:

Engineer – Civil/Structural/Fire/Geotechnical: An engineer has been engaged to carry out inspections on behalf of the owner. Please ensure those inspections are carried out. A Certificate of Expert Opinion is required to be supplied to Council.

Site: A surveyor's report, or visible boundary markers provided, so that the set out of the project can be clearly identified.

Foundation: When all formwork has been completed, footings have been excavated and reinforcement is in place.

Piles: When all holes are excavated prior to installation.

Pre-slab plumbing (concrete slab): When all plumbing and drainage pipes have been installed, prior to backfilling.

Pre-slab building (concrete slab): When DPM has been placed with all laps and penetrations sealed, reinforcing in place and in position with chairs as appropriate.

Sub-floor (timber suspended floor): When all sub-floor connections, joists and required blocking, and any suspended plumbing pipe work have been completed, but before any flooring or base boards have been fitted. Relocated dwellings require a sub-floor inspection prior to the base boards being fitted.

Drainage: When all drainage works is completed and is under test, before backfilling occurs. As laid drainage plan is required at time of inspection.

Pre-wrap: When all roof and wall framing is complete, including any exterior sheet bracing, but before building wrap has been installed.

Pre-clad: When building wrap and window and door flexible flashing tape has been installed, cavity battens (cavity systems), cavity closers and all flashing systems are in place, but before joinery has been installed.

Weathertightness: When exterior cladding has been installed, flashings in place, air seals fitted to openings, but before any coating system have been applied.

Pre-line (plumbing): When all internal plumbing is complete and plumbing pipe work is under test pressure.

Pre-line (building): When the exterior of the building is weather tight, all structural components (including bracing) have been completed and wall insulation is in place.

Wet area membranes, decks, roofs and shower area: Pre application of wet area membrane, when all outlets and flashings have been installed, ready for membrane installation.

Post application of wet area membrane.

Post-line: When all interior linings have been installed, but before skirting, Scotia or plastering stopping. Installation details of screws check of wall bracing element.

Block: When all masonry block work including block foundations for slab floors, is completed, reinforcing in place and washout openings in place.

Brick Veneer: When veneer is at half height and all flashings are in place.

Retaining wall: Pre application of waterproof membrane system.

Post application of waterproof membrane system.

Protection installed for membrane system and installation of perforated drainage system before back filling.

Final: Final inspections are carried out at the completion of the project and may include entire project, plumbing, drainage and storm water. The Code Compliance Certificate is issued if the building work complies with the approved building consent.

Free Standing Fireplace: Final inspection required only.

In Built Fireplace: Inspection required before installation and after installation.

PRIVACY STATEMENT

1. The information contained on the Building Inspection Report Sheet is held by the Kapiti Coast District Council, whose address is Private Bag 601, Paraparaumu.
2. The information is available for public inspection as part of the Building Record of the property concerned.
3. You have the right under the Privacy Act 1993 to obtain access to, and to request correction of any personal information held by Council concerning you.

Council has a Privacy Officer. If you have any concerns regarding privacy issues please telephone (04) 296 4700

SCOPE OF INSPECTION SHEET

Date 27/3/2012 SR or BCA 100764 Inspection No. 2

Time 11.00 AM Documents on site YES / NO ☒ YES

Owner Name Project Description Culvert

Site Address 86. Emerald Glen Road Restricted Building Work YES / NO

☒ Building ☐ Plumbing

Circle appropriate Type & Inspection Other

Site Found Piles PreSlab Subfloor Drainage PreWrap PreClad Weathertight PreLine PostLine Block Brick Ret.Wall ☒ Final

Engineer Supervision ☒ Yes ☐ No n/a - Name Open

Notes Culvert. Completed. Diagram (if necessary)

Fence on western side in
Place.

Engineers. P.S.4 Approved.
Water. Policy 002.

2/4/12

Photo's Taken - Nos. saved / scanned? Y / N

Site instruction issued Engineers. Note. Change on P.S.4.

Confirmation of person on site

Name LBP No.

Result: ☒ Pass ☐ Part pass ☒ Fail

Your next inspection will be.....
(Guidance notes for inspection on the rear of this sheet)

Agreed..... Sign.....

Officer Name Tom King Sign.....

Information entered into data base ☐ Initial Date 27/3/2012

FINAL INSPECTION

Date 27/3/2012 SR or BCA 100764 Inspection No. 2
Time 11.00 AM Documents on site YES / NO
Owner Name _____ Project Description Culvert
Site Address 86. EDWARDS GLEN ROAD

All inspections are carried out with strict reference to the building consent granted.

P = Pass F = Fail NA = Non-Applicable

Inspection decisions are based on the extent of work inspected at the time of inspection.

Please refer to the related scope sheet for definition of the inspections extent.

Applicable to all inspections

Sighted previous inspection notes	<u>Y</u>	N	NA	Conforms with approved drawings	<u>Y</u>	N	NA	Confirm Additional information/amended plan	Y	N	<u>NA</u>
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FINAL – Exterior

Sub-floor ventilation	P	F	NA	Roof fixings	P	F	NA	Spouting + Down-pipes	P	F	NA
Deck Structure	P	F	NA	Flashings – Roof	P	F	NA	Penetrations – Flashings	P	F	NA
Barriers	<u>P</u>	F	NA	Gully Dishes	P	F	NA	Storm-water + site drainage	P	F	NA
Handrails	P	F	NA	Wastes to gully	P	F	NA	Fixings – Durability	P	F	NA
Steps/Stairs	P	F	NA	Terminal vents/AAV's	P	F	NA	Cladding painted/sealed	P	F	NA
Flashings – Windows/Doors	P	F	NA	Cladding Ground Clearance	P	F	NA		P	F	NA

FINAL – Interior

Service areas – floors/walls	P	F	NA	Food storage/preparation	P	F	NA	Hot water temp	P	F	NA
Stair design	P	F	NA	Ventilation	P	F	NA	Solid fuel heater	P	F	NA
Handrails	P	F	NA	Insulation + clearance	P	F	NA	Water supply tank	P	F	NA
Balustrades	P	F	NA	HWC flues	P	F	NA	Trap seals + AAV's	P	F	NA
Lightings	P	F	NA	HWC valves	P	F	NA	Cistern flushing	P	F	NA
Laundrying	P	F	NA	HWC restraint	P	F	NA	WC pan connection	P	F	NA
Cooking facilities	P	F	NA	Tempering valve	P	F	NA	Septic tank	P	F	NA
Smoke detectors	P	F	NA	Glazing – Human Impact	P	F	NA	Window labels	P	F	NA

FINAL – Intertency/Commercial

Fire/Life systems sighted	P	F	NA	Guarantees/Warranties/Certificates/Expert Opinion	P	F	NA	Intertency-STC	P	F	NA
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All Inspections

Road Crossing Complete	P	F	NA	Energy Certificates supplied	P	F	NA	Final Check Complete	Y	N	NA
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Name: Tony King Officer Sign: _____ Date: 27/3/2012

SCOPE OF INSPECTION SHEET

Date 26/10/10 SR or BCA 100764 Inspection No. _____
 Time 2:35 Documents on site YES / NO _____
 Owner Name OPUS Project Description Culvert
 Site Address 86 EMERALD GLEN RD.

Building ☒ Plumbing ☐

Circle appropriate Type & Inspection

Other _____

Site Found Piles PreSlab Subfloor Drainage PreWrap PreClad Weathertight PreLine PostLine Block Brick Ret.Wall Final

Engineer Supervision Yes ☒ No ☐ n/a - Name _____

Notes _____

Diagram (if necessary)

Base for Culvert

Photo's Taken - Nos. _____ saved / scanned? Y / N

Site instruction issued _____

ENGINEER. PSY FOR BASE FOOTINGS
TO COME.

Result: ☒ Pass ☐ Part pass ☐ Fail

Confirmation of inspection by person on site

Person on site: Builder ☒ Owner ☐ Other ☐ No One on site ☐

Your next inspection will be _____
(Guidance notes for inspection on the rear of this sheet)

Agreed, Wane Holcroft Sign [Signature] Date 26/10/10
(Print Name)

Officer Name ALA Sign [Signature] Date 26/10/10

Additional Insp. Sheet No. _____

Information entered into data base ☐ Initial _____ Date 1/1/11

SITE / FOUNDATIONS / PILES / PRE SLAB / SUBFLOOR / DRAINAGE

Date 26/10/13 SR or BCA 100764 Inspection No. 2-33
 Time 2-33 Documents on site YES / NO
 Owner Name OPUS Project Description Culvert
 Site Address 86 EMERALD GLEN RD.

All inspections are carried out with strict reference to the building consent granted.

P = Pass F = Fail NA = Non-Applicable

Inspection decisions are based on the extent of work inspected at the time of inspection.

Please refer to the related scope sheet for definition of the inspections extent.

SITE

Conforms with approved drawings	Y	N	NA	Checked driveway	Y	N	NA	Engineer supervision required	Y	N	NA
Scope of Resource consent granted?	Y	N	NA	Checked position off relevant boundaries and datum height	Y	N	NA		Y	N	NA
Expert Opinion (Producer statement) required	Y	N	NA	Name of Surveyor Company	Name of Engineer Company						

FOUNDATIONS

PILES

SUBFLOOR

Footing Depth	P	F	NA	Pile size	P	F	NA	Pile/Bearer/Joist Fixings	P	F	NA
Good bearing (NZS 3604)	P	F	NA	Pile layout is to plan	P	F	NA	Notching, Holes, etc	P	F	NA
Reinforcing	P	F	NA	Special piles - point load	P	F	NA	Solid Blocking	P	F	NA
Siting	P	F	NA	Timber Treatment	P	F	NA	Joists	P	F	NA
Sub-floor vents	P	F	NA	Hole depth	P	F	NA	Beams/Bearers	P	F	NA

PRE SLAB

Rebate for brick veneer	P	F	NA	Sand blinding	P	F	NA	Steel reinforcing	P	F	NA
Ground clearance	P	F	NA	Slab thickenings	P	F	NA	Bar chairs	P	F	NA
Concrete Cover	P	F	NA	Vapour barrier (DPC)	P	F	NA	DPC penetrations taped	P	F	NA
Driveway s/w control	P	F	NA	Bottom plate fixings	P	F	NA	Minimum Floor Level	P	F	NA
Secondary flow path	P	F	NA	TPR line	P	F	NA	Wastes and drains - size / fall	P	F	NA
Pipe material	P	F	NA	Plumbing layout	P	F	NA	Test - Water / Air	P	F	NA
Pipe protection	P	F	NA	Pipes supported	P	F	NA	Visual leak check	P	F	NA

DRAINAGE

Pipe diameter	P	F	NA	Connections/Angles	P	F	NA	Holding water	P	F	NA
Bedding material	P	F	NA	Branch length	P	F	NA	Terminal vent	P	F	NA
Gully trap(s)	P	F	NA	Direction/Fall	P	F	NA	LBP/Registration No:			

Officer Name: _____ Sign: _____ Date: ____/____/____

FOR COUNCIL USE:

DEPARTMENT	TICK FOR REFERRAL	COMMENTS	APPROVING OFFICER/ DATE
Site Inspection	✓	Refer to Form 241 for completion of site inspection & sign off this form when completed.	
Resource Consents (refer PIM Sheet)	✓		
Plumbing and Drainage	✓	N/A.	
Building	✓	Approved - Specific design by C. E. G. G.	SAGE 5/10/10
Fire Design			
Compliance Schedule			
Environmental Health			
Other.....			
NOTES:			



Building Consent

Building Project

Location (Site Address)

Building Category

100764
Retaining Walls over culvert
Road Reserve near
86 Emerald Glen Road

Circle appropriate category 0 1 2 3

Application workflow checklist	
Name & Signature	Date
RECEIVED 20 SEP 2010 KAPITI COAST COUNCIL # Note: Checked, date stamped and initialled by TO/BCO. Select and circle appropriate category above & identify allocation to appropriate technical groups and if required to go to "site inspection" AP-WI-001	
Payment taken and receipted (details entered inside this cover sheet) by CSO CS-WI-008	Angie Sikes 20/9/10
Data entered and invoiced by CSO CS-WI-008	Angie Sikes 20/09/10
Pages numbered by CSO CS-WI-008	Angie Sikes 20/9/10
BC Application and documentation scanned, checked & status update by CSO CS-WI-009	Angie 20/9/10
BC Application signed over to BC CS-WI-009	Angie 20/9/10
Application reviewed by Technical Officers who sign off at back of this form AP-WI-026 & AP-WI-009	Angie SAGE 5/10/10
Once Technical review complete, passed to CS for Plan Production and Invoicing AP-WI-009	
CSO checks data entry input, including required inspections CS-WI-012	
BC, plans and other appropriate paperwork produced by CSO CS-WI-012	
BC, Approved plans and appropriate paperwork sent to applicant or agent with receipt by CSO CS-WI-012	
Inspection envelope prepared by CSO with endorsed plans and other appropriate paperwork and filed to cabinet. CS-WI-012	

CONSENT DATA INPUT REQUIREMENTS

REQUIRED INSPECTIONS					
☒	Foundation/Piles	32	☒	Lapse and cancellation after 12mths of consent	23
	Block	2	☒	Engineer design specific components	23A
	Pre-Slab (Plumbing)	3		Foundations to solid bearing of 200mm	25
	Pre-Slab (Building)	4		NZS3604 as acceptable solution in terms of NZ Building Code	30
	Sub-Floor	6		Not approved as Habitable Room	31
	Drainage	9		Note and comply with all endorsements on approved plans	33
	Pre-Wrap	10		Names and Registration of plumber and drain-layer required	34
	Pre-Clad	11		Concealed soil and TV pipes and gas flues only allowed in ducts or minimum framed	38
	Weather-tightness	12		Hot & Cold water pipes tested to 1500kPa	39
	Pre-lining (Building)	13		Roof stormwater to be connected to existing	40
	Pre-lining (Plumbing)	14		Copper water pipes not permitted	43
	Wet areas - membrane	15		Owner/builder to ensure prop foundation height will allow satisfactory connection	44
	Post Lining	21		Minimum FFL to comply with NZS3604	
	Brick Veneer				
	Retaining Wall				
	Final (Dwelling)				
☒	Final (Commercial & Multi-res)				

STANDARD CONSENT CONDITIONS	
37	
72	
80	

NON-STANDARD ADDENDA

N/A

FOR OFFICE USE ONLY

FEES		APPLICATION FEE \$.....	
Building Fee	\$718.00	\$1350.00	
Sewer Connection	\$.....	RECEIPT NO	DATE
Water Connection	\$.....	INVOICE NO	DATE
Vehicle Crossing	\$.....	9737	20-09-10
Damage Deposit	\$.....	RECEIPT NO	DATE
Code Compliance Administration	\$.....	356771	20-09-10
Building Research Levy	\$200.00	FEES PAID BY:	
Development Levy	\$.....	Opus Int Consultants	
DBH Levy	\$394.00	BUILDING CONSENT	
Compliance Schedule	\$38.00	NUMBER	
Digital Storage Fee	\$.....	100764	
Certificate of Title Charge	\$.....	AUTHORISATION FOR REFUND OF DEPOSIT	
Photocopying	\$.....	Deposit held = \$.....	
SUBTOTAL	\$.....	Cost to Repair Damage Caused During Construction = \$.....	
Less Fee Paid	\$.....	Total Balance to be Refunded	
TOTAL	\$1350.00	Authority Number:	
GST INCLUDED	\$.....	Authorised: DATE:	

GST 51-860-608

Received with thanks by 36/01
KAPITI COAST DISTRICT COUNCIL

20-09-10 12:35 Receipt no.356771

COPY *COPY* *COPY*
DR BC100764 OPUS 1,350.00-
Opus::Opus Int : Majestic Centre:100 Wi
CQ B
Opus Int consultants 1,350.00



Kāpiti Coast District Council

Private Bag 60 601
Paraparaumu 5254
New Zealand

Tel (04) 296 4700
or 0800 486 486

Fax (04) 296 4830 ■
kapiti.council@kapiticoast.govt.nz ■
www.kapiticoast.govt.nz ■

Building consent 100764

Section 51, Building Act 2004

The building

Street address of building: Near building on 86 Emerald Glen Road Paekakariki

Legal description of land where building is located:

Valuation number:

Building name:

Location of building within site/block number:

Level/unit number:

The owner

Name of owner: KDC Road Reserve

Contact person:

Mailing address: 175 Rimu Road, Private Bag 60 601, Paraparaumu 5254

Street address/registered office:

Phone number: Landline: 2964700

Mobile:

Daytime:

After hours:

Facsimile number: 042964830

Email address:

Website:

First point of contact for communications with the council/building consent authority:

Full Name: Opus

Mailing Address: Opus Int : Majestic Centre, 100 Willis Street, PO Box 1200, Wellington

Phones:: 04 4717000

Fax: 04 499 3699

Email: richard.coles@opus.co.nz

Building work

The following building work is authorised by this building consent:

Retaining walls(over culvert) to support a new road. Road to connect existing Emerald Glen Road to Mackays Crossing roundabout to provide access once water fall road level closes.

Currently there is a stream, after construction there will be a road over stream.

This building consent is issued under section 51 of the Building Act 2004. This building consent does not relieve the owner of the building (or proposed building) of any duty or responsibility under any other Act relating to or affecting the building (or proposed building).

This building consent also does not permit the construction, alteration, demolition, or removal of the building (or proposed building) if that construction, alteration, demolition, or removal would be in breach of any other Act.

Compliance schedule

A compliance schedule is not required for the building.

Attachments

Copies of the following documents are attached to this building consent:

Signature 


Position

On behalf of: KAPITI COAST DISTRICT COUNCIL

Date: 5/10/10

7 October 2010

KCDC Road Reserve
175 Rimu Road
Private Bag 60 601
Paraparaumu 5254

Dear Sir/Madam

Approval of Building Consent: 100764

Description: Retaining walls(over culvert) to support a new road. Road to connect existing Emerald Glen Road to Mackays Crossing roundabout to provide access once water fall road level closes.
Location: Near building on 86 Emerald Glen Road Paekakariki
Legal Description:

I am pleased to inform you that the above building consent application has been approved. The building consent documents have been issued and are enclosed.

Before starting any work please read ALL the building consent documents carefully and in full so that you as the owner understand the conditions under which the consent has been issued.

As the owner you are also responsible for the following:

- ensuring that building work starts within 12 months of the approval of the building consent and that work progresses at a reasonable pace.
 - ensuring that all required inspections are completed and that a Code Compliance Certificate is obtained once the work has been completed – please be aware that before a Code Compliance Certificate is issued, the building work must comply with the Building Act 2004 and the Building Code as at the date of final inspection.
 - that no work with noisy equipment is to be carried out before 7.30 a.m. or after 6 p.m. Monday to Saturday. During the week there may be workers on site from 6.30 a.m. quietly preparing for work. Work with noisy equipment is not permitted on Sundays and public holidays.
-
- ensuring that no damage occurs to public property, for example footpaths or retaining structures. Should damage occur you will be required to pay for any repairs.
 - ensuring that if you wish to make any changes to the approved building consent, then an amendment to the building consent will need to be lodged with us.

If you have any questions regarding your building consent or your responsibilities, please contact Kapiti Coast District Council on (04) 9045 617.

Addenda to Application

Application

Opus	No.	100764
Opus Int : Majestic Centre 100 Willis Street PO Box 1200		

Project

Description	Retaining Walls Being Stage 1 of an intended 1 Stages Retaining walls(over culvert) to support a new road. Road to connect existing Emerald Glen Road to Mackays Crossing
Intended Life	Indefinite, but not less than 50 years
Intended Use	Currently there is a stream, after construction there will be a road over stream.
Estimated Value	\$200,000
Location	Near building on 86 Emerald Glen Road Paekakariki
Legal Description	
Valuation No.	

Designer: R Coles Opus 100 Willis Street Wellington : 044993699
Contractor: Fletcher c/o Gerald Carty Level 1, 816 Great South Rd
Penrose 1142 : 0276711870

General Requirements

- a) The Building Inspector is to be given 24 hours' notice before carrying out the following inspections:
 1. Foundations
 2. Final Inspection - Commercial
- b) To arrange for any of the above required inspections by Building Control Team, please contact the Call Centre on (04) 296 4700 between the hours of 8am and 5pm.

1. LAPSE AND CANCELLATION OF BUILDING CONSENT. A building consent shall lapse and be of no effect if the building work concerned has not been commenced within 12 calendar months after the date of issue. Or within such further period as the territorial authority allows.
2. The inspection of all specific design components of the building is the responsibility of the Design Engineer who is also required to supply a written report to Council.

I have read the foregoing addenda to application, and the Owner/Builder/Authorised Person hereby agrees to comply with them.

OWNER/BUILDER/
AUTHORISED PERSON

Date: _____

WATERFALL ROAD CONNECTION, KAPITI

DRAWING INDEX

5/2573/1/7504

GENERAL ARRANGEMENT

1. DRAWING INDEX

ROADING

101. PLAN AND LONGITUDINAL SECTION - SHEET 1
102. PLAN AND LONGITUDINAL SECTION - SHEET 2
111. PLAN LAYOUT - Sta. 60 to Sta. 260
112. PLAN LAYOUT - Sta. 260 to Sta. 450
113. PLAN LAYOUT - Sta. 450 to Sta. 640
114. PLAN LAYOUT - Sta. 640 to Sta. 830
115. PLAN LAYOUT - Sta. 830 to Sta. 1020
116. PLAN LAYOUT - Sta. 1020 to End
121. TYPICAL CROSS SECTIONS
122. MISCELLANEOUS DETAILS
131. CLOSURE OF S.H.1 MEDIAN BARRIER BREAK

STORMWATER & DRAINAGE

301. EMERALD GLEN CULVERT - PLAN
302. EMERALD GLEN CULVERT - SECTION & FISH PASS DETAILS
303. EMERALD GLEN CULVERT - TERRAMESH EARTH WALL DETAILS
304. EMERALD GLEN CULVERT - END ELEVATIONS - TERRAMESH WALL DETAILS
305. EMERALD GLEN CULVERT - HEADWALL DETAIL
311. TRAMWAY AND MINOR CULVERT DETAILS
312. MANHOLE AND WINGWALL ARRANGEMENTS
313. SCRUFFY DOME INSTALLATION
321. TYPICAL SWALE DETAIL AND EROSION PROTECTION FOR BRIDGE
322. DIVERSION CHANNEL DETAILS
331. ROAD DRAINAGE DETAILS - SHEET 1
331. ROAD DRAINAGE DETAILS - SHEET 2

LANDSCAPE

401. TYPICAL CROSS SECTION & DESIGN PRINCIPLES
404. PLANTING PLAN 1
405. PLANTING PLAN 2

BRIDGING (DRAFT)

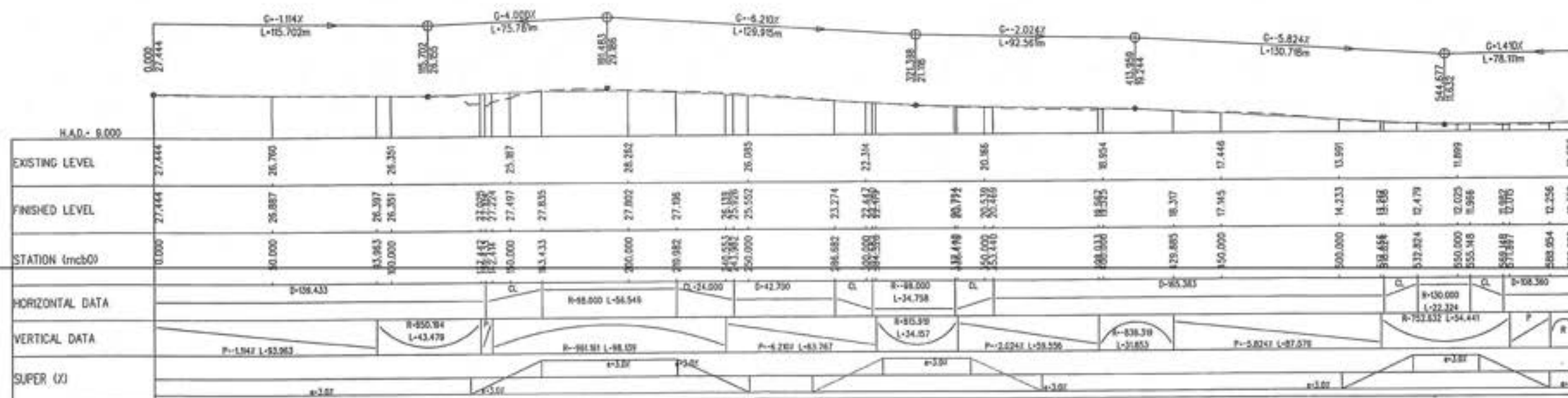
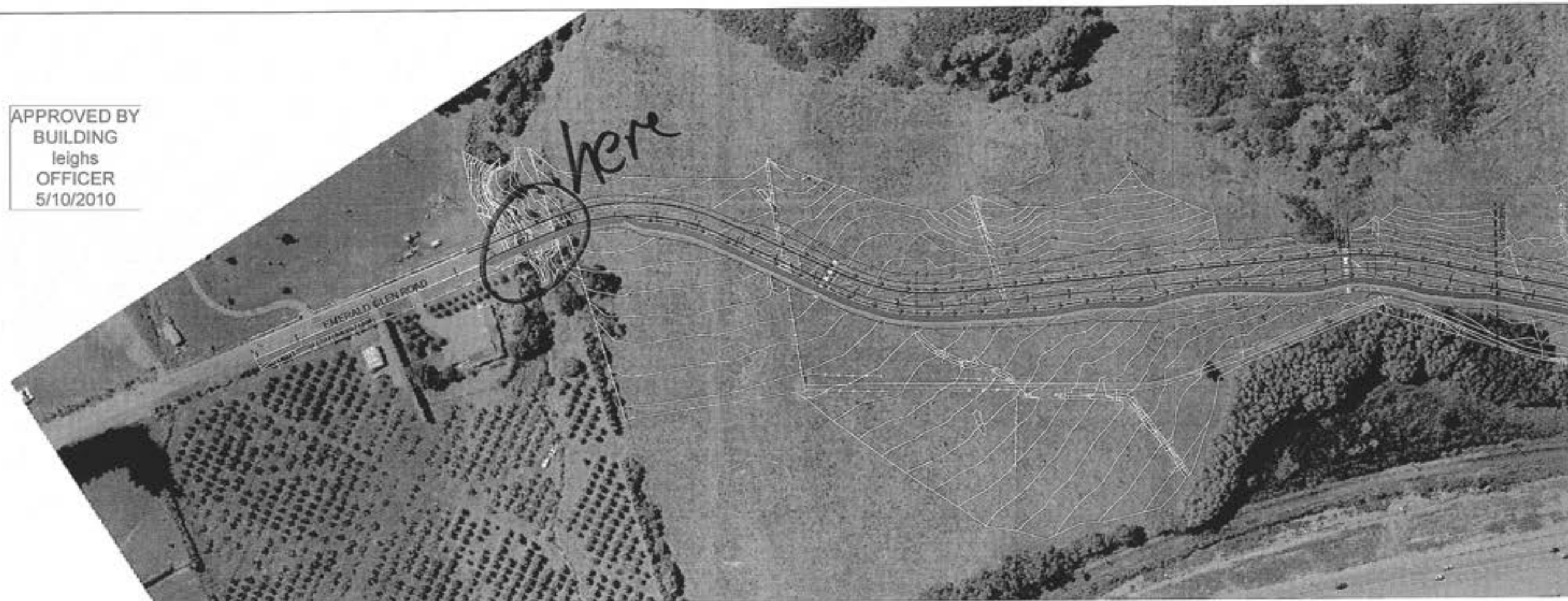
500. GENERAL ARRANGEMENT
501. ABUTMENT GEOMETRY
502. ABUTMENT A REINFORCEMENT
503. ABUTMENT B REINFORCEMENT
504. ABUTMENT & PILE REINFORCEMENT DETAILS
505. 14m SPAN DHC BEAMS - REINFORCEMENT & STRESSING DETAILS
506. 14m SPAN DHC BEAMS - OUTER BEAM UNIT TYPE 2
507. 14m SPAN DHC BEAMS - UNIT DATA & LAYOUT PLAN
508. DECK PLAN & GUARDRAIL LAYOUT
509. W-SECTION GUARDRAIL - ASSEMBLY & FIXING DETAILS
510. ALUMINIUM HANDRAIL - ASSEMBLY & FIXING DETAILS
511. ALUMINIUM HANDRAIL - FABRICATION DETAILS
512. CONCRETE NOTES

APPROVED BY
BUILDING
leighs
OFFICER
5/10/2010





APPROVED BY
BUILDING
leighs
OFFICER
5/10/2010



LONGITUDINAL SECTION
HORIZONTAL SCALE 1:1000
VERTICAL SCALE 1:1000

KEY

- POSSIBLE ARCHAEOLOGICAL SITE
- PROPOSED FOOTPATH LOCATION
- SWALES
- STOCK PROOF FENCE
- PROPOSED STOCK CROSSINGS
- CULVERTS



Rev	Description	Rev	Date
1	ISSUED FOR CONSTRUCTION	1	05/05/2010



Wellington Office
P.O. Box 12-083
Wellington 6142, New Zealand
+64 4 471 7000

Drawn	Checked	Approved	Project
RD	RD		
Sheet No.	Scale	Drawn	Project
5-C1744.01	1:1000 (A1)		

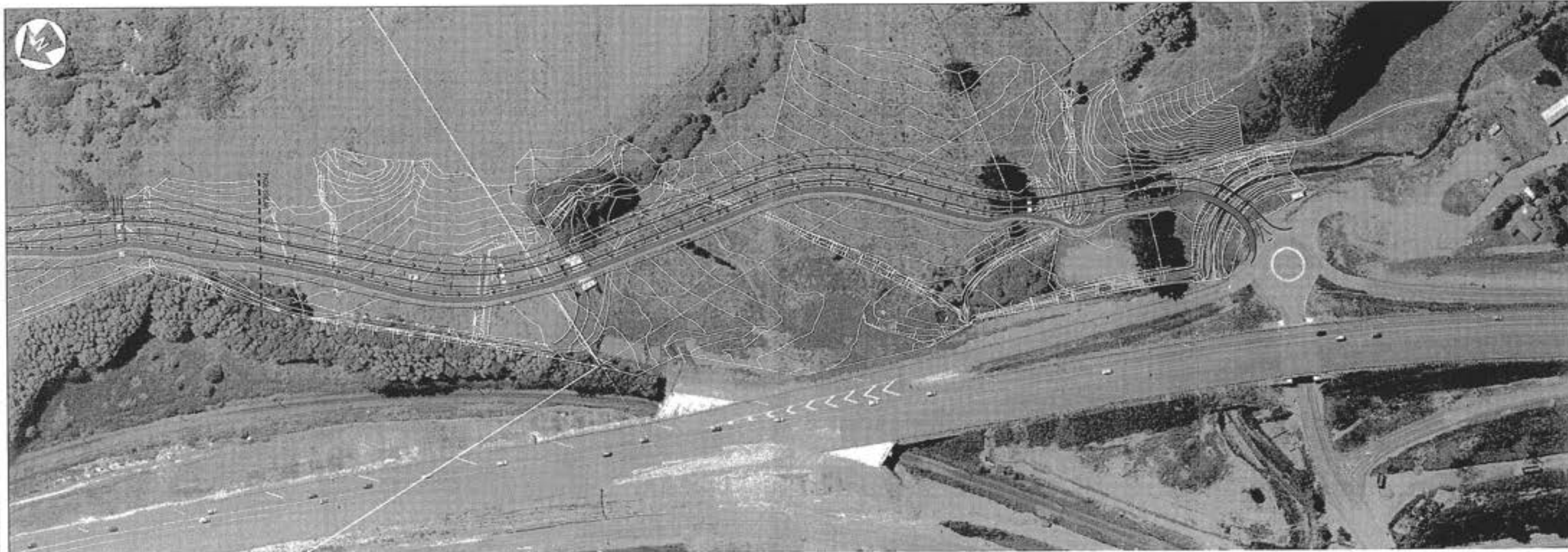
NEW ZEALAND TRANSPORT AGENCY
EMERALD GLEN LINK ROAD, KAPITI
ROAD ALIGNMENT

PRELIMINARY ALIGNMENT - FOR CONSTRUCTION
SHEET 1 OF 2

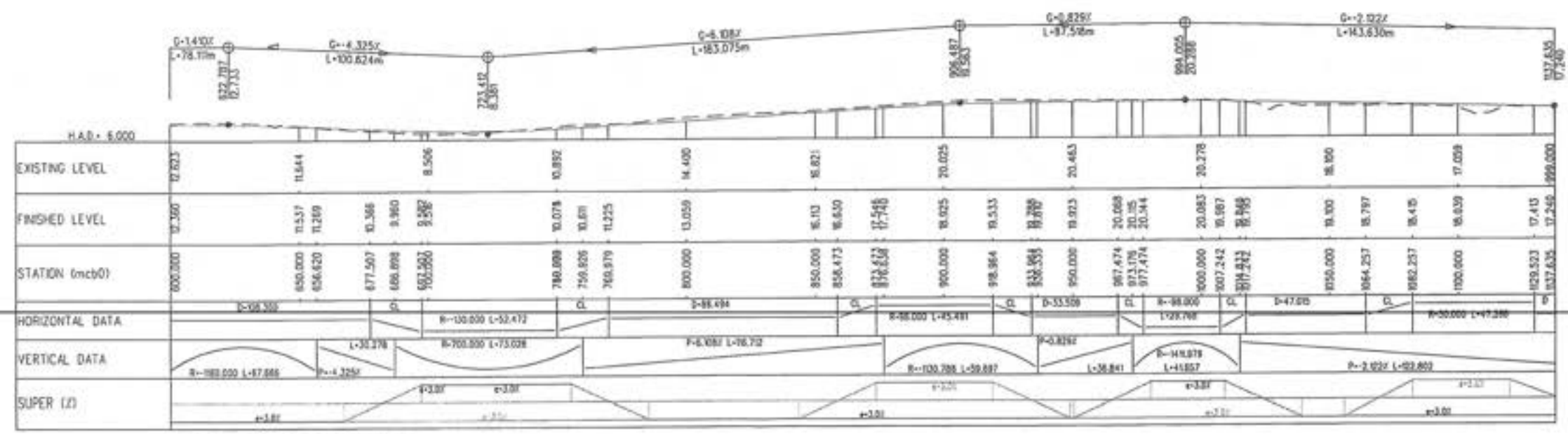
5 / 2573 / 1 / 7504

101 R2

CONSTRUCTION



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OFFICER
5/10/2010



LONGITUDINAL SECTION
HORIZONTAL SCALE 1:1000
VERTICAL SCALE 1:1000

- KEY**
- POSSIBLE ARCHAEOLOGICAL SITE
 - PROPOSED FOOTPATH LOCATION
 - SWALES
 - STOCK PROOF FENCE
 - PROPOSED STOCK CROSSINGS
 - CULVERTS



NO.	ADDRESS	DATE	REVISION
01	ISSUED FOR CONSTRUCTION	CL	00000010



Wellington Office P.O. Box 10-000 Wellington 6144, New Zealand +61 4 471 7000		NEW ZEALAND TRANSPORT AGENCY EMERALD GLEN LINK ROAD, KAPITI ROAD ALIGNMENT	
5-C1744.01		1:1000 (A1)	
5 / 2573 / 1 / 7504		102 R2	

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STOCK PROOF
FENCE TO TIE INTO
EXISTING FENCE
LINE

DIVERSION CHANNEL 1A
DETAILS SHEET 303

ELECTRA DISTRIBUTION
POWER POLE (49381A)

TL - 3 W-BEAM SAFETY
BARRIER EXTENDED
ACROSS CULVERT TO
PROTECT POWER POLE

ELECTRA DISTRIBUTION
POWER POLE LOCATED
NEAR TO CUT SLOPE
(49381A)

CONTRACTOR TO BACKFILL
BEHIND CULVERT HEADWALL
WITH 150mm OF TOPSOIL AND
HYDROSEED

WW.M.1

PW-36

PW-37

SUMP 1

SUMP 2

RIP-RAP SCOUR
PROTECTION AS
SHOWN ON SHEET 302

TIMBER POST
AND RAIL FENCE

NORTHERN CULVERT
FOR DETAILS OF NORTHERN
CULVERT (INC. WING WALL,
TERRAMESH, RIPRAP AND
EARTH WALLS) SEE SHEETS
301 TO 305

SWALE TYPE 3

SWALE TYPE 2

3.0m LONG CONCRETE
CHANNEL (150mm THICK)
TO TRANSITION FROM
K&C TO SWALE

SOLID 100mm WHITE
PAINTED CENTRELINE
AROUND CURVE

TRAFFIC CALMING MEASURES FOR
EXISTING EMERALD GLEN ROAD
STILL TO BE AGREED WITH KDC

- NOTES:
1. CONTOURS ARE SHOWN AT 1m SPACING
 2. ALL BARRIER TERMINAL ENDS ARE TO BE STANDARD LEADING AND TRAILING EXCEPT WHERE SHOWN OTHERWISE
 3. EACH SWALE END WILL HAVE 3.0m CHANNEL (150mm THICK) TRANSITIONS FROM/TO KERB AND CHANNEL OR TO DISCHARGE
- SERVICES KEY
- TRANSPower 110KV OH LINES
 - VECTOR UNDERGROUND TRANSMISSION GAS PIPELINES
 - ELECTRA DISTRIBUTION OH LINES
 - TELECOM UNDERGROUND FIBRE-OPTIC LINES

- LEGEND
- MINOR DRAINAGE CHANNEL
 - MINOR STREAM
 - SIGNIFICANT STREAM
 - DIVERSION CHANNEL
 - SWALES
 - FOOTPATH / BRIDLEWAY
 - GRASSED BRIDLEWAY
 - PROPOSED ROAD ALIGNMENT
 - EXISTING ROAD SIGNS & LIGHTING
 - NEW ROAD SIGNS & LIGHTING
 - TIMBER POST & RAIL FENCE
 - STOCK PROOF FENCING
 - STOCK CROSSING
 - SIGHT RAILS
 - MANHOLE
 - SCRUFFY DOME MH
 - WINGWALL
 - CULVERT

NO	ISSUED FOR CONSTRUCTION	C.L.	03/09/2010



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NEW ZEALAND TRANSPORT AGENCY
EMERALD GLEN CONNECTION, KAPITI
ROAD ALIGNMENT

PLAN LAYOUT
Sta 60 to Sta 260

Rev	Desc	Rev	Desc
R.G.	R.G.	C.L.B.	02/09/2010
SC1744.01		1:250(A1), 1:500(A3)	

5/2573/1/7504

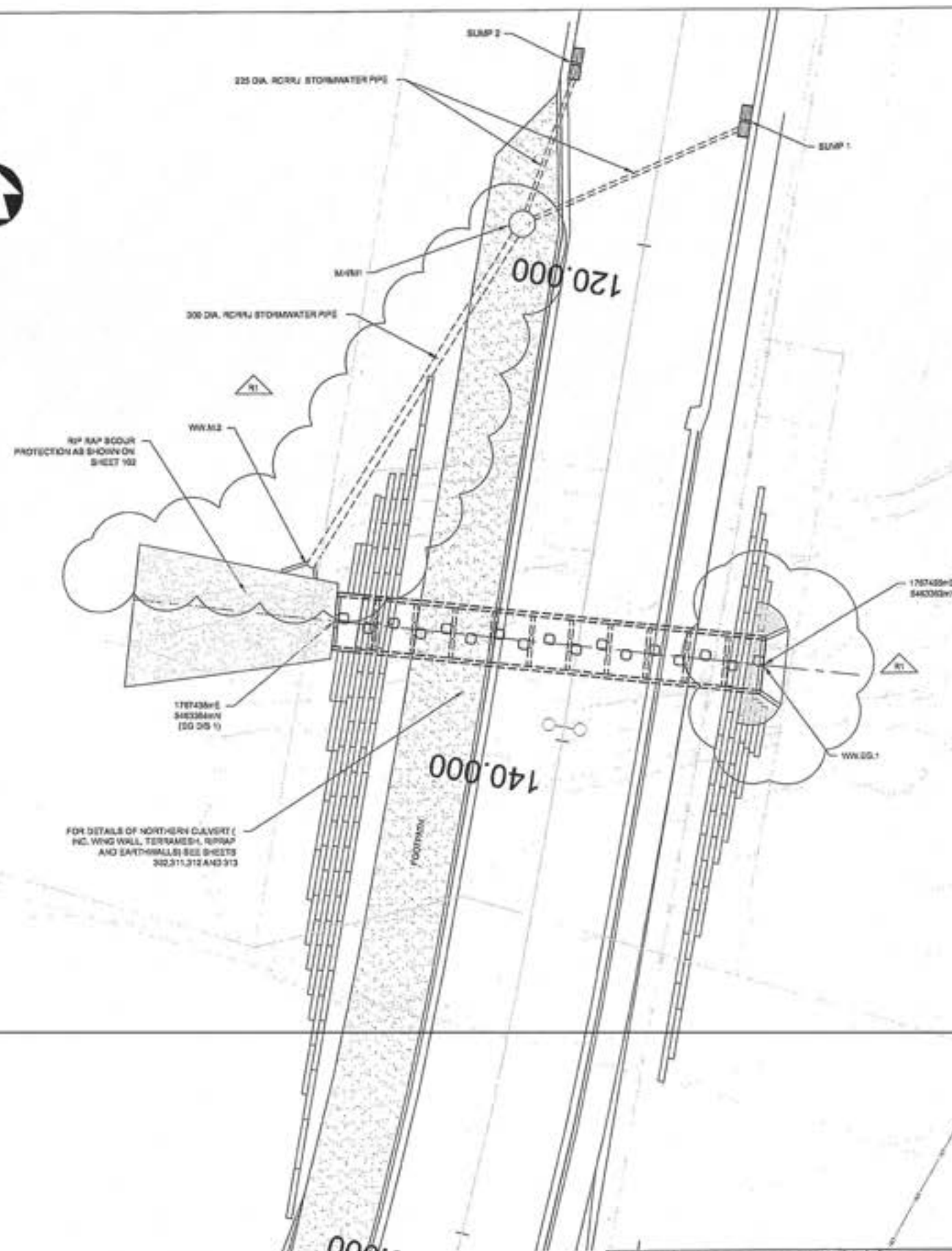
111 R2



CONSTRUCTION



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BUILDING
OFFICER
5/10/2010



CONSTRUCTION

Revised	Comments	By	Date
R0	ISSUED FOR RFP	C.L.	20/01/10
R1	ISSUED FOR CONSTRUCTION	C.L.	03/01/10



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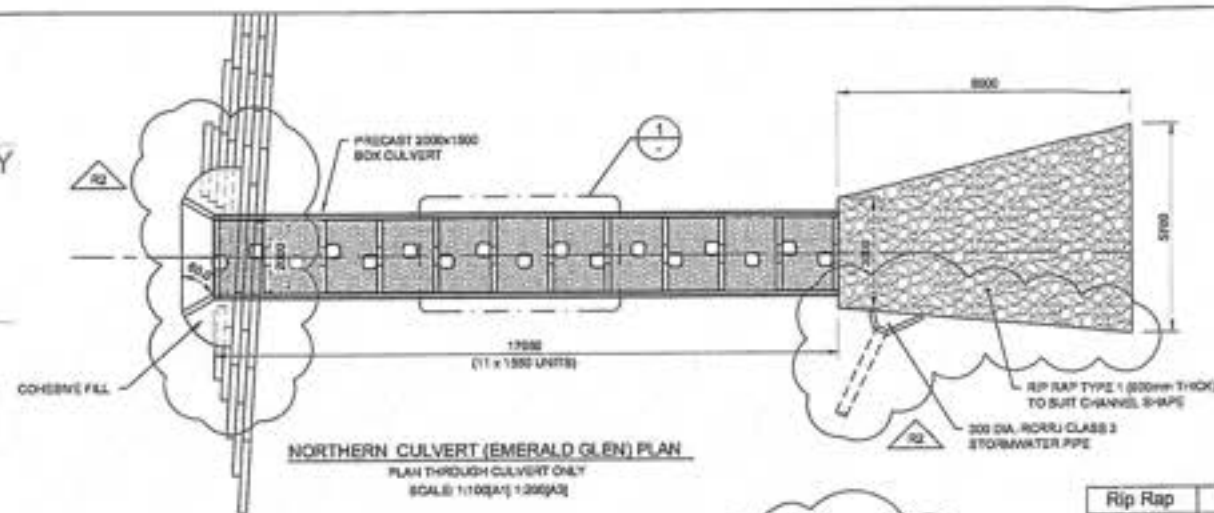
NZ Transport Agency
Emerald Glen Connection Road, Kapiti
Road Alignment

Culverts - Emerald Glen Culvert
Plan Layout

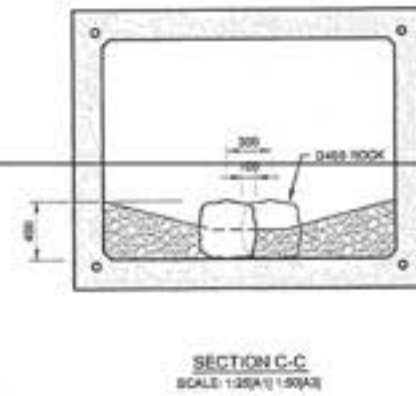
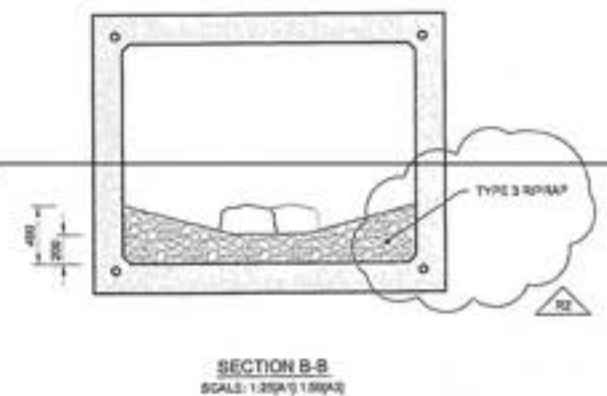
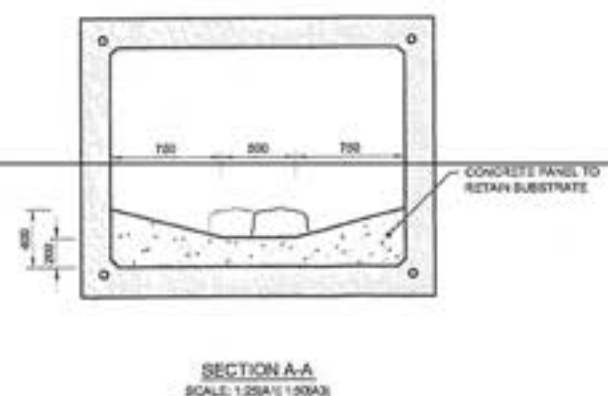
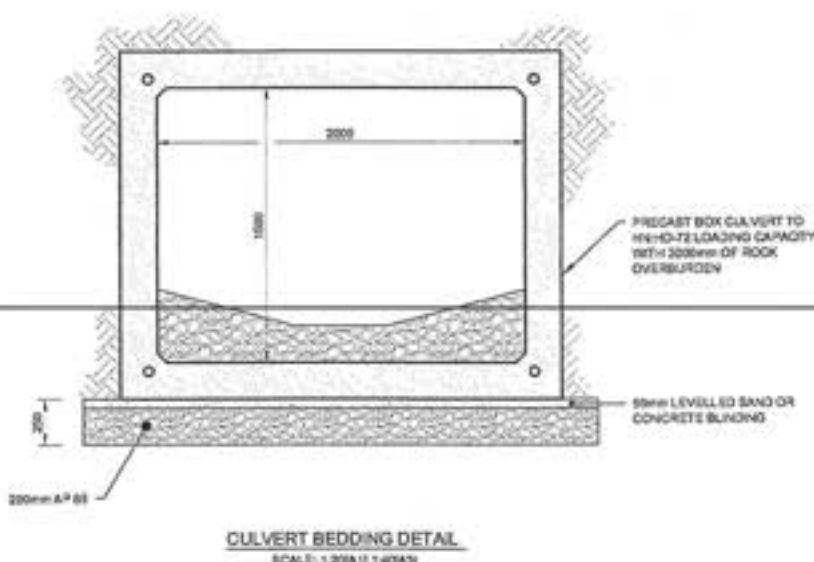
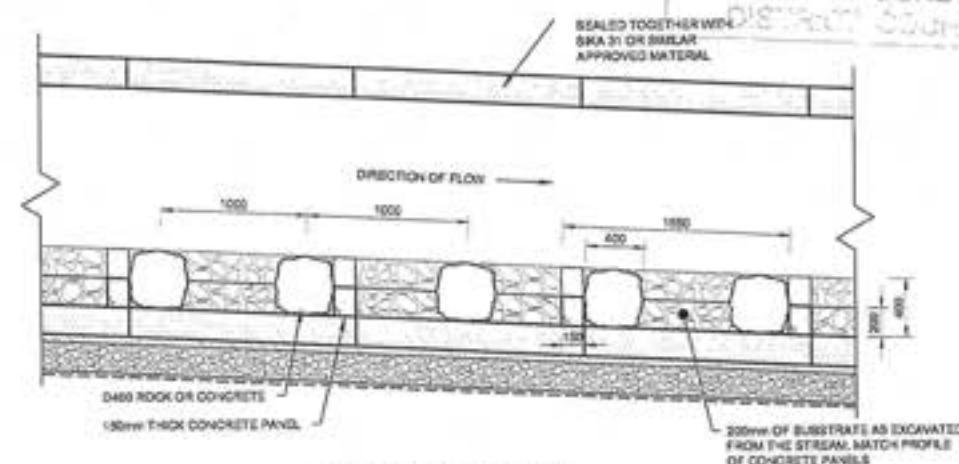
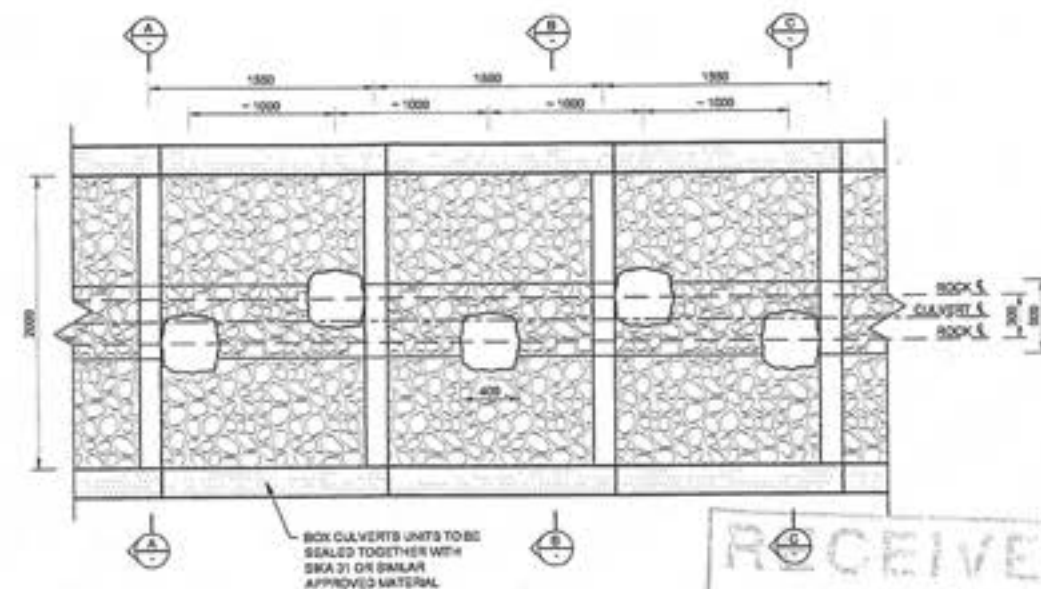
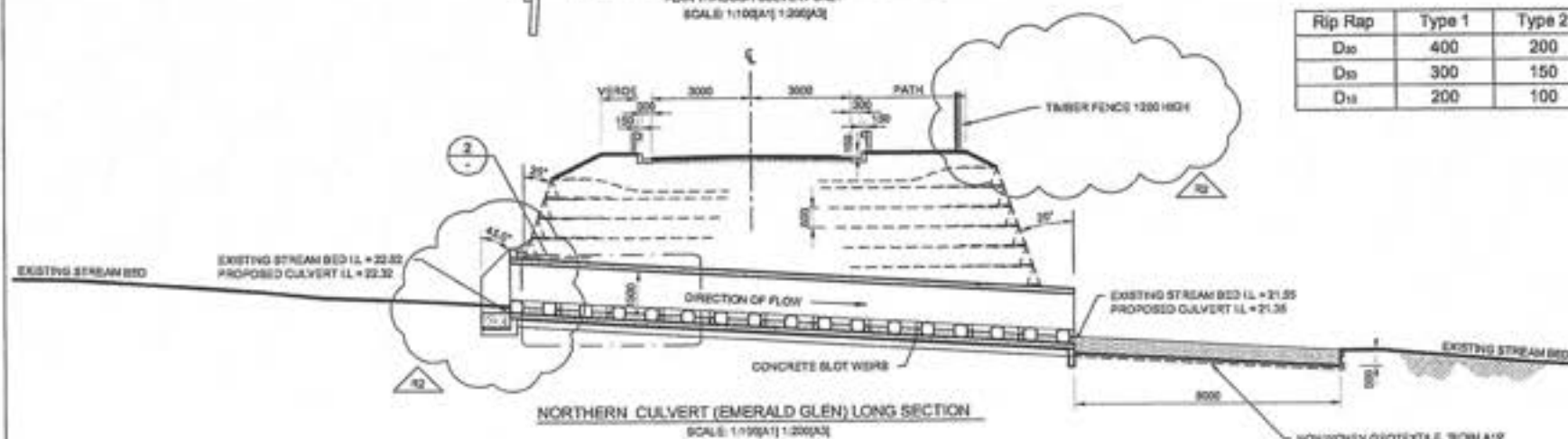
Drawn	Checked	Approved	Revision
Karl Winger	R. Goss	C. L. A.	0001 2010
5-C1744.01	1:100(A1), 1:200(A3)	5 / 2573 / 1 / 7504	301 R1

Sheet No.	301	Rev.	R1
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OFFICER
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Rip Rap	Type 1	Type 2
D ₅₀	400	200
D ₂₅	300	150
D ₁₀	200	100



REFERENCE DRAWINGS:
S2573/1/7504/001 TO 300 CULVERTS-EMERALD GLEN
S2573/1/7504/011 TO 313 CULVERTS-EMERALD GLEN
S2573/1/7504/031 AND 302 OPEN CHANNELS
S2573/1/7504/031 ROAD DRAINAGE

Rev	Description	By	Check	Date
01	ISSUED FOR CONSENT			
02	ISSUED FOR MFD	C.L.		18/07/10
03	ISSUED FOR CONSTRUCTION	C.L.		05/07/10

NZ TRANSPORT AGENCY
WAKANUI

OPUS
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Wellington 6144, New Zealand
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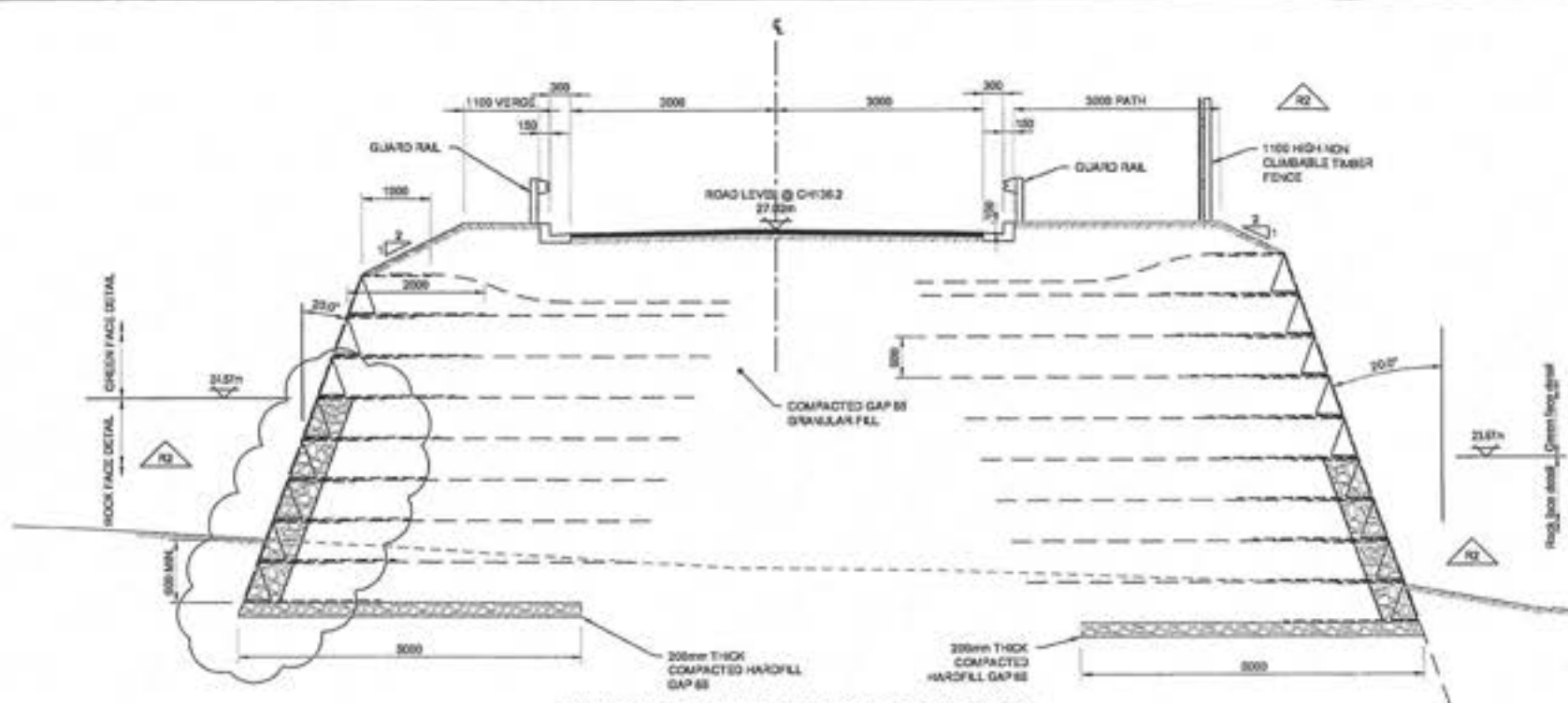
NZ Transport Agency
Emerald Glen Connect Road, Kapiti
Road Alignment
Culvert Details - Emerald Glen Culvert
Culvert Section and Fish Pass Details

Drawn	Checked	Approved	Revised
H. Clarke	R. Davis	C. Lee	SEPT 2010
Project No.	5-C1744.01	As Shown	

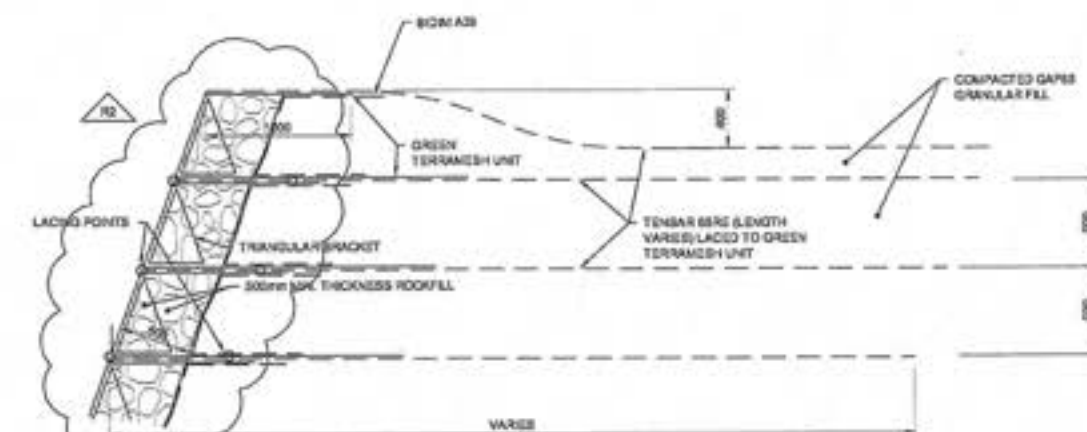
Drawn No. 5 / 2573 / 1 / 7504

302 R2

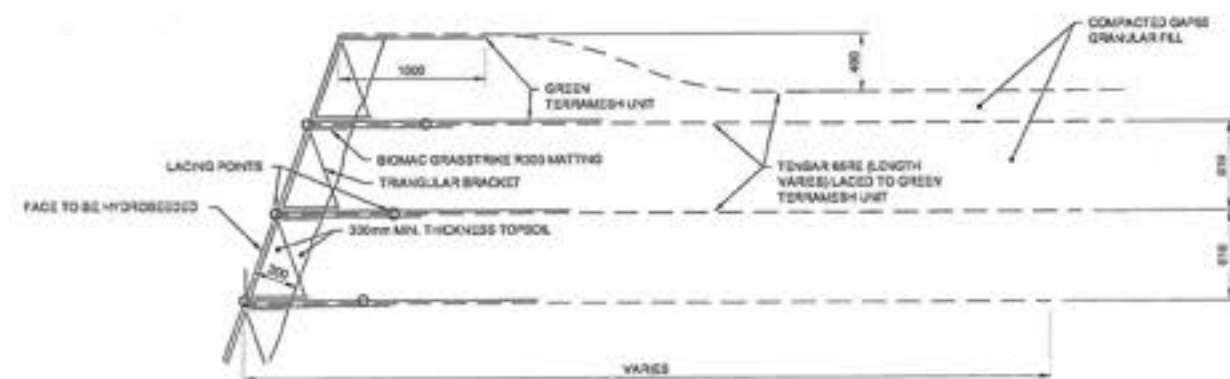
CONSTRUCTION



TYPICAL EMBANKMENT CROSS SECTION
(South of Northern Culvert, chainage 138m)
SCALE: 1:50[A1] 1:100[A3]

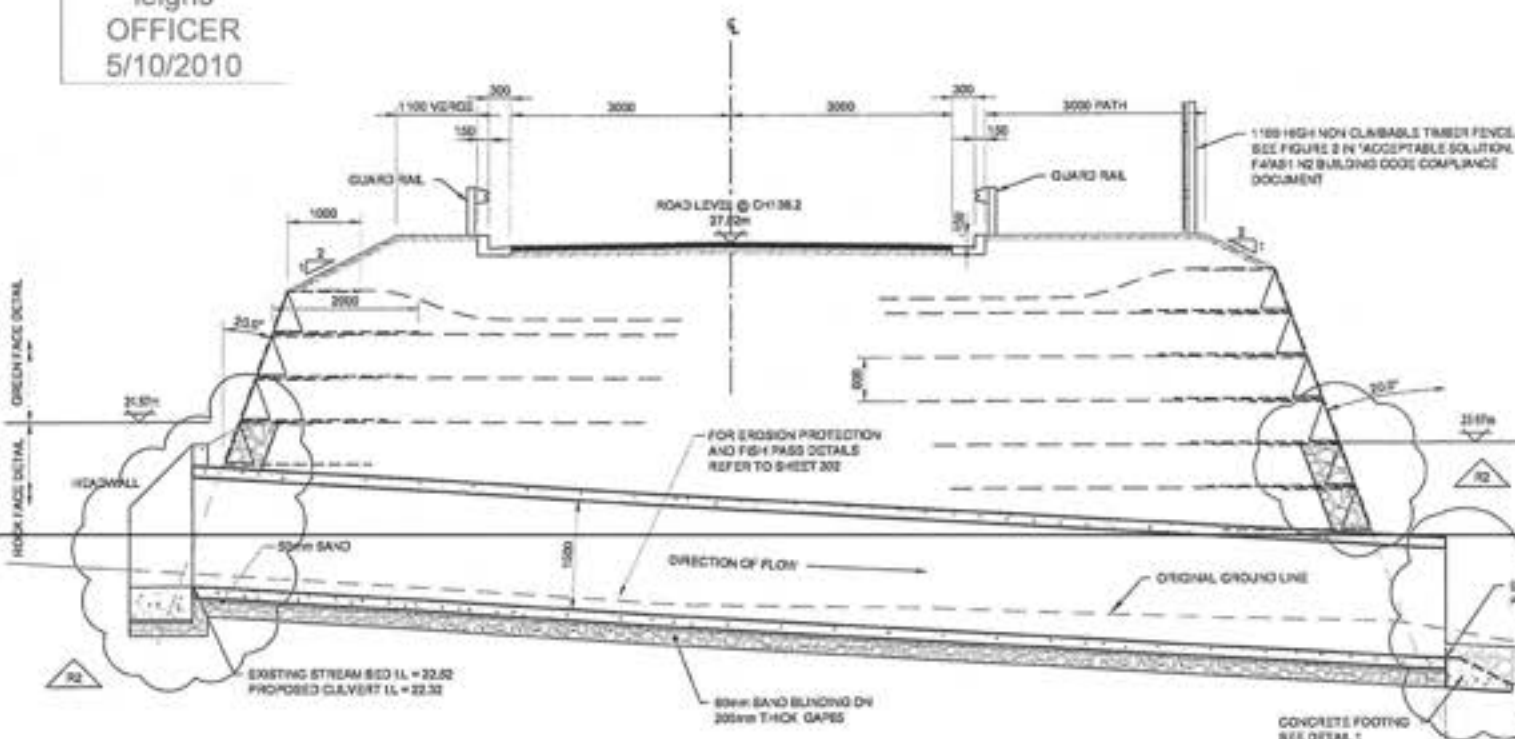


ROCK FACE DETAIL
(Below R.L. of water level during flood)
SCALE: 1:25[A1] 1:50[A3]



GREEN FACE DETAIL
(Above R.L. of water level during flood)
SCALE: 1:25[A1] 1:50[A3]

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NORTHERN CULVERT CROSS SECTION
(by Emerald Glen Road, Chainage 136m)
SCALE: 1:50[A1] 1:100[A3]



**CONCRETE FOOTING
DETAIL 1**
SCALE: 1:20[A1] 1:40[A3]

1. Reinforcement shall be Tensar BSR or BSR Geogrid.
2. The reinforcement shall be connected to the Terramesh unit by lacing at 2 points.
3. The reinforcement shall extend from the face of the Terramesh unit into the fill embankment (lengths shown).
4. The reinforcement shall be overlapped with the highest strength sets perpendicular to the wall face alignment.
5. Reinforcement shall be continuous throughout their embedment lengths and joined side by side to provide 100% coverage.
6. The reinforcement shall be pulled tight prior to backfill placement on the reinforcement.

REFERENCE DRAWINGS
S2573/1/7504/01 TO 305 CULVERTS-EMERALD GLEN
S2573/1/7504/01 TO 313 CULVERTS-ANOR
S2573/1/7504/02 AND 322 OPEN CHANNELS
S2573/1/7504/03 ROAD DRAINAGE

Rev	Description	Appr'd	Reason
R0	ISSUED FOR MFD		
R1	RE-AMENDED AND REVISED	CL	10/09/10
R2	ISSUED FOR CONSTRUCTION	CL	05/10/10



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Project
NZ Transport Agency
Emerald Glen Connection Road, Kapiti
Road Alignment

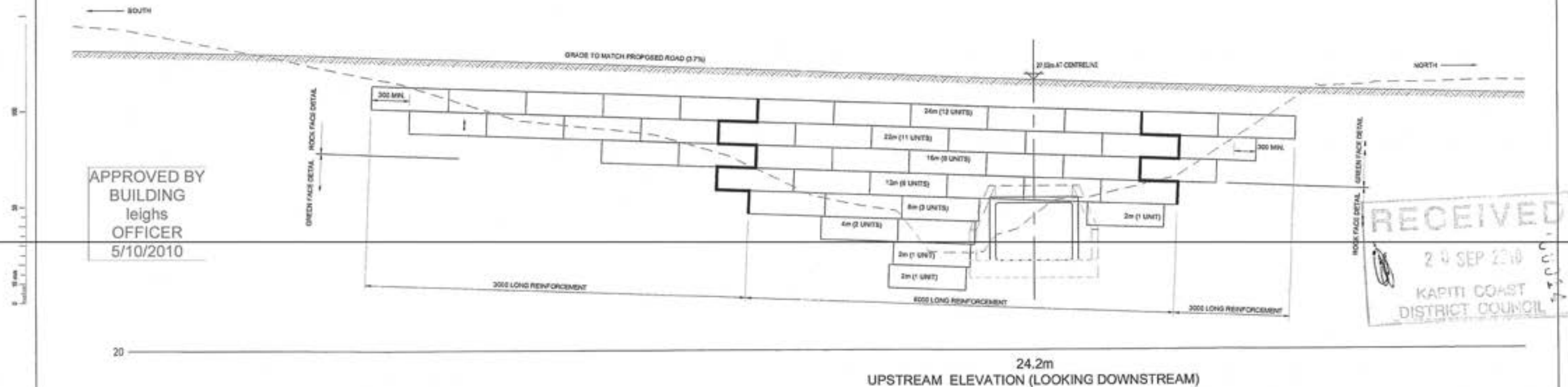
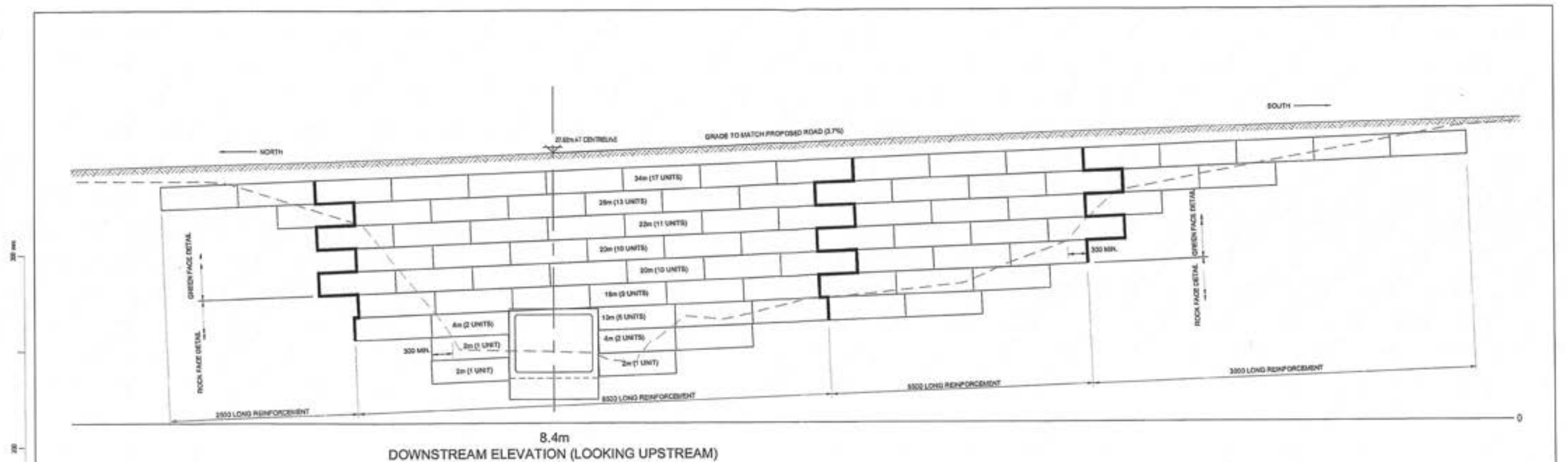
Section
Culverts - Emerald Glen Culvert
Section Showing Terramesh Earth Wall Details

Page No.
5 / 2573 / 1 / 7504

Sheet No.
303

Rev
R2

CONSTRUCTION



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- Notes:
1. Reinforcement shall be Tensar GFRG or GFRG Geogrid.
 2. The reinforcement shall be connected to the Terramesh unit by using at 2 points.
 3. The reinforcement shall extend from the face of the Terramesh unit into the fill embankment (lengths shown).
 4. The reinforcement shall be extended with the highest strength and perpendicular to the wall face alignment.
 5. Reinforcement shall be continuous throughout their embedment lengths and placed side by side to provide 100% coverage.
 6. Spliced connections between shorter pieces of Geogrid or gaps between adjacent pieces of reinforcement are not permitted.
 7. The reinforcement shall be pulled tight prior to backfill placement on the reinforcement.

REFERENCE DRAWINGS

5/2573/1/7504/001 TO 008 CULVERTS-EMERALD GLEN
5/2573/1/7504/011 TO 013 CULVERTS-MANOR
5/2573/1/7504/021 AND 022 OPEN CHANNELS
5/2573/1/7504/031 ROAD DRAINAGE

Rev	Description	Appr'd	Revision Date
R1	ISSUED FOR INFO		
R1	REVALUED AND REVISED	C.L.	15/09/10
R2	ISSUED FOR CONSTRUCTION	C.L.	05/09/10

NZ TRANSPORT AGENCY
WAIKATO

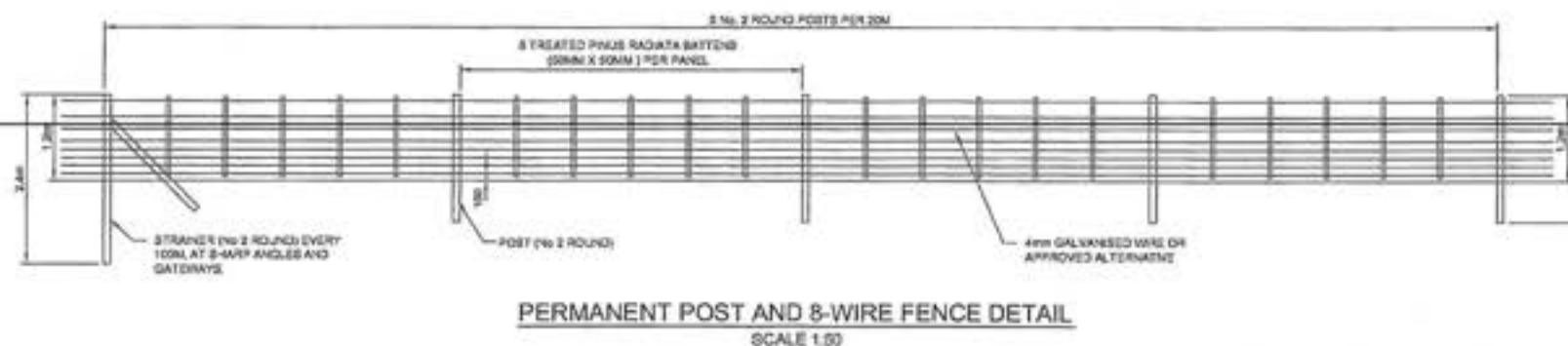
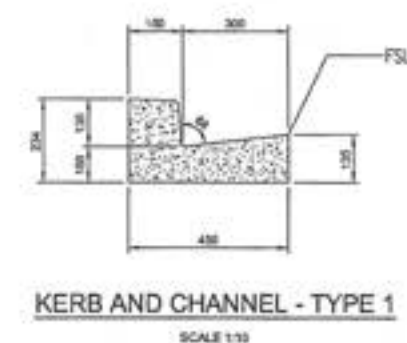
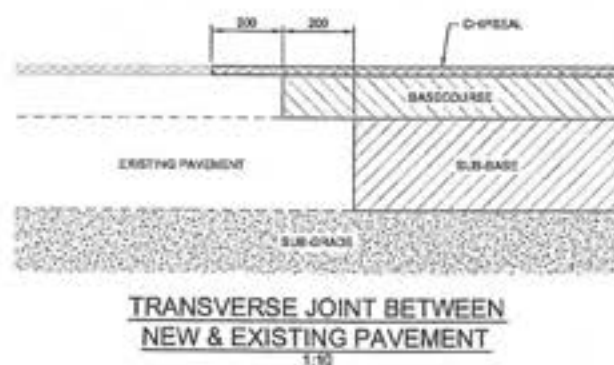
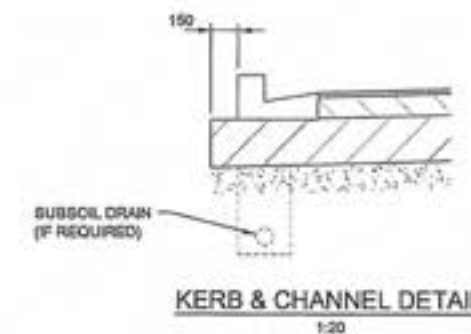
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Wellington 6144, New Zealand
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Project	NZ Transport Agency Emerald Glen Connection Road, Kapiti Road Alignment
Drawn	Culverts - Emerald Glen Culvert End Elevations Showing Terramesh Earth Wall Details
Drawn No.	5 / 2573 / 1 / 7504
Sheet No.	304
Rev	R2

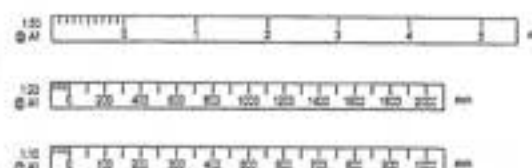


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2 SEP 1964
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DISTRICT COUNCIL

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Order	Account	Amount	Order Date
01	ISSUED FOR CONSTRUCTION	0.00	02/03/2010



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Wellington 6144, New Zealand
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NEW ZEALAND TRANSPORT AGENCY
EMERALD GLEN CONNECTION, KAPITI
ROAD ALIGNMENT

MISCELLANEOUS DETAILS

Drawn	Designed	Approved	Checked Date
G.J. YOUNG	R.S.		
Project No.		Scale	
SC1744.01		AS SHOWN	

5/2573/1/7504

122	R2
-----	----

1 Introduction

The level crossing for the railway between Waterfall Road and State Highway 1 (SH1) is to be closed due to safety concerns. In order to prevent serious inconvenience to the residents who live in the Waterfall Road area, the New Zealand Transport Agency (NZTA) is planning to extend Emerald Glen Road to connect with the existing SH1 connection at Mackays Crossing as shown in Figure 1.

The proposed road alignment crosses predominantly gently sloping (less than 10%) land in the lee of the foothills of the Tararua Range. The alignment crosses two perennial watercourses and one which we consider intermittent. There are also two crossing places for ephemeral land drainage as shown in Figure 2.

Hydraulic analysis has been performed to determine the required sizes of these culverts. Due to the close proximity of the rail tracks and SH1 downstream of the Emerald Glen extension, the influence from downstream culverts was investigated as part of the analysis.

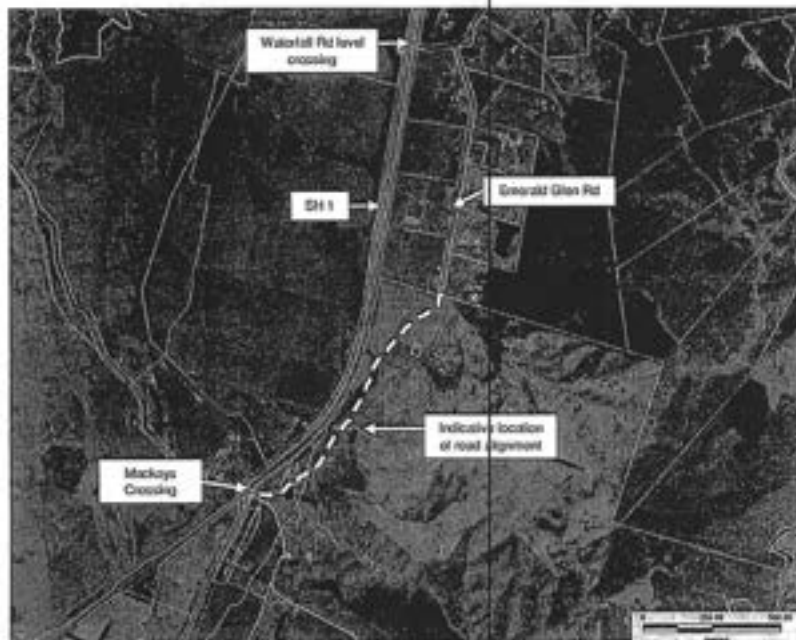


Figure 1 Location of Emerald Glen Road Extension

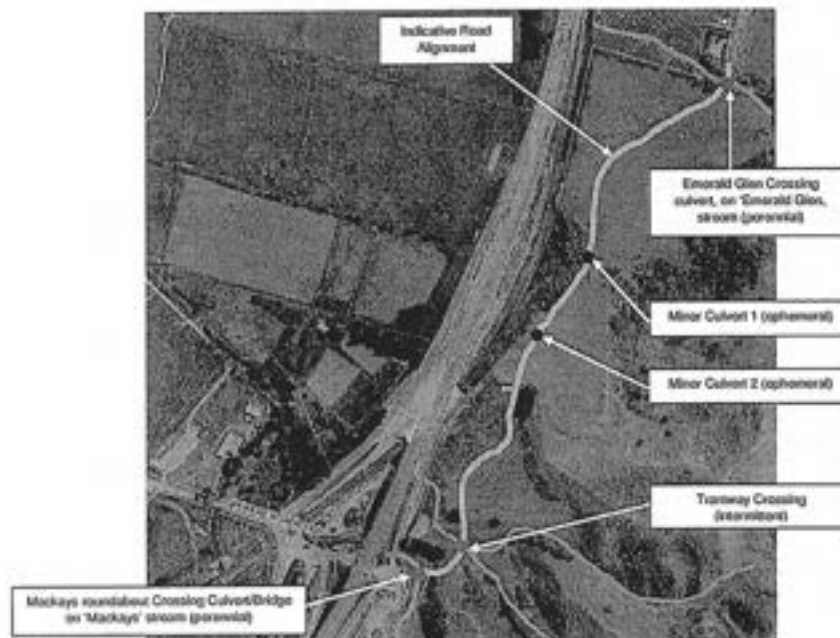


Figure 2 Major & minor waterway crossings along Emerald Glen Road extension

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Figure 1 - Showing Minor Catchments as Hydrology Report.

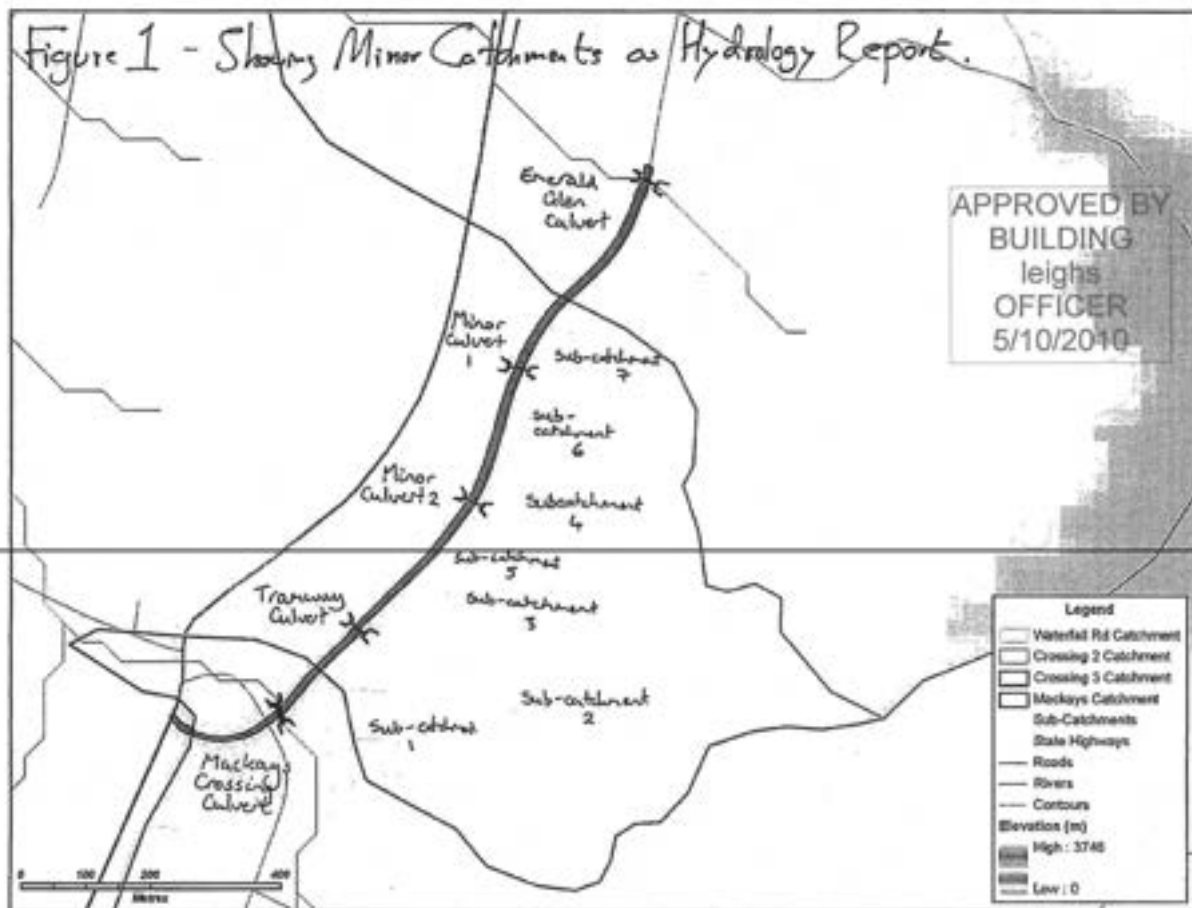
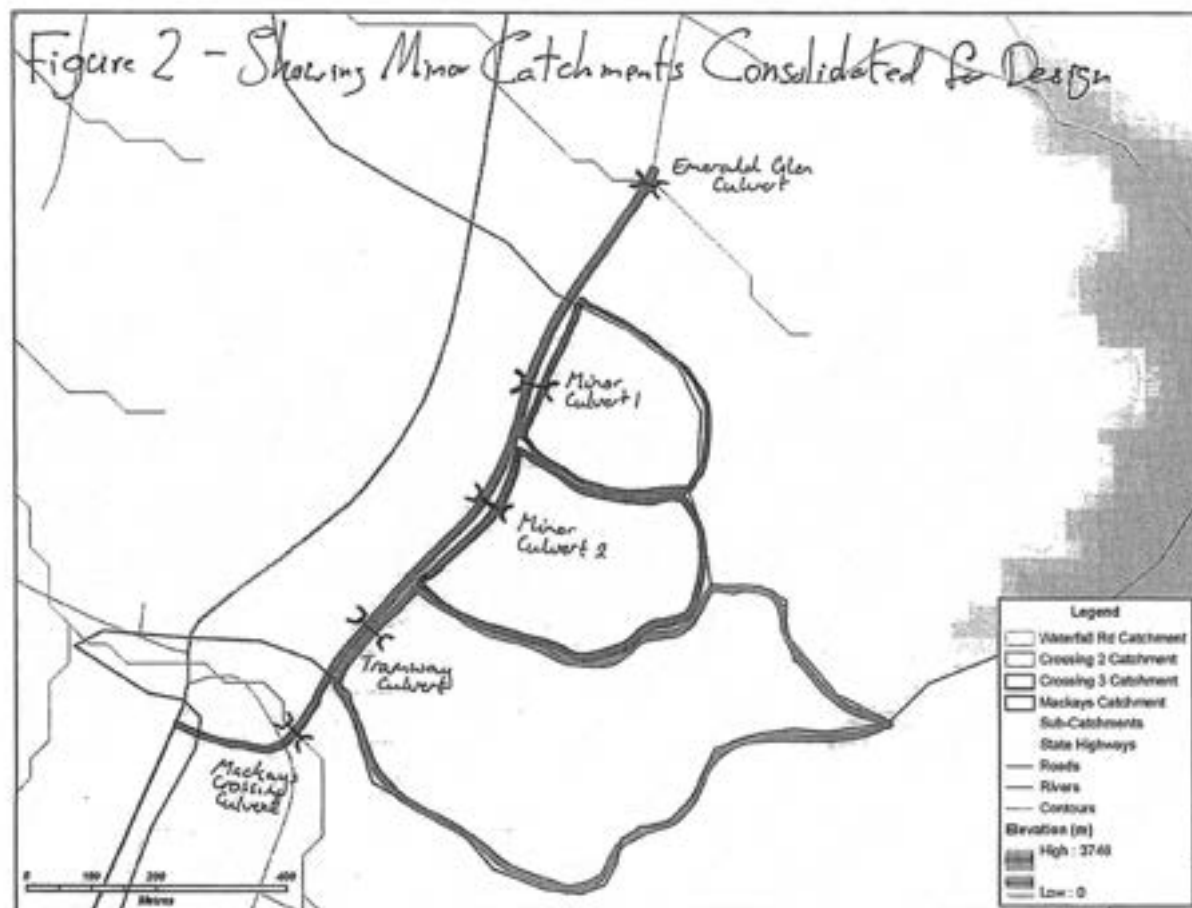
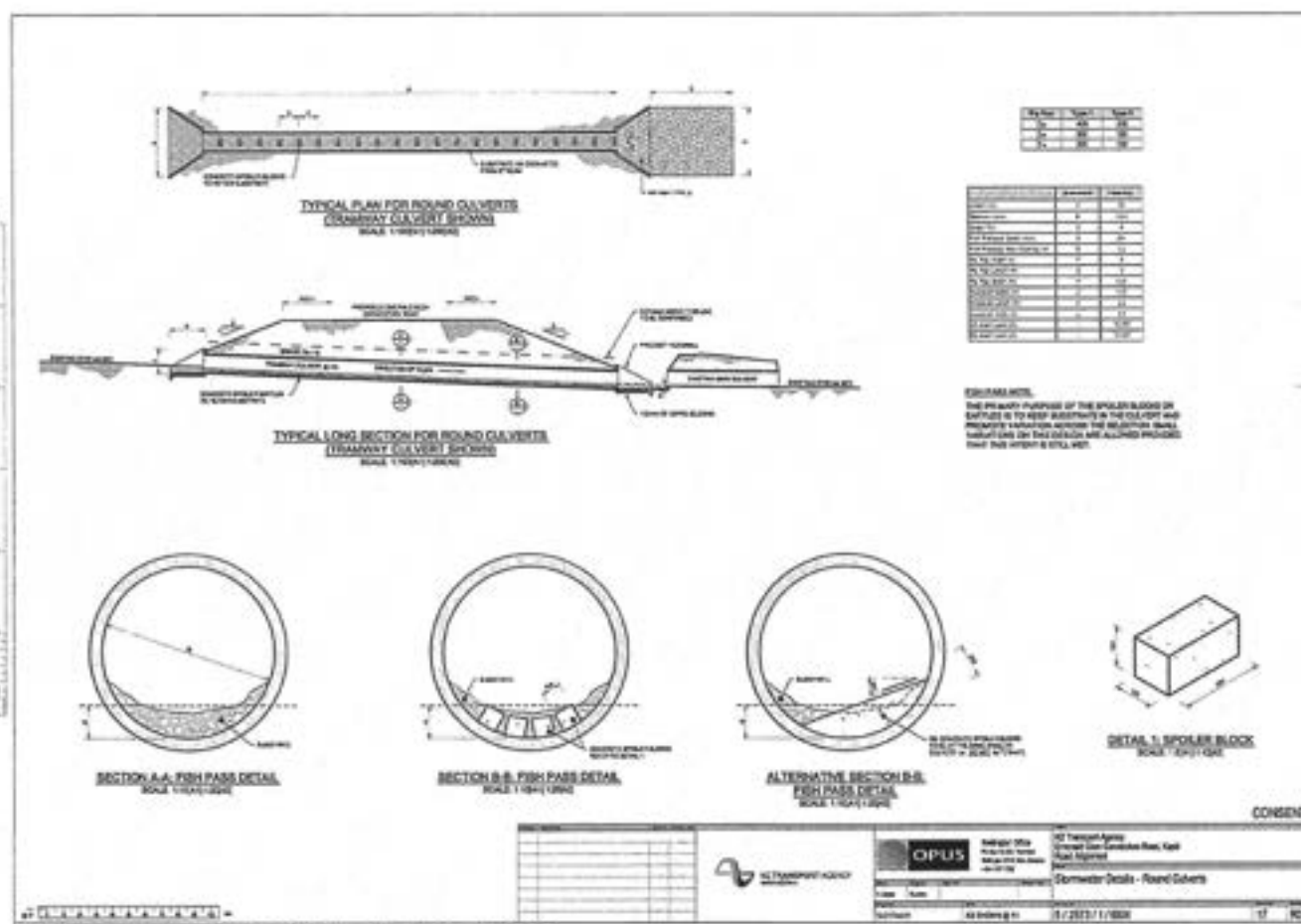
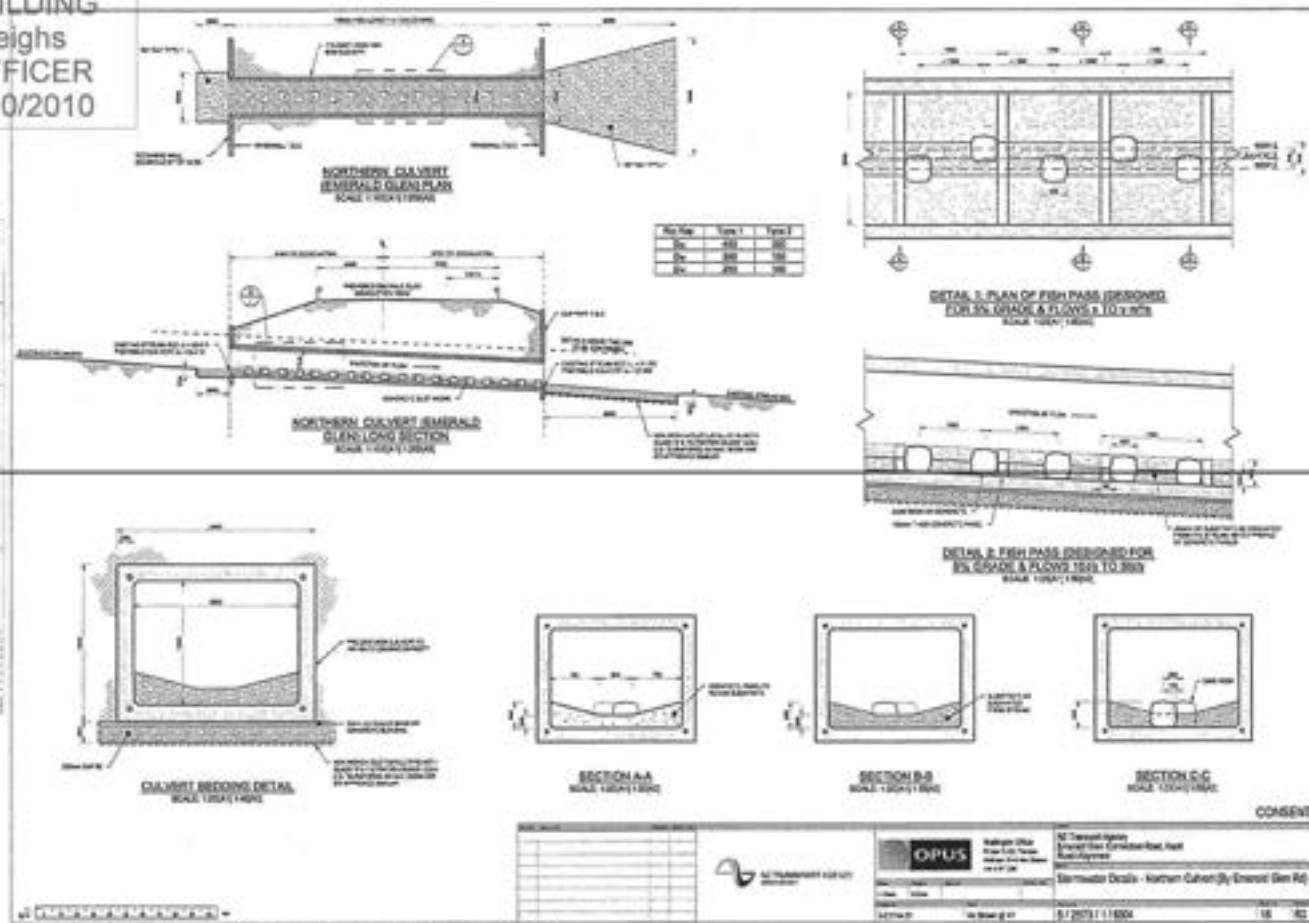


Figure 2 - Showing Minor Catchments Consolidated for Design



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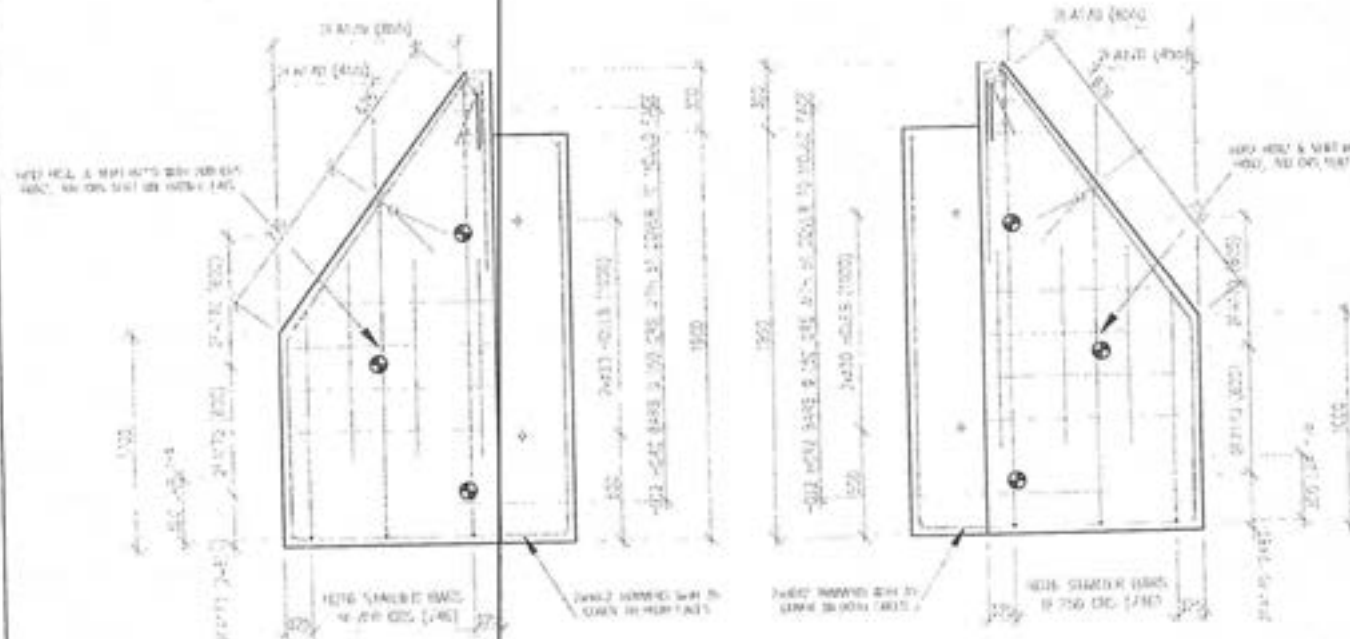
The figure contains two detailed drawings of reinforced concrete beams, labeled (A) and (B), showing their cross-sections and reinforcement layouts.

- Drawing (A):** Shows a rectangular beam section with a total width of 124 inches. It features two main longitudinal bars (top and bottom) and two additional bars near the top corners. Stirrups are provided at 750 c/c. A diagonal bar is shown extending from the bottom corner towards the top center, with dimensions indicating its position relative to the beam's edges.
- Drawing (B):** Shows a similar rectangular beam section with a total width of 124 inches. It also features two main longitudinal bars and two additional bars near the top corners. Stirrups are provided at 750 c/c. A diagonal bar is shown extending from the bottom corner towards the top center, with dimensions indicating its position relative to the beam's edges.

1	MANUFACTURE IN ACCORDANCE WITH NZS 3109:1997 (PRECAST UNITS)
2	CONCRETE STRENGTH @ 28 DAYS 50MPa, MIN. STRIPPING STRENGTH 15MPa
3	SURFACE FINISHES TO NZS 3114:1987, FORMED SURFACE - F4 FINISH TROWELLED SURFACE - U2 FINISH
4	CONCRETE COVER TO REQ. 35mm UNLESS STATED
5	BAR DIMENSIONS TO OUTSIDE
6	B1\B2 EXPOSURE
7	DESIGN: a) HN-HO-72 b) LIFE 100 YEARS

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20 SEP 2000 00004
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DISTRICT COUNCIL

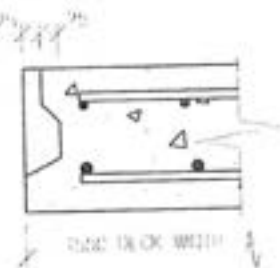
20 SEP 1960

[illegible]

VOLUME:- 0.38m³
WEIGHT:- 0.96Tonne
— REQUIRED

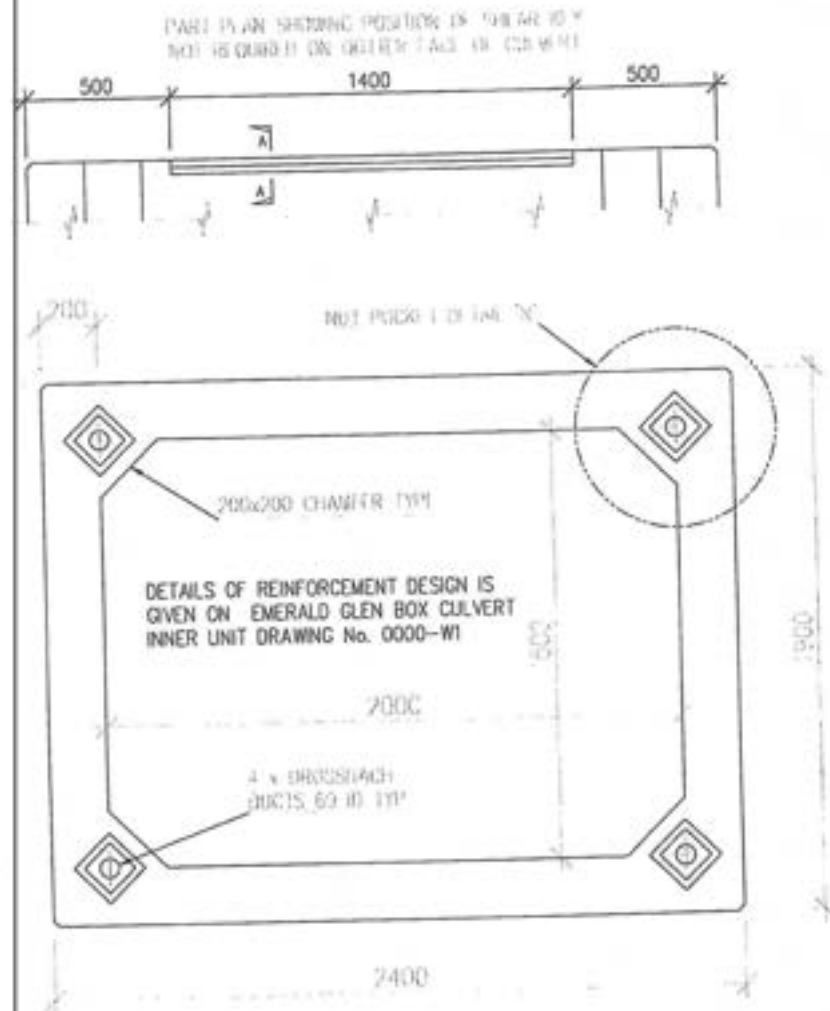
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Rev	Description	Date	By	Proj	Product SPC	Detailed On SHEET 4/4	Status NTS
A	CORRECTIONS	6-28-2019	DB	-	Code 00000	Layout On -	Green GB
					Project Name		Date 9-08-2019
					EMERALD GLEN BOX CULVERTS		Checked By -
					WING WALLS		(ID) -
					Tag Title ELEVATIONS		00000-W4/A
							Drawn By



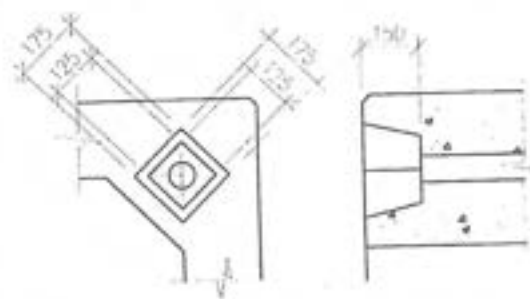
SECTION A-A
SHAR KEY

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BY
BUILDING
leighs
OFFICER
5/10/2010



NOTES:

- 1 MANUFACTURE IN ACCORDANCE WITH NZS 3109:1997 (PRECAST UNITS)
- 2 CONCRETE STRENGTH @ 28 DAYS 40MPa, MIN. STRIPPING STRENGTH 15MPa
- 3 SURFACE FINISHES TO NZS 3114:1987, FORMED SURFACE - F4 FINISH & TROWELLED SURFACE - U2 FINISH
- 4 BOX UNITS ARE NOT SUITABLE FOR USE IN SEA WATER OR TIDAL SITES
- 5 CONCRETE COVER TO REINFORCEMENT IS 40mm INSIDE BOX SURFACE, 35mm OUTSIDE BOX SURFACE
- 6 REINFORCEMENT DENOTED 'DH' IS DEFORMED GRADE 500E STEEL
- 7 DESIGN - LOADING HN-HC-72 DEPTH OF FILL 1000-4000mm DESIGN LIFE 100 YEARS EXPOSURE B1/B2
- 8 BOX UNITS ARE TO BE CURED



DETAIL 'N' OF TAIL

FOR APPROVAL ONLY
NOT FOR CONSTRUCTION



Humes Pipeline Systems
Private Bag 92817
Auckland
Freephone 0800 002 112
www.humes.co.nz

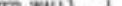
VOLUME 3.88m³
WEIGHT 9.71Tonne
— REQUIRED

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Rev	Description	Date	By	Check	Product	Details On	Scale
1	FOR APPROVAL	10/05/2010	DL		CULVERT	SHEET 2/3	NTS
					Code	SPC00000	Layout On
					Project Name	EMERALD GLEN BOX CULVERTS	
						WINGWALL END UNIT	
					Proj Title	ELEVATIONS	

00000-W2A
Rev. 1

[illegible]

ID = 10-HD16 @ 155 CRS. (INNER DECK)	
IW = 7-HD12 @ 221 CRS. (INNER WALL)	
OW = 11-HD12 @ 141 CRS. (OUTTER WALL)	
IB = 10-HD16 @ 155 (INNER BASE)	
OD = 11-HD12 @ 141 CRS (OUTTER DECK)	
OB = 11-HD12 @ 141 (OUTTER BASE)	
IC = 7-HD12 @ 221 CRS. (INNER CORNER)	

LWS = 28-HD10 (INNER, OUTER
WALL DECK & BASE)

- 1 MANUFACTURE IN ACCORDANCE WITH
NZS 3109:1997 (PRECAST UNITS)
- 2 CONCRETE STRENGTH @ 28 DAYS 40MPa,
MIN. STRIPPING STRENGTH 15MPa
- 3 SURFACE FINISHES TO NZS 3114:1987,
FORMED SURFACE - F4 FINISH
& TROWELLED SURFACE - U2 FINISH
- 4 BOX UNITS ARE NOT SUITABLE FOR USE
IN SEA WATER OR TIDAL SITES
- 5 CONCRETE COVER TO REINFORCEMENT IS
40mm INSIDE BOX SURFACE,
35mm OUTSIDE BOX SURFACE
- 6 REINFORCEMENT DENOTED 'DH' IS
DEFORMED GRADE 500E STEEL
- 7 DESIGN:- LOADING HN-H0-72
DEPTH OF FILL 1000-4000mm
DESIGN LIFE 100 YEARS
EXPOSURE B1\B2
- 8 BOX UNITS ARE TO BE CURED



VOLUME:- 2.54m³
WEIGHT:- 6.58Tonne
— REQUIRED

Rev	Description	Date	By	Product	CULVERT	Details On	SHEET 1/4	Scale	NTS
A	FOR 2000X200	8-28-08	JB	Code	00000	Layout On	-	Drawn	GB
				Project Name	EMERALD GLEN BOX CULVERTS			Date	8-29-08
					2000x1500x200 INNER UNIT			Checked By	-
								(S)	-
				Tag Title	ELEVATIONS			00000-WI	A
								Dep. No.	



NEW ZEALAND INSTITUTE OF
ARCHITECTS
INCORPORATED



Building Code Clause(s) **B1**

PRODUCER STATEMENT – PS4 – CONSTRUCTION REVIEW

(Guidance notes on the use of this form are printed on the reverse side)

Validated in
accordance with KCDC Policy 002
14 APR 2012

ISSUED BY: Beca Carter Hollings and Ferner, Ltd.

(Construction Review Firm)

TO: NZTA

(Client/Developer)

TO BE SUPPLIED TO: Kapiti Coast District Council

(Building Consent Authority)

IN RESPECT OF: Northern Box Culvert

(Description of Building Work)

AT: Emerald Glen Connection Road, Kapiti

(Address)

Signature: 

LOT DP SO

Beca Carter Hollings and Ferner, Ltd.

(Construction Review Firm)

has been engaged by NZTA

to provide ☐ CM1 ☐ CM2 ☒ CM3 ☐ CM4 ☐ CM5 (Engineering Categories) or ☐ OL1 ☐ OL2 ☐ OL3 ☐ OL4 (Architectural Categories)

observation ☐ or other services

(Extent of Engagement)

in respect of clause(s) **B1** of the Building Code for the building work described in

documents relating to Building Consent No. **100767** and those relating to

Building Consent Amendment(s) Nos. **N/A** issued during the

course of the works. We have sighted these Building Consents and the conditions of attached to them.

Authorised instructions / variation(s) No. **N/A** (copies attached)

or by the attached Schedule ☐ have been issued during the course of the works.

On the basis of ☒ this ☐ these review(s) and information supplied by the contractor during the course of the works, I believe on reasonable grounds that ☐ All ☒ Part only of the building works have been completed in accordance with the relevant requirements of the Building Consents and Building Consent Amendments identified above, with respect to Clause(s) **B1** of the Building Regulations. **(EXCLUDES LONGITUDINAL DESIGN OF MULTIPLE CULVERTS)**

I, Geoff Brown

(Name of Construction Review Professional)

am: ☐ CPEng No.

☐ Reg Arch No.

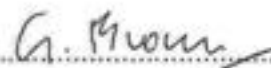
I am a Member of: ☐ IPENZ ☐ NZIA and hold the following qualifications: **BEng, MICE, MStructE, CEng (UK)**

The Construction Review Firm issuing this statement holds a current policy of Professional Indemnity Insurance no less than \$200,000*

The Construction Review Firm is a member of ACENZ: ☒ YES ☐ NO

SIGNED BY Geoff Brown ON BEHALF OF Beca Carter Hollings and Ferner, Ltd.

Date: 15-02-2012

Signature: 

Note: This statement shall only be relied upon by the Building Consent Authority named above. Liability under this statement accrues to the Construction Review Firm only. The total maximum amount of damages payable arising from this statement and all other statements provided to the Building Consent Authority in relation to this building work, whether in contract, tort or otherwise (including negligence), is limited to the sum of \$200,000*.

This form to accompany Forms 6 or 8 of the Building (Form) Regulations 2004 for the issue of a Code Compliance Certificate.

GUIDANCE ON USE OF PRODUCER STATEMENTS

Producer statements were first introduced with the Building Act 1992. The producer statements were developed by a combined task committee consisting of members of the New Zealand Institute of Architects, Institution of Professional Engineers New Zealand, Association of Consulting Engineers New Zealand in consultation with the Building Officials Institute of New Zealand. The original suite of producer statements has been revised as at the date of this form as a result of enactment of the Building Act (2004) by these organisations to ensure standard use within the industry.

The producer statement system is intended to provide Building Consent Authorities (BCAs) with reasonable grounds for the issue of a Building Consent or a Code Compliance Certificate, without having to duplicate design or construction checking undertaken by others.

PS1 Design	Intended for the use by a suitably qualified independent design professional in circumstances where the BCA accepts a producer statement for establishing reasonable grounds to issue a Building Consent;
PS2 Design Review	Intended for use by a suitably qualified independent design professional where the BCA accepts an independent design professional's review as the basis for establishing reasonable grounds to issue a Building Consent;
PS3 Construction	Forms commonly used as a certificate of completion of building work are Schedule 6 of NZS 3910:2003 ¹ ; or Schedules E1/E2 of NZIA's SCC 2007 ²
PS 4 Construction Review	Intended for use by a suitably qualified independent design professional who undertakes construction monitoring of the building works where the BCA requests a producer statement prior to issuing a Code Compliance Certificate. This must be accompanied by a statement of completion of building work (Schedule 6).

The following guidelines are provided by ACENZ, IPENZ and NZIA to interpret the Producer Statement.

Competence of Design Professional

This statement is made by a Design Firm that has undertaken a contract of services for the services named, and is signed by a person authorised by that firm to verify the processes within the firm and competence of its designers.

A competent design professional will have a professional qualification and proven current competence through registration on a national competence-based register, either as a Chartered Professional Engineer (CPEng) or a Registered Architect.

Membership of a professional body, such as the Institution of Professional Engineers New Zealand (IPENZ) or the New Zealand Institute of Architects (NZIA) provides additional assurance of the designer's standing within the profession. If the design firm is a member of the Association of Consulting Engineers New Zealand (ACENZ), this provides additional assurance about the standing of the firm.

Persons or firms meeting these criteria satisfy the term "suitably qualified independent design professional".

* Professional Indemnity Insurance

As part of membership requirements, ACENZ requires all member firms to hold Professional Indemnity Insurance to a minimum level.

The PI insurance minimum stated on the front of this form reflects standard, small projects. If the parties deem this inappropriate for large projects the minimum may be up to \$500,000.

Professional Services during Construction Phase

There are several levels of service which a Design Firm may provide during the construction phase of a project (CM1-5)³ (OL1-OL4)². The Building Consent Authority is encouraged to require that the service to be provided by the Design Firm is appropriate for the project concerned.

Requirement to provide Producer Statement PS4

Building Consent Authorities should ensure that the applicant is aware of any requirement for producer statements for the construction phase of building work at the time the building consent is issued as no design professional should be expected to provide a producer statement unless such a requirement forms part of the Design Firm's engagement.

Attached Particulars

Attached particulars referred to in this producer statement refer to supplementary information appended to the producer statement.

Refer Also:

- 1 *Conditions of Contract for Building & Civil Engineering Construction NZS 3910: 2003*
- 2 *NZIA Standard Conditions of Contract SCC 2007 (1st edition)*
- 3 *Guideline on the Briefing & Engagement for Consulting Engineering Services (ACENZ/IPENZ 2004)*

www.acenz.org.nz
www.ipenz.org.nz
www.nzia.co.nz



PRODUCER STATEMENT – PS1 – DESIGN

(Guidance notes on the use of this form are printed on the reverse side*)

 ISSUED BY: Opus International Consultants Limited
(Design Firm)

 TO: New Zealand Transport Authority
(Owner/Developer)

 TO BE SUPPLIED TO: Kapiti Coast District Council
(Building Consent Authority)

 IN RESPECT OF: Terramesh Green Wall
(Description of Building Work)

 AT: Emerald Glen Connection, Northern Culvert
(Address)
N/A

 LOT N/A

DP

N/A

 SO N/A

 We have been engaged by the owner/developer referred to above to provide design of retaining wall
at the northern culvert services in respect of the requirements of

 Clause(s) B1 and B2 of the Building Code for

☐ All or ☒ Part only (as specified in the attachment to this statement), of the proposed building work.

The design carried out by us has been prepared in accordance with:

☒ Compliance Documents issued by Department of Building & Housing B1/VM4
(verification method / acceptable solution)
☐ Alternative solution as per the attached schedule

 The proposed building work covered by this producer statement is described on the drawings titled Culverts - Emerald Glen Culvert Section Showing and numbered S/2573/117504/303m2 & 304 R2
Terramesh Earth Wall Details together with the specification, and other documents set out in the schedule attached to this statement.

On behalf of the Design Firm, and subject to:

- (i) Site verification of the following design assumptions refer to the Specification
- (ii) All proprietary products meeting their performance specification requirements;

I believe on reasonable grounds the building, if constructed in accordance with the drawings, specifications, and other documents provided or listed in the attached schedule, will comply with the relevant provisions of the Building Code.

 I, Alexei Murashev am: ☒ CPEng 178533 #

(Name of Design Professional)
☐ Reg Arch #

 I am a Member of: ☒ IPENZ ☐ NZIA and hold the following qualifications:

 The Design Firm issuing this statement holds a current policy of Professional Indemnity Insurance no less than \$200,000*.
 The Design Firm is a member of ACENZ ☒ YES ☐ NO

 SIGNED BY Alexei Murashev ON BEHALF OF Opus International Consultants
(Design Firm)

 Date 14/09/10 (signature) [Signature]

Note: This statement shall only be relied upon by the Building Consent Authority named above. Liability under this statement accrues to the Design Firm only. The total maximum amount of damages payable arising from this statement and all other statements provided to the Building Consent Authority in relation to this building work, whether in contract, tort or otherwise (including negligence), is limited to the sum of \$200,000*.

This form is to accompany Form 2 of the Building (Forms) Regulations 2004 for the application of a Building Consent.

Tony King

From: Leigh Sage
Sent: Thursday, 29 March 2012 5:01 p.m.
To: Tony King
Subject: FW: Geoff Brown CV
Attachments: Geoff Brown CV.pdf

Hi Tony,

Here is some information you can use in determining whether Mr. Brown is acceptable under AP_WI_020 – Acceptance of Expert Opinion, in line with Policy 002 – Producer Statements

Leigh Sage
Building Control Manager
Te Kaiwhakahaere Tautuku Whare

Kāpiti Coast District Council
Tel 04 296 4650
Mobile 027 5555 650
www.kapiticoast.govt.nz

From: Geoff Brown [mailto:Geoff.Brown@beca.com]
Sent: Thursday, 29 March 2012 4:32 p.m.
To: Leigh Sage
Subject: Geoff Brown CV

Leigh

Further to our discussion, I attach my CV which demonstrates my extensive experience in the design and construction of bridges over the last 33 years. I am the Technical Director for bridges in Beca's Wellington office where I look after bridge design and construction management across the lower North island and South Island.

I am a professional engineer and member of the Institutions of Civil Engineers and Structural Engineers in the UK and I am a Chartered Engineer in the UK. I have been working in New Zealand for nearly 15 years and have previously been MIPENZ and Registered Engineer in New Zealand.

I am responsible for bridge design and construction management for many bridges across New Zealand and submit PS1, 2 and 4 statements on a regular basis to all the main local authorities including Auckland Council, Wellington and Christchurch where my qualifications and experience are accepted.

Please contact me if you require any additional information.

Regards

Geoff

Geoff Brown
Technical Director, Bridges
Beca
Phone +64-4-473 7551 Fax +64-4-473 7911
DDI +64-4-496 2527 Mobile +64-021-242 4837
geoff.brown@beca.com

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Geoff Brown – Bridges & Civil Structures

BEng (Hons Class 2:1) Civil and Structural Engineering, University of Bradford, 1978

Certificate in Management, Sheffield Business School, 1998

MICE, MStruct E, Chartered Engineer.

Geoff is a structural specialist with over 30 years experience in the design and construction of bridges and civil structures. He also has led the design of major highway projects during the scheme investigation and detailed design stages. He is currently a Technical Director with Beca Group in New Zealand where he leads the bridge and civil structures engineering discipline in the Wellington office. Geoff was previously Chief Engineer, Bridges & Structures with RailCorp in Sydney, Principal Engineer and Partner with Opus International Consultants in Wellington and commenced his career with Husband and Company and subsequently Mott MacDonald in the UK where he was the Principal Engineer for bridges in the Sheffield office until 1997. Geoff also spent 12 months working in Taiwan with Mott MacDonald.

Geoff has specialist skills across the full spectrum of bridge design, concrete box girder bridges constructed in balanced cantilever and by incremental launching and asset management of bridges. He also has specialist skills related to rail structures through the design of numerous rail bridges and from his role with RailCorp. Geoff has strong design management skills for bridge and highway projects as demonstrated through his team leadership of a major road design projects.

Membership	- Member, Institution of Civil Engineers, 1982 - Member, Institution of Structural Engineers, 1984 - Chartered Engineer (UK)
Special competence	Project leadership for transportation projects, bridges and civil structures design and asset management of structures
Awards	- ACENZ Merit Award in 2002 for Shelley Beach Overbridge Lengthening - ACENZ Gold Award in 2002 for Candys Bend to Starvation Point Reconstruction - International Roads Federation Environmental (IRF) Award in 2003 for Candys Bend to Starvation Point Reconstruction - Austroads Bridge Award in 2004 for the structures at Candys Bend to Starvation Point Reconstruction
Background	2008 – Present Technical Director, Bridge Engineering, Beca Group, Wellington, NZ 2005 – 2008 Chief Engineer, Bridges & Structures, RailCorp, Sydney, Australia 1997 – 2005 Principal, Bridges & Civil Engineering and Partner, Opus International Consultants, Wellington, NZ 1991 – 1997 Principal Engineer, Mott MacDonald Group, Sheffield, UK & Taiwan 1978 – 1991 Graduate, Assistant, and Senior Engineer, Husband & Co, Sheffield, UK

Selected experience

SH73 Christchurch Southern Motorway, Christchurch, New Zealand Transport Agency, 2009-Present

Overall structural design leadership and verification for design and construct tender phase and now detailed design stage for 16 structures comprising 3 span overbridges and underpasses, interchange bridges, subways and culverts for \$100M motorway duplication and extension working for major contractor.

SH20 Waterview Connection, Auckland, New Zealand Transport Agency, 2008-Present Structural design package leader responsible for developing structural forms and providing structural inputs to the resource consent applications for structures on this \$1.2B section of new motorway. Structures comprise a grade separated interchange comprising 4 major elevated ramps up to 500m long, a 450m cut and cover tunnel, a 3-span overbridge, a stream crossing and 4 pedestrian bridges.

Review of ONTRACK Bridge Inspection Standards, Wellington, ONTRACK, 2008

Review ONTRACK's bridge inspection standards on basis of types of structures, condition and materials based on literature search of similar organisations, site inspections and risk assessment and prepared report with recommended changes to standards.

SH2 Matahorua Gorge Realignment, Napier, New Zealand Transport Agency – 2008-09

Structural team leader for the specimen design of a major bridge over Matahorua Gorge and a Rail Overbridge over the Napier-Gisborne Railway. Estimated total cost of structures is \$11M. Gorge crossing is a 3-span bridge, 150m long with 50m high piers. Post award review of the designs was also undertaken.

Port Botany Expansion, Sydney, Sydney Ports Corporation, 2008-09

Structural verifier for the design of the marine structures for the expansion of the container port in Sydney involving 1.5km of 20m high RC counterfort walls, blockwork caissons, coping beams and crane beams. The design was procured under a design and construct contract and Geoff worked for the project verifier team as the structural reviewer. Total project cost is \$500M.

Johnsonville Tunnel Modifications, Wellington, ONTRACK, 2008

Design of modifications to the 7 existing brick tunnel linings for invert lowering and tunnel widening to suit larger trains. The tunnel brick lining was anchored with ground anchors and a design prepared to widen the tunnels by installing precast wall units to replace the brickwork over their lower height. Total project cost \$2.5M.

Grade Leadership of Corporate Governance Program, Sydney RailCorp, 2005 – 2008

Discipline head responsible for the lead program of corporate governance activities related to bridges and structures including asset integrity audits, development of new standards and preparation of the Annual Asset Integrity Report for Bridges and Structures, including presentation to the RailCorp Board of Directors. Also includes technical leadership activities across RailCorp and with external suppliers.

Review of Design for New Rail Infrastructure being designed for TIDC, Sydney, RailCorp, 2005-2008

Discipline head responsible for the review and acceptance of designs on behalf of RailCorp produced by TIDC and their contractors for new rail infrastructure including bridges, retaining walls, station structures, overhead wiring structures, station buildings and drainage as part of \$3B Rail Clearways project. This includes allocation of engineering authority for all design to appropriately qualified personnel. Reviews included extensive retaining walls associated with track duplications and turn-back roads.

MacKays Crossing D&C, Wellington Fulton Hogan Construction, 2004 – 2005

Design Manager responsible for the leading tender and detailed design for roading and bridging aspects of this design and construct (D&C) grade separated interchange project, comprising a rail overbridge, underpass and approach embankments with \$18m tender cost. Working for Fulton Hogan Ltd on Opus' first successful D&C roading related project in NZ.

SH2 Rimutaka Hill (Muldoon's Corner) Upgrade, Wellington, Transit New Zealand, 2003-2005

Team leader responsible for the investigation of options for scheme assessment stage of this project to improve the alignment of SH2 on its approach to the summit. A range of geometric options were considered with different design speeds to find the most appropriate overall solution. Large scale cuttings and geogrid reinforced embankments were required and ecological considerations were an important aspect of this project.

Newmarket Viaduct Upgrade, Auckland Transit New Zealand, 2002 – 2003

Structural Group Leader responsible for the assessment of existing bridge and development of scheme options to retrofit for dead and live loads and widen the 16 span 650m long Newmarket Viaduct originally constructed in balanced cantilever with typical 42m spans. \$60M scheme cost.

Upper Harbour Bridge, Auckland Transit New Zealand, 2001 – 2003

Bridge Team Leader and Principal's Advisor for design/build responsible for the conceptual and specimen design of \$30M duplicate crossing of Upper Harbour using 450m long box girder bridge to be constructed in balanced cantilever with 73m typical spans.

Shelly Beach Bus Priority, Auckland Transit New Zealand, 1999 – 2001

Christchurch Southern Motorway, Christchurch, Transit New Zealand, 2001 – 2002

Team Leader responsible for the development of the scheme for \$100m motorway extension and duplication of 10.5km length including five grade separated interchanges and construction over landfills. Managing project team across Opus' Wellington, Christchurch and Auckland offices.

Waiohine Bridge Upgrade, Wellington, Transit New Zealand, 1999 – 2002

Team Leader responsible for the investigation of options and detailed design for bridge upgrade to improve width and security against scour and seismic failure. Options include full replacement and widening of existing bridge.

Shelly Beach Bus Priority, Auckland Transit New Zealand, 1999 – 2001

Structural Team Leader responsible for the development of innovative design solutions to increase the span of the existing bridge over SH1 Motorway on approach to Harbour Bridge, options report and detailed design of selected option. Winner of Bronze ACENZ Award in 2002.

Candys Bend to Starvation Point Reconstruction, Arthurs Pass, Transit New Zealand, 1998 – 2001

Structural Team Leader responsible for the design and scheme assessment for structures on the Candys Bend to Starvation Point Realignment on SH73 in Otira Gorge for Transit New Zealand. Structures include cantilever half bridging, rock shelters and a three span stream bridge. Winner of Gold ACENZ Award in 2002, IRF Environmental Award in 2003 and Austroads Bridge Award in 2004.

Grade Separated Interchange, Malaysia United Engineers Construction, 1997 - 1998

Project manager for concept and detailed design of grade separated interchange on existing North-South Freeway. Co-ordinated inputs from roading, geotechnical and bridging engineers and production of tender documentation.

Bridge Asset Management Contract, Wellington, Transit New Zealand, 1997– 2000 & 2004 -2005

Team leader and Bridge Inspection Engineer responsible for the Region 9 bridge inspection contract comprising the inspection and asset management of 160 bridges in the Wellington region. Geoff carried out general and detailed bridge inspections, developed list of repairs for structural and routine maintenance and organised repair contracts each year for the works. Geoff also provided a range advise to Transit on asset management and specific problems that arose.

High Speed Railway, Taiwan Taiwan High Speed Rail Authority, 1995

Structural Team Leader responsible for the preliminary design of 30 km of high-speed railway viaduct to be constructed using span by span techniques, based in Taipei. Led team of local engineers through analysis, design of box girder prestress and construction methodology for 25m long box girder units to be precast and erected in full span lengths.

Freeway Bridge Widening, Taiwan Taiwan Highway Bureau, 1993 – 1994

Structural Team Leader responsible for the preliminary and detailed design of widening to existing freeway structures to provide an additional traffic lane. Based in Taipei. Structures comprised a 28 span river crossing, a 9 span viaduct and two 3 span river bridges.

Balanced Cantilever Freeway Bridge, Taiwan Taiwan Highway Bureau, 1993 – 1994

Structural Team Leader responsible for the detailed design of twin 150m long box girder bridges to be constructed in balanced cantilever with 70m main spans using specialist time dependent analysis software. Based in Taipei.

A638 Improvements, Doncaster, UK Department of Transport, 1991 - 1993

Bridge Group Leader responsible for the design supervision of the design team for preliminary and detailed design of six highway structures including subways, footbridge, flood relief structures and retaining walls in an urban environment with high traffic flows.

Grade Separated Underpasses, Qatar, Middle East, Government of Qatar, 1982-1985

Design Team Leader responsible for the design of three grade separated underpasses to improve capacity of congested intersections in Doha City. Underpasses comprised major concrete retaining walls and roundabout bridges to be constructed in heavily trafficked and serviced corridor.

A55 Improvements, North Wales, UK Welsh Office, 1991-1993

Bridge Group Leader responsible for the design supervision of the design team for preliminary and detailed design of five highway structures including two steel-concrete composite overbridges to be constructed over existing trunk road, and prestressed concrete stream bridges.

16 Span Glued Segmental Viaduct, Doncaster, UK Doncaster MBC, 1987-1992

Structural Team Leader responsible for the conceptual and detailed design of a 650m long 16 span concrete box girder viaduct with typical 42m spans to be constructed using balanced cantilever precast glued segmental construction.

Tyne Crossing, Newcastle, UK Department of Transport, 1985 - 1986

Team Leader responsible for the detailed independent design check of twin 300m long 5 span concrete box girder river bridges constructed using balanced cantilever over the River Tyne with 107m main span and 17 span steel composite approach viaduct.

Stanley Ferry Aqueduct, Wakefield, British Waterways Board, 1979 -1980

Bridge Designer responsible for the design of an innovative push-launch aqueduct structure to carry a canal over the River Calder. Design of prestress and launching analysis for the large U-shaped trough structure including temporary cable stays to support the cantilever during launching.

Thorne Colliery Overbridge, Doncaster, National Coal Board, 1978 -1981

Bridge Designer and Resident Engineer responsible for the design of a single span prestressed concrete access road bridge to new colliery constructed over railway. Site supervision of road and bridge construction.

Training Courses Attended

1992	Certificate in Management, Sheffield Business School (1 year part time)
1997	Project Management, APESMA (2 days)
1998	Managing Strategic Relationships, Target Marketing Systems (2 days)
1998	Effective Proposal Management, Opus (1 day)
2000	Business Relationship Skills, Opus (2 days)
2000	Business Management, Melbourne Business School (4 days)
2001	Contract Law, Ingenium (1 day)
2002	Expert Witness, Simpson Grierson (1/2 day)
2002	Transfund CPP Procedures, NZIHT (1 day)
2003	Presentation Skills (1 day)
2004	Risk Management (1 day)

2005 Leadership, Change & Thinking (1day)
2006 Just Culture (3 days)
2008 Project Director (Beca)

Papers and Presentations

New Standard Bridge Beams for New Zealand, Austroads Bridge Conference, Auckland, 2009.

A Bridge Management System for Sydney's Railways, Austroads Bridge Conference, Perth 2006.

Newmarket Viaduct Upgrade, Auckland, Austroads Bridge Conference, Perth 2006.

Shelley Beach Overbridge Lengthening, Austroads Bridge Conference, Hobart, 2004.

New Standard Precast Concrete Bridge Beams: Stage 1 – Identification of New Standard Beam Shapes, Transfund New Zealand Research Report No 252, Gray, Gaby, Brown, Kirkcaldie, Cato and Sweetman, 2003.

Highway Structures in the Gulf, Institution of Civil Engineers/Institution of Structural Engineers, Sheffield, 1986.

KCDC Building Consent Authority
Building Consent Documentation Filed



BC No.
Street Address.

**Retaining Walls over culvert
Road Reserve near
86 Emerald Glen Road**

Tick indicates document(s) included in this Building Consent

Building Consent Application,	☒
Application checksheet,	☒
Costing schedule,	☒
Plans and specifications,	☒
Invoice and receipt,	☒
Processing records/checksheets,	☒
Inspection page printouts	
1-9 18 37 40 55-58 72	
Building consent and memorandum,	☒
Inspection reports,	☒
Application for CCC,	☒
Code of Compliance Certificate.	☒
Producer statements and supporting documentation,	☒
Peer review documents,	☐
Fire design documents,	☐
Requests for further information and other correspondence,	☐
Amendments,	☐
Supplementary information,	☐
Energy work certificates,	☐
Notices to Fix.	☐

This form comprises part of the Building Consent documentation and indicates the contents of the Building Consent File. Refer KCDC Policy 029.

Criteria

s or major projects. Refer BI-WI-024

Address:

**Retaining Walls over culvert
Road Reserve near
86 Emerald Glen Road**

Date: 27/3/2012

BC No: 100764.

application for code compliance certificate (Form 278) completed

☒ Y☐ N

Confirm all required inspections have been completed and passed:

Answer: ☒ Yes or No

If inspections have been missed, what supporting info and method of verification has been provided to compensate for missed inspection. (Describe):

Work in accordance with approved consent and approved amendments

☒ Yes☐ No

If no, has NTF been issued:

☒ N/A☐ Yes☐ No

Have items of non compliance been resolved and NTF uplifted.

☒ N/A☐ Yes☐ No

Note: If work is not in accordance with consent, (and approved amendments) or NTF matters have not been resolved, suspend application for Code Compliance Certificate

Supporting documentation required in building consent addenda (Accept in accordance with KCDC Policy 002) Form completed during BC Approval process

Tick (✓) indicates yes, N/A indicates considered - but not applicable	Req'd	Supplied	Accept
copy of electrical certificate	N/A		
copy of gas certificate	↑		
an "as-built" drainage plan	↓		
plumbing (Form 125) for pressure test and installation piped services	↓		
drainage (Form 126) for stormwater installation	↓		
drainage (Form 127) for private sewer installation	N/A		
a Construction Review (PS4) statement for Structural engineering	✓	✓	✓
a Construction Review (PS4) statement for Fire engineering	N/A		
a Construction Review (PS4) statement for Geo-technical engineering	↑		
Roof cladding warranty	↑		
Solar water heating technical specifications	↑		
wall cladding/texture coating installation applicator warranty	↑		
a copy of the lift certificate	↑		
a certificate from the fire alarm installer	↑		
a certificate from the sprinkler installer	↑		
a copy of wet-area membrane installers warranty	↓		
A copy of deck membrane manufacturer's installers warranty/guarantee	↓		
FPIS (verifier) Fire Systems Certification	N/A		

KCDC CODE COMPLIANCE REVIEW CHECK SHEET

If specified supporting documentation has not been provided, what alternative information or verification method has been provided to compensate. (describe):

Compliance Schedule

Is a new or amended compliance schedule required?	Y	☒ N
If "Y" Has it been issued? (Refer note below)	Y	☒ N
If the new/ revised compliance schedule has not been issued, what action has been taken to ensure it will be issued? Outline:		

Note: If the draft/ revised compliance schedule has not been issued, the Code Compliance Certificate cannot be issued. Suspend issuing CCC until this matter is addressed.

Building consent requirements.

Are there any item specified in the building consent or PIM that have not been completed.

Outline: _____

Are there any development impact fees to be paid outstanding? Yes/ No *(if yes, CCC cannot be issued)*

Approve to issue Code Compliance Certificate if all other outstanding fees are paid. YES/NO

Building officer:

Signature: _____

Date: 2, 4, 12

Name: Tony King

Co-reviewer:

Signature: _____

Date: 12, 4, 12

Name: Russen Brook

Form 436

Retaining Walls over culvert
Road Reserve near
86 Emerald Glen Road

KLIST

BC.....

No = Plans, specifications & documentation complying with the Building Code

Yes = Plans, specifications & documentation complying with the Building Code &/or Bld. Act (The reason specified alongside each prompt)

N/A = Not Applicable to this application. (The deletion of whole sections as N/A is permitted)

For alternatives comment below each section.

Note - The further information requests are identified by hand written numbers and corresponding on FIR letters / e-mails sent

Category.....

RECEIVED

20 SEP 2010

KAPITI COAST
DISTRICT COUNCIL

00021

Site Plan (NZS 3604 Sec 3.4, 4.5 Code clB1, B2, E1, E2 F1 & F2 Sect. 72, 75 BA04)

N/A NO YES

Comments

- ☒ ☐ ☒ Wind zone High V/ High Specific..... GIS
- ☒ ☐ ☐ Flooding / ponding hazards GIS
- ☒ ☐ ☐ Slippage/ fault line hazards GIS
- ☒ ☐ ☐ Erosion, river & coastal hazards GIS
- ☒ ☐ ☐ Subsidence/ soft soil hazards Site inspection notes
- ☒ ☐ ☒ Corrosion zone Sea spray or Zone 1 GIS
- ☒ ☐ ☐ Soil contamination GIS
- ☐ ☐ ☒ Distances to boundaries / siting District plan
- ☐ ☐ ☒ Floor levels/ contours E2/AS1 & NZS 3604 E1
- ☒ ☐ ☐ Two or More Allotments Sec 75 - 83 Bld Act
- ☒ ☐ ☐ Noise / sound insulation (airport & highway)

Culvert in stream bed.

Food designates
Road width 2.7.02

Specific Design (Verified Online PS2 B1 B2 VM1)

Comments

- ☒ ☐ ☒ PS1 Design Certificate B1 B2 VM1
- ☒ ☐ ☒ PS2 Peer Review B1 B2 VM1
- ☒ ☐ ☒ Calculations VM1
- ☒ ☐ ☒ CPeng Verified Online

Mike Atkinson

Concrete / Foundation Plan (NZS 3604 NZS 4210 & 4229 E2/AS1)

Comments

Layout

- ☒ ☐ ☒ Layout dimensions NZS 3604
- ☒ ☐ ☐ Control joints NZS 3604
- ☒ ☐ ☐ Slab thickening NZS 3604, cl 7.5
- ☐ ☐ ☒ Alternative specify reason

Specify eng design B1

Section

- ☒ ☐ ☐ Depth & width NZS 3604
- ☒ ☐ ☐ Reinforcement NZS 3604; B2
- ☒ ☐ ☐ Plate fixings NZS 3604; B2 cl 7.5
- ☒ ☐ ☐ DPM NZS 3604; B2
- ☐ ☐ ☒ Engineer design and Calcs B1; B2
- ☐ ☐ ☒ Concrete strength B1; B2 cl 4.8.2
- ☐ ☐ ☒ Alternative specify reason

pre formed culvert

40 mpa @ 28 days

Timber / Subfloor Plan (NZS3604 NZS 3602 & 3640, B1,B2)

Comments

Layout

N/A NO YES

- ☐ ☐ ☐ Layout dimensions.....
☐ ☐ ☐ Bearers details.....B1 ; B2.....
☐ ☐ ☐ Joist details.....B1;B2.....
☐ ☐ ☐ Alternative.....specify reason.....

Section

- ☐ ☐ ☐ Foundation type/size..... NZS 3604;B2.....
☐ ☐ ☐ Piles post sizes & treatment NZS 3604;3602;3640;B2.....
☐ ☐ ☐ Pile c/c..... NZS 3604.....
☐ ☐ ☐ Fixing..... NZS 3604;B2.....
☐ ☐ ☐ Coatings(durability)..... NZS3604 ;B2.....
☐ ☐ ☐ Ventilation& Access to sub floor spaces NZS 3604.....
☐ ☐ ☐ Alternative.....specify reason

Culvert - Prefabricated

If the addition is an upper storey show details on upgrading existing foundations, joints, piles, etc (may require Engineer design)

Floor plans / layout (B1, B2, C1-4, D1, E3, F7, G1, G2, G12 G3, G9, G11, G13)

Comments

General

N/A NO YES

- ☐ ☐ ☐ Room dimensions.....
☐ ☐ ☐ Sectional lines.....
☐ ☐ ☐ Location of partitions.....
☐ ☐ ☐ All designated spaces.....
☐ ☐ ☐ Lintels..... NZS 3604;B2.....
☐ ☐ ☐ Beams..... NZS 3604, B2 by engineer.....
☐ ☐ ☐ Point loads by engineer or pre-naller.....B1 B2.....
☐ ☐ ☐ Smoke detectors..... F7.....
☐ ☐ ☐ Wet areas / WCs D1, Table 2; B2.....
☐ ☐ ☐ Kitchen.....G3, G9,G11.....
☐ ☐ ☐ Alternative.....specify reason

Culvert - Prefabricated

Changes to existing work requires both an existing floor plan & a proposed floor plans
 Facility provided for washing utensils / food G3, Section 1.1 & Figure 1:
 Preparation Area and Access to Facility Paragraphs 1.1.2 and 1.5

Wall / Framing B1, B2, E3

Comment

General

- ☐ ☐ ☐ Identify engineering / Structural items/ portals.B1 B2.....
☐ ☐ ☐ By pre-naller or to NZS 3604.....B1 B2.....
☐ ☐ ☐ Fixing types NZS 3604.....B1 B2.....
☐ ☐ ☐ Stud c/c as per wind zone.... NZS 3604.....B1 B2.....
☐ ☐ ☐ Building wrap..... NZS 3604.....
☐ ☐ ☐ Stud heights of rooms..... NZS 3604.....B1 B2.....
☐ ☐ ☐ Alternative.....specify reason

Roof..... NZS 3604,3602 &3640 B1B2E2 E2/AS1

Trussed / by preaniler or Framed roof NZS 3604

Comments

N/A NO YES

- ☐ ☐ ☐ Fixing layout & details.....B1 B2
- ☐ ☐ ☐ Correct layout.....B1
- ☐ ☐ ☐ Max span (12m or Engineer).....B1
- ☐ ☐ ☐ Purlin or batten c/c.....B1 B2
- ☐ ☐ ☐ Rafters / Beams.....B1 B2
- ☐ ☐ ☐ Alternative.....specify reason

Roof cladding - Metal ; Profiled Metal; Masonry, Membranes Code E2, E2/AS1 B2

- ☐ ☐ ☐ Pitch / profile.....E2
- ☐ ☐ ☐ Underlay.....E2
- ☐ ☐ ☐ Flashings.....E3;E2.....B1 B2
- ☐ ☐ ☐ Parapets.....B1 E2 B2
- ☐ ☐ ☐ Membrane Roofs and substrate Code E2/AS1 B2
- ☐ ☐ ☐ Gutters, Barges & Fascias detail Refer E2, section 8.3, of 8.3.9
- ☐ ☐ ☐ Alternative.....specify reason

Installation of profiled metal roof cladding shall be by trained installers, approved by the tile manufacturer or NZ agent. In some cases experienced builders undertake installation, refer E2, section 8.4, cl 8.4.2 'General'.

Claddings CodeB1 & B2 E2/AS1, Branz good practice Guides & Weather tightness Vol 12&3

Comments

- ☐ ☐ ☐ Risk matrix Cavity or direct.....E2
- ☐ ☐ ☐ Brick Veneer.....E2 "The brick book" Branz Masonry Guide ; B2
- ☐ ☐ ☐ Timber WeatherboardE2 Branz Timber Guide Vol 1&2; B2
- ☐ ☐ ☐ Stucco.....B1 NZS 3604 Branz good practice guide; B2
- ☐ ☐ ☐ Profiled Metal.....E2 Metal Roofing Practice; B2
- ☐ ☐ ☐ PlywoodE2 ; B2
- ☐ ☐ ☐ Fiber Cement W/b.....E2; B2
- ☐ ☐ ☐ Board & Batten..... Branz good practice guide; B2
- ☐ ☐ ☐ Fiber Cement Sheet.....E2; B2
- ☐ ☐ ☐ Weathertightness details.....E2 ; B2
- ☐ ☐ ☐ Alternative.....specify reason

Bracing NZS 3604 B1, B2, B3

Comments

N/A NO YES

- ☐ ☐ ☐ Subfloor bracing.....B1& NZS 3604
- ☐ ☐ ☐ Wall bracing..... B1 & NZS 3604
- ☐ ☐ ☐ Roof bracing..... B1& NZS 3604
- ☐ ☐ ☐ Bracing calculations..... B1 & NZS 3604
- ☐ ☐ ☐ Provide bracing plan layout..... B1 & NZS 3604
- ☐ ☐ ☐ Double top plate (if > 5m)..... B1 & NZS 3604
- ☐ ☐ ☐ Indicate dragon tie (if > 6m)..... B1& NZS 3604
- ☐ ☐ ☐ Diaphragm ceilings (if > 7.5m)..... B1 & NZS 3604
- ☐ ☐ ☐ Check for distribution B1 & NZS 3604
- ☐ ☐ ☐ Alternative.....specify reason

Light & Ventilation B1 G4, Clause 1.2; G4, clause 1.3 G7, G9 F2

General

Comments

N/A NO YES

- ☐ ☐ ☐ Vent 5%.....G4
- ☐ ☐ ☐ Light 10%.....G7
- ☐ ☐ ☐ Electrical.....G9
- ☐ ☐ ☐ Mechanical ventilation.....G4
- ☐ ☐ ☐ Exterior space for awareness.....G7

Glazing

- ☐ ☐ ☐ Safety glass.....F2
- ☐ ☐ ☐ Wind zone.....B1, NZS 3604
- ☐ ☐ ☐ Alternative.....specify reason

Insulation BRANZ Home Insulation Guide, NZS 4218, AS/NZS 2918

Comments

- ☐ ☐ ☐ Floor.....H1
- ☐ ☐ ☐ Wall.....H1
- ☐ ☐ ☐ Roof.....H1
- ☐ ☐ ☐ Windows / glazing.....H1
- ☐ ☐ ☐ Alternative.....specify reason

Stairs / Landings / Handrails D1, sec 4

Comments

- ☐ ☐ ☐ Stair tread & risers.....D1
- ☐ ☐ ☐ Landing size.....D1
- ☐ ☐ ☐ Handrail.....D1
- ☐ ☐ ☐ Riser opening (min) where children.....D1
- ☐ ☐ ☐ Residential Accessibility.....D1
- ☐ ☐ ☐ Alternative.....specify reason

Stair type

Minor Private: Max rise 220mm/Min tread 220mm,

Secondary Private: Max rise 200mm/Min tread 250mm,

Main Private: Max rise 190mm/Min tread 280mm. (Main private includes exterior stairs)

Decks / Balconies / Barriers D1, F4 & NZS 3604, Section 6 & 7,

Comments

- ☐ ☐ ☒ Barrier & deck fixing durability.....D1 B1 B2
- ☐ ☐ ☒ Top rails, balusters, bottom rails, palling.....D1 B1
- ☐ ☐ ☒ If Engineer then calculations required.....B1 B2
- ☐ ☐ ☒ Max 3m height or Engineer.....B1 B2
- ☐ ☐ ☐ Deck joist sizes, grade, treatment, span & spacing.....D1 B1 B2
- ☐ ☐ ☐ Piles & bearers the correct grade, treatment, size, spacing & span.....D1 B1 B2
- ☐ ☐ ☐ Slip resistance.....D1
- ☐ ☐ ☐ Appropriate waterproofing membrane.....B1 B2
- ☐ ☐ ☐ Stringer size & connections.....D1 B1 B2
- ☐ ☐ ☐ Thresholds.....D1 E2
- ☐ ☐ ☐ Alternative.....specify reason

N/A NO YES

- ☐ ☐ ☐ Travel distances max 24m.....C3.15
☐ ☐ ☐ Smoke detectors.....F7
☐ ☐ ☐ Wood burnersF7
☐ ☐ ☐ Garage fire wall detailsC7 (C7.10 C7.8)
☐ ☐ ☐ Alternative.....specify reason

Swimming Pool & Fencing *Fencing of Swimming Pools Act 1987**

Comments

- ☐ ☐ ☐ Refer to Compliance for sign off
☐ ☐ ☐ Specific Engineering Design
☐ ☐ ☐ Proprietary system/design
☐ ☐ ☐ Section/construction
☐ ☐ ☐ Pool backwash discharging the sewer
☐ ☐ ☐ Fence height
☐ ☐ ☐ Gate(s) / door(s) open outward

Ambiguity / Contradictions

Comments

- ☐ ☐ ☐ Manufacturers Specs
☐ ☐ ☐ Engineer compared with Designer/Architect
☐ ☐ ☐ Any Other contradictions

Alternative Solutions

Record assessment, decision and reasons for decisions below. Has the designer substantiated why they believe the proposal complies with the building code? Is there a determination on a similar proposal? Has corroborative evidence been provided (peer review, appraisal or determination)? Include reference to comparison with acceptable solutions, other documents / standards / best practice guides / publications, expert opinion, determinations, in-service history, product certification, professional judgment, building code objectives. Refer BRANZ Bulletin # 456 (Dec 2004). List any conditions / communications / actions:

Opus consultants is a specialist with NZTA. Ewald Allen extension to road joining at McKay's crossing to South. Hume pre-formed concrete culvert section with earth embankments with road over. Specific design by Alexei Murachov. OPEN. Calcs supplied & relevant.

This sheet relevant to plumbing & drainage only

General

Comments

N/A NO YES

- ☒ ☐ ☐ Kitchen area.....G3/AS1
- ☒ ☐ ☐ Laundry.....
- ☒ ☐ ☐ Bathrooms.....
- ☒ ☐ ☐ Location of sanitary fixtures.....
- ☒ ☐ ☐ Plumbing layout.....

Plumber's Reg no.....

Drainlayers Reg no.....

Effluent Disposal Systems

Comments

- ☒ ☐ ☐ Engineering Site Investigation
- ☒ ☐ ☐ Design Specification & Producer Statement

(The design criteria should include; Capacity based on bedroom numbers, Loading calculation, site evaluation, soil profile, site characteristics.)

Water Supplies

Comments

- ☒ ☐ ☐ G 12
- ☒ ☐ ☐ Town supply.....
- ☒ ☐ ☐ Private scheme.....
- ☒ ☐ ☐ Type & location.....
- ☒ ☐ ☐ Alternative.....specify reason.....

Sanitary Drainage

Comments

- ☒ ☐ ☐ G13.....
- ☒ ☐ ☐ Layout
- ☒ ☐ ☐ Council availability or
- ☒ ☐ ☐ Septic tank design & calcs.....
- ☒ ☐ ☐ Alternative.....specify reason.....

Stormwater Drainage

Comments

- ☐ ☐ ☒ E1 Existing stream bed.
- ☐ ☐ ☒ Layout
- ☒ ☐ ☐ Council availability or
- ☐ ☐ ☒ Alternative.....specify reason Open - Emerald Glen flow regimes report

Jack McConchuz, Principal Water Resource Scientist - DWR. Feb. 2016

Sanitary Plumbing

Comments

- ☒ ☐ ☐ G13
- ☒ ☐ ☐ Layout
- ☒ ☐ ☐ Alternative.....specify reason

Accessibility D1; NZS 4121

Comments

N/A NO YES

- ☐ ☐ ☐ Car parks
- ☐ ☐ ☐ Projections
- ☐ ☐ ☐ Kerb Ramps
- ☐ ☐ ☐ Footpaths/Ramps / Landings
- ☐ ☐ ☐ Entrances
- ☐ ☐ ☐ Public Facilities
- ☐ ☐ ☐ Lifts
- ☐ ☐ ☐ Stairs
- ☐ ☐ ☐ Stair / handrails
- ☐ ☐ ☐ Doorways/Doors/Corridors/ Lobbies
- ☐ ☐ ☐ Controls
- ☐ ☐ ☐ Places of assembly/entertainment/Recreation
- ☐ ☐ ☐ Warning Systems
- ☐ ☐ ☐ Toilet Facilities / Laundering
- ☐ ☐ ☐ Food preparation / accommodation
- ☐ ☐ ☒ Signs

Fire report C1,C2, C3, C4

Comments

- ☐ ☐ ☐ Purpose group FHC
- ☐ ☐ ☐ Fire cell separation /use of building
- ☐ ☐ ☐ f- rating
- ☐ ☐ ☐ s- rating
- ☐ ☐ ☐ Occupancy numbers
- ☐ ☐ ☐ Means of escape
- ☐ ☐ ☐ Travel distances
- ☐ ☐ ☐ External walls/unprotected areas
- ☐ ☐ ☐ Fire resistance to supporting elements/floors/ walls
- ☐ ☐ ☐ Risks to/below escape routes
- ☐ ☐ ☐ Fire safety precautions
- ☐ ☐ ☐ SFI's / SDI's
- ☐ ☐ ☐ Alternatives
- ☐ ☐ ☐ Referral to DRU
- ☐ ☐ ☐ Alternative.....specify reason

Solid waste G15

Comments

- ☐ ☐ ☐ Waste bin area
- ☐ ☐ ☐ Environmental health referral
- ☐ ☐ ☐ Alternative.....specify reason

Hazardous Agents, Materials & substances F1 - F3

Comments

- ☐ ☐ ☐ Waste bin
- ☐ ☐ ☐ Environmental health referral

OPA. NETA.
 Reformed culvert - Humes.
 Earth boring - Terrameth. & Gabor Borer.

General stamp prompts

Wind Zone
 Corrosion
 Cladding
 Electrical
 Bracing Highlighted
 Boundary setting out
 Engineers design

Set Status
 Complete checklist
 Addenda
 Section 72 / ponding
 Set inspections

Dialogue Record

List further information requested, info received, records / reference of communication etc. via Phone / Email / letter / counter / fax

[illegible]

Change Of Use – Sec 114: (record decisions and basis for decisions – confirm whether sec 115 applies)		
Date	N/A – New road section	
Alteration to Existing Building – Sec 112: (record decisions and basis for decisions – confirm whether sec 118 applies and if so, has it been complied with?). List conditions / communications / actions:		
Date	N/A	
Hazards – Sec 71-74 : (record decisions and basis for decisions – see under "site plan above already done")		
Date:	N/A	
Compliance Schedule – Sec 100 - 107: (record if new / amended compliance schedule required, and attach copy of replica compliance schedule. Ensure that note is recorded on building consent that new / amended compliance schedule will be issued).		
Date	N/A – New road section – No specified yrs	

<i>Note :- Refer to the associate code clause along side each prompt above for compliance or reason of decision</i> Plan checked yet Unsatisfactory (request for further information- see attached copy)	I am satisfied on reasonable grounds that the provisions of the Building Code are met and the Building consent may be granted (refer Bld Act Sec 49(1)) All previous Further Information Requests now satisfactory.
Signature, name & date Bldg..... Plumbing..... Fire.....	Signature, name & date Bldg..... SAGE 5/10/10 (for all pages except those listed below) Plumbing..... N/A (For pages 6 of 11 unless otherwise listed) Fire..... N/A (As listed on page 7 of 11)

Addenda to Form 436

Allocation of work to approvals officer outside current competency

Building Category	Circle appropriate category	0	1	2	3

1) Authorized by Team leader or delegate	Level of supervision reqd	Reason for decision
Name, Signature & date	Coaching Mentoring Peer review on completion	In training Vetting one level above competency Other
2) Approval Officer undertaking training	Record of Learning	Decision
Name, Signature & date		
Current competency level Category 0,1 or 2		I am satisfied on reasonable grounds that the provisions of the Building Code are met and the building consent may be granted (refer Bld Act 49(1))
3) Coach/Mentor/Peer reviewer	Training needs identified	Comments
Name, Signature & date		
Current competency level Category 1,2 or 3		I am satisfied on reasonable grounds that the provisions of the Building Code are met and the building consent may be granted (refer Bld Act 49(1))

Allocation notes

- Categories of building are determined on receipt of application
- All applications are re assessed for the category and allocated to each appropriate Officer at the earliest opportunity and then placed in processing shelves accordingly
- Approvals staff are aware of their own competency level and check the allocation of work.
- Approvals staff may select, for training purpose, a consent 1 (one) category higher than their current competency for the purpose of increasing their skill level provided they gain approval from the Team Leader Building Consent Approvals. Once approval is given the Team Leader Building Consent Approvals will arrange for the appropriate supervision and final peer review.

FOR COUNCIL USE:

DEPARTMENT	TICK FOR REFERRAL	COMMENTS	APPROVING OFFICER/ DATE
Site Inspection	✓	Refer to Form 241 for completion of site inspection & sign off this form when completed.	
Resource Consents (refer PIM Sheet)	✓		
Plumbing and Drainage	✓	N/A.	
Building	✓	Approved - Specific design by C/Egg.	SACU 5/10/10
Fire Design			
Compliance Schedule			
Environmental Health			
Other.....			
NOTES:			



Building Consent

Building Project

Location (Site Address)

Building Category

Circle appropriate category

0 1 2 3

Application workflow checklist

RECEIVED 20 SEP 2010 KAPITI COAST DISTRICT COUNCIL	Name & Signature Note # A signature confirms that the application contains sufficient information to enable compliance with the building code to be assessed	Date
# Note: Checked, date stamped and initialled by TO/BCO. Select and circle appropriate category above & identify allocation to appropriate technical groups and if required to go to "site inspection" AP-WI-001	JP Palmer	20/9/10
Payment taken and receipted (details entered inside this cover sheet) by CSO CS-WI-008	Angie Sikes	20/9/10
Data entered and invoiced by CSO CS-WI-008	Angie Sikes	20/09/10
Pages numbered by CSO CS-WI-008	JP Palmer	20/9/10
BC Application and documentation scanned, checked & status update by CSO CS-WI-009	Angie	20/9/10
BC Application signed over to BC CS-WI-009	Angie	20/9/10
Application reviewed by Technical Officers who sign off at back of this form AP-WI-026 & AP-WI-009	JP Palmer SACU	5/10/10
Once Technical review complete, passed to CS for Plan Production and Invoicing AP-WI-009		
CSO checks data entry input, including required inspections CS-WI-012		
BC, plans and other appropriate paperwork produced by CSO CS-WI-012	} Yingjun	6/10/10
BC, Approved plans and appropriate paperwork sent to applicant or agent with receipt by CSO CS-WI-012		
Inspection envelope prepared by CSO with endorsed plans and other appropriate paperwork and filed to cabinet. CS-WI-012		

CONSENT DATA INPUT REQUIREMENTS

REQUIRED INSPECTIONS					
✓ Foundation/Piles	32	✓	Lapse and cancellation after 12mths of consent	23	Installation of solid fuel appliance (wood burner) comply and secured
Block	2	✓	Engineer design specific components	23A	Second hand wood burner certified
Pre-Slab (Plumbing)	3		Foundations to solid bearing of 200mm	25	Contractor to leave site clean and clear
Pre-Slab (Building)	4		NZS3604 as acceptable solution in terms of NZ Building Code	30	To comply with Food Hygiene Regulations 1974
Sub-Floor	6		Not approved as Habitable Room	31	As in CCZ, all fixings need to be stainless steel
Drainage	9		Note and comply with all endorsements on approved plans	33	Approval only given for relocation purposes
Pre-Wrap	10		Names and Registration of plumber and drain-layer required	34	If cladding to have pre-finished colour coating, to be appropriate for coastal conditions
Pre-Clad	11		Concealed soil and TV pipes and gas flues only allowed in ducts or minimum framed	38	Window sashes on upper levels less than 760mm above floor to have stays fixed
Weather-tightness	12		Hot & Cold water pipes tested to 1500kPa	39	Construction of playground to comply with NZS5828
Pre-lining (Building)	13		Roof stormwater to be connected to existing	40	Fencing, gates, doors directly leading to swimming pool to comply with Pool Act 1987
Pre-lining (Plumbing)	14		Copper water pipes not permitted	43	Textured systems should be applied by licensed, trained applicators – supply warranty
Wet areas - membrane	15		Owner/builder to ensure prop foundation height will allow satisfactory connection	44	Exterior cladding system to be completed and inspected prior to internal lining
Post Lining	21		Minimum FFL to comply with NZS3604		
Brick Veneer					
Retaining Wall					
Final (Dwelling)					
✓ Final (Commercial & Multi-resd)					

STANDARD CONSENT CONDITIONS	
37	
72	
80	

NON-STANDARD ADDENDA

N/A

FOR OFFICE USE ONLY

FEES		APPLICATION FEE \$.....	
Building Fee	\$718.00	\$1350.00	
Sewer Connection	\$.....	RECEIPT NO	DATE
Water Connection	\$.....		
Vehicle Crossing	\$.....	INVOICE NO	DATE
Damage Deposit	\$.....	9937	20-09-10
Code Compliance Administration	\$.....	RECEIPT NO	DATE
Building Research Levy	\$200.00	356771	20-09-10
Development Levy	\$.....	FEES PAID BY:	
DBH Levy	\$394.00	Opus Int Consultants	
Compliance Schedule	\$38.00		
Digital Storage Fee	\$.....	BUILDING CONSENT NUMBER	DATE ISSUED
Certificate of Title Charge	\$.....	100764	
Photocopying	\$.....	AUTHORISATION FOR REFUND OF DEPOSIT	
SUBTOTAL	\$.....	Deposit held =	\$.....
Less Fee Paid	\$.....	Cost to Repair Damage Caused During Construction =	\$.....
TOTAL	\$1350.00	Total Balance to be Refunded	
GST INCLUDED	\$.....	Authority Number:	
		Authorised:	DATE:

GST 51-860-608

Received with thanks by 36/01
KAPITI COAST DISTRICT COUNCIL

20-09-10 12:35 Receipt no.356771

COPY *COPY* *COPY*
DR BC100764 OPUS 1,350.00-
Opus:Opus Int : Majestic Centre:100 Wi
CQ B
Opus Int consultants 1,350.00



Kāpiti Coast District Council

Private Bag 60 601
Paraparaumu 5254
New Zealand

Tel (04) 296 4700
or 0800 486 486

Fax (04) 296 4830 ■
kapiti.council@kapiticoast.govt.nz ■
www.kapiticoast.govt.nz ■

Building consent 100764

Section 51, Building Act 2004

The building

Street address of building: Near building on 86 Emerald Glen Road Paekakariki

Legal description of land where building is located:

Valuation number:

Building name:

Location of building within site/block number:

Level/unit number:

The owner

Name of owner: KCDC Road Reserve

Contact person:

Mailing address: 175 Rimu Road, Private Bag 60 601, Paraparaumu 5254

Street address/registered office:

Phone number: Landline: 2964700

Mobile:

Daytime:

After hours:

Facsimile number: 042964830

Email address:

Website:

First point of contact for communications with the council/building consent authority:

Full Name: Opus

Mailing Address: Opus Int : Majestic Centre, 100 Willis Street, PO Box 1200, Wellington

Phones: 04 4717000

Fax: 04 499 3699

Email: richard.coles@opus.co.nz

Building work

The following building work is authorised by this building consent:

Retaining walls(over culvert) to support a new road. Road to connect existing Emerald Glen Road to Mackays Crossing roundabout to provide access once water fall road level closes.

Currently there is a stream, after construction there will be a road over stream.

This building consent is issued under section 51 of the Building Act 2004. This building consent does not relieve the owner of the building (or proposed building) of any duty or responsibility under any other Act relating to or affecting the building (or proposed building).

This building consent also does not permit the construction, alteration, demolition, or removal of the building (or proposed building) if that construction, alteration, demolition, or removal would be in breach of any other Act.

Compliance schedule

A compliance schedule is not required for the building.

Attachments

Copies of the following documents are attached to this building consent:

Signature 


Position

On behalf of: KAPITI COAST DISTRICT COUNCIL

Date: 5/10/10

7 October 2010

KCDC Road Reserve
175 Rimu Road
Private Bag 60 601
Paraparaumu 5254

Dear Sir/Madam

Approval of Building Consent: 100764

Description: Retaining walls(over culvert) to support a new road. Road to connect existing Emerald Glen Road to Mackays Crossing roundabout to provide access once water fall road level closes.

Location: Near building on 86 Emerald Glen Road Paekakariki

Legal Description:

I am pleased to inform you that the above building consent application has been approved. The building consent documents have been issued and are enclosed.

Before starting any work please read ALL the building consent documents carefully and in full so that you as the owner understand the conditions under which the consent has been issued.

As the owner you are also responsible for the following:

- ensuring that building work starts within 12 months of the approval of the building consent and that work progresses at a reasonable pace.
- ensuring that all required inspections are completed and that a Code Compliance Certificate is obtained once the work has been completed – please be aware that before a Code Compliance Certificate is issued, the building work must comply with the Building Act 2004 and the Building Code as at the date of final inspection.
- that no work with noisy equipment is to be carried out before 7.30 a.m. or after 6 p.m. Monday to Saturday. During the week there may be workers on site from 6.30 a.m. quietly preparing for work. Work with noisy equipment is not permitted on Sundays and public holidays.
- ensuring that no damage occurs to public property, for example footpaths or retaining structures. Should damage occur you will be required to pay for any repairs.
- ensuring that if you wish to make any changes to the approved building consent, then an amendment to the building consent will need to be lodged with us.

If you have any questions regarding your building consent or your responsibilities, please contact Kapiti Coast District Council on (04) 9045 617.

Addenda to Application

Application

Opus	No.	100764
Opus Int : Majestic Centre 100 Willis Street PO Box 1200		

Project

Description	Retaining Walls Being Stage 1 of an intended 1 Stages Retaining walls(over culvert) to support a new road. Road to connect existing Emerald Glen Road to Mackays Crossing
Intended Life	Indefinite, but not less than 50 years
Intended Use	Currently there is a stream, after construction there will be a road over stream.
Estimated Value	\$200,000
Location	Near building on 86 Emerald Glen Road Paekakariki
Legal Description	
Valuation No.	

Designer: R Coles Opus 100 Willis Street Wellington : 044993699
Contractor: Fletcher c/o Gerald Carty Level 1, 816 Great South Rd
Penrose 1142 : 0276711870

General Requirements

- a) The Building Inspector is to be given 24 hours' notice before carrying out the following inspections:
 1. Foundations
 2. Final Inspection - Commercial
- b) To arrange for any of the above required inspections by Building Control Team, please contact the Call Centre on (04) 296 4700 between the hours of 8am and 5pm.

1. LAPSE AND CANCELLATION OF BUILDING CONSENT. A building consent shall lapse and be of no effect if the building work concerned has not been commenced within 12 calendar months after the date of issue. Or within such further period as the territorial authority allows.
2. The inspection of all specific design components of the building is the responsibility of the Design Engineer who is also required to supply a written report to Council.

I have read the foregoing addenda to application, and the Owner/Builder/Authorised Person hereby agrees to comply with them.

OWNER/BUILDER/
AUTHORISED PERSON

Date: _____

CONSTRUCTION

1. DRAWING INDEX

101. PLAN AND LONGITUDINAL SECTION - SHEET 1
102. PLAN AND LONGITUDINAL SECTION - SHEET 2
111. PLAN LAYOUT - Sta. 60 to Sta. 260
112. PLAN LAYOUT - Sta. 260 to Sta. 450
113. PLAN LAYOUT - Sta. 450 to Sta. 640
114. PLAN LAYOUT - Sta. 640 to Sta. 830
115. PLAN LAYOUT - Sta. 830 to Sta. 1020
116. PLAN LAYOUT - Sta. 1020 to End
121. TYPICAL CROSS SECTIONS
122. MISCELLANEOUS DETAILS
131. CLOSURE OF S.H. 1 MEDIAN BARRIER BREAK

301.	EMERALD GLEN CULVERT - PLAN
302.	EMERALD GLEN CULVERT - SECTION & FISH PASS DETAILS
303.	EMERALD GLEN CULVERT - TERRAMESH EARTH WALL DETAILS
304.	EMERALD GLEN CULVERT - END ELEVATIONS - TERRAMESH WALL DETAILS
305.	EMERALD GLEN CULVERT - HEADWALL DETAIL
311.	TRAMWAY AND MINOR CULVERT DETAILS
312.	MANHOLE AND WINGWALL ARRANGEMENTS
313.	SCRUFFY DOME INSTALLATION
321.	TYPICAL SWALE DETAIL AND EROSION PROTECTION FOR BRIDGE
322.	DIVERSION CHANNEL DETAILS
331.	ROAD DRAINAGE DETAILS - SHEET 1
331.	ROAD DRAINAGE DETAILS - SHEET 2

401. TYPICAL CROSS SECTION & DESIGN PRINCIPLES
404. PLANTING PLAN 1
405. PLANTING PLAN 2

500. GENERAL ARRANGEMENT
501. ABUTMENT GEOMETRY
502. ABUTMENT A REINFORCEMENT
503. ABUTMENT B REINFORCEMENT
504. ABUTMENT & PILE REINFORCEMENT DETAILS
505. 14m SPAN DHC BEAMS - REINFORCEMENT & STRESSING DETAILS
506. 14m SPAN DHC BEAMS - OUTER BEAM UNIT TYPE 2
507. 14m SPAN DHC BEAMS - UNIT DATA & LAYOUT PLAN
508. DECK PLAN & GUARDRAIL LAYOUT
509. W-SECTION GUARDRAIL - ASSEMBLY & FIXING DETAILS
510. ALUMINIUM HANDRAIL - ASSEMBLY & FIXING DETAILS
511. ALUMINIUM HANDRAIL - FABRICATION DETAILS
512. CONCRETE NOTES



LOCALITY PLAN
N.T.S.

[illegible]

Wellington Office
P.O. Box 12-000
Wellington 6144, New Zealand
+ 64 4 471 7200

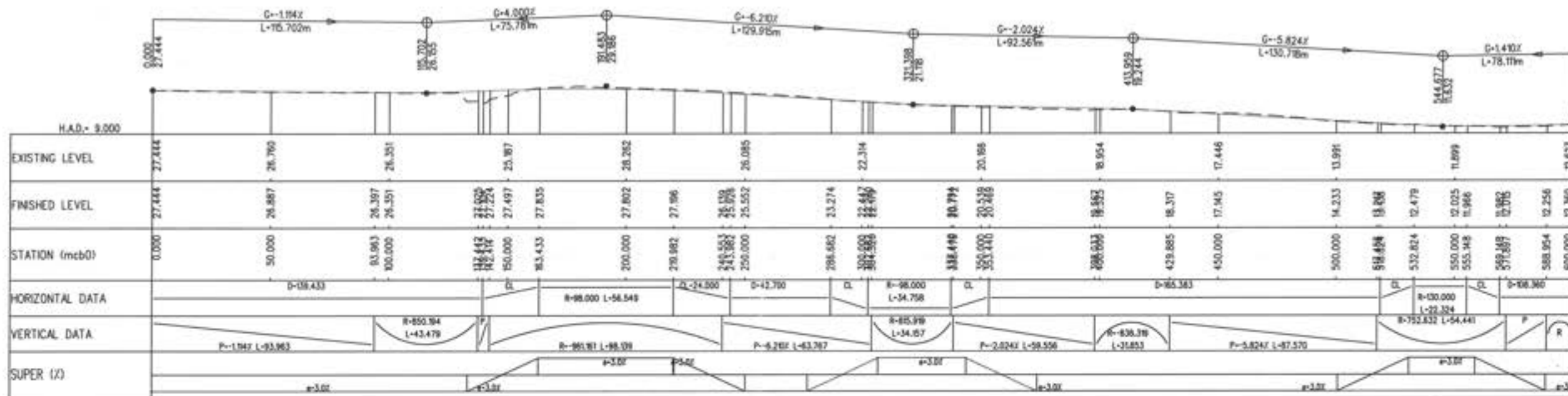
NEW ZEALAND TRANSPORT AGENCY
EMERALD GLEN CONNECTION, KAPITI
ROAD ALIGNMENT

LOCALITY PLAN & DRAWING INDEX

Class	Group	Activity	Session Date
R.O	IT	CLIE	02/03/2019

Page No.	Sub
SC1744.02	N.T.S

Drawing No.	Sheet No.	Revision
5/2573/1/7504	1	R2



LONGITUDINAL SECTION
HORIZONTAL SCALE 1:1000
VERTICAL SCALE 1:1000

KEY

- POSSIBLE ARCHAEOLOGICAL SITE
- PROPOSED FOOTPATH LOCATION
- SWALES
- STOCK PROOF FENCE
- PROPOSED STOCK CROSSINGS
- CULVERTS



Rev	Description	By	Date
01	ISSUED FOR CONSTRUCTION	C.L.	02/09/10



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NEW ZEALAND TRANSPORT AGENCY
EMERALD GLEN LINK ROAD, KAPITI
ROAD ALIGNMENT

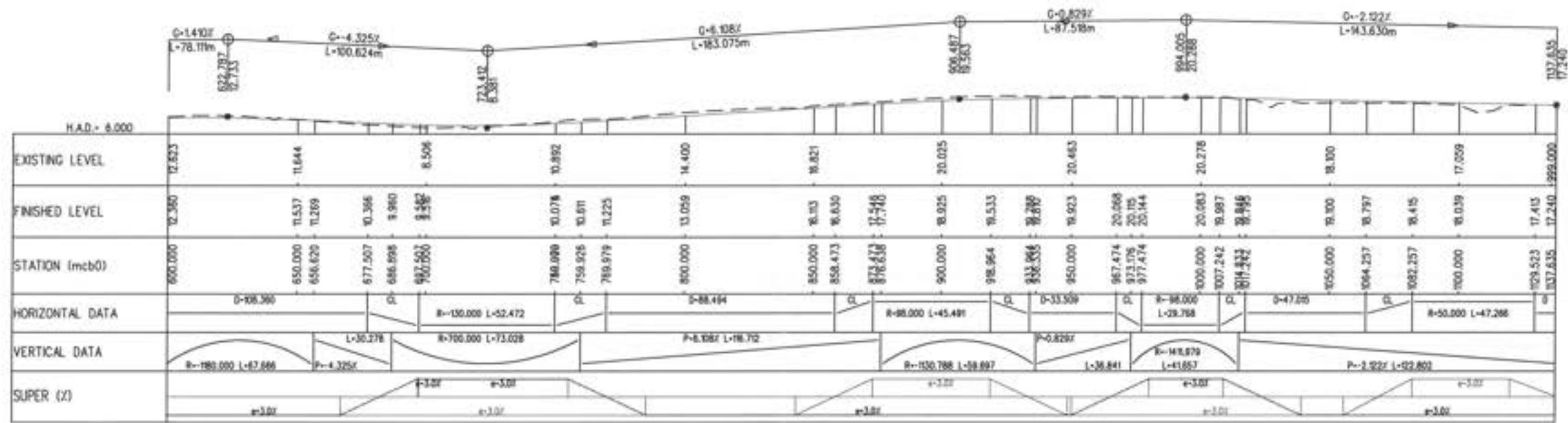
PRELIMINARY ALIGNMENT - FOR CONSTRUCTION
SHEET 1 OF 2

Project: 5-C1744.01 Scale: 1:1000 (A1)

Sheet: 5 / 2573 / 1 / 7504 Plot No: 101 Drawn: R2

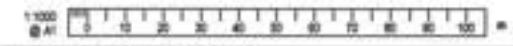
CONSTRUCTION

RECEIVED
20 SEP 2010
KAPITI COAST
DISTRICT COUNCIL



LONGITUDINAL SECTION
HORIZONTAL SCALE 1:1000
VERTICAL SCALE 1:1000

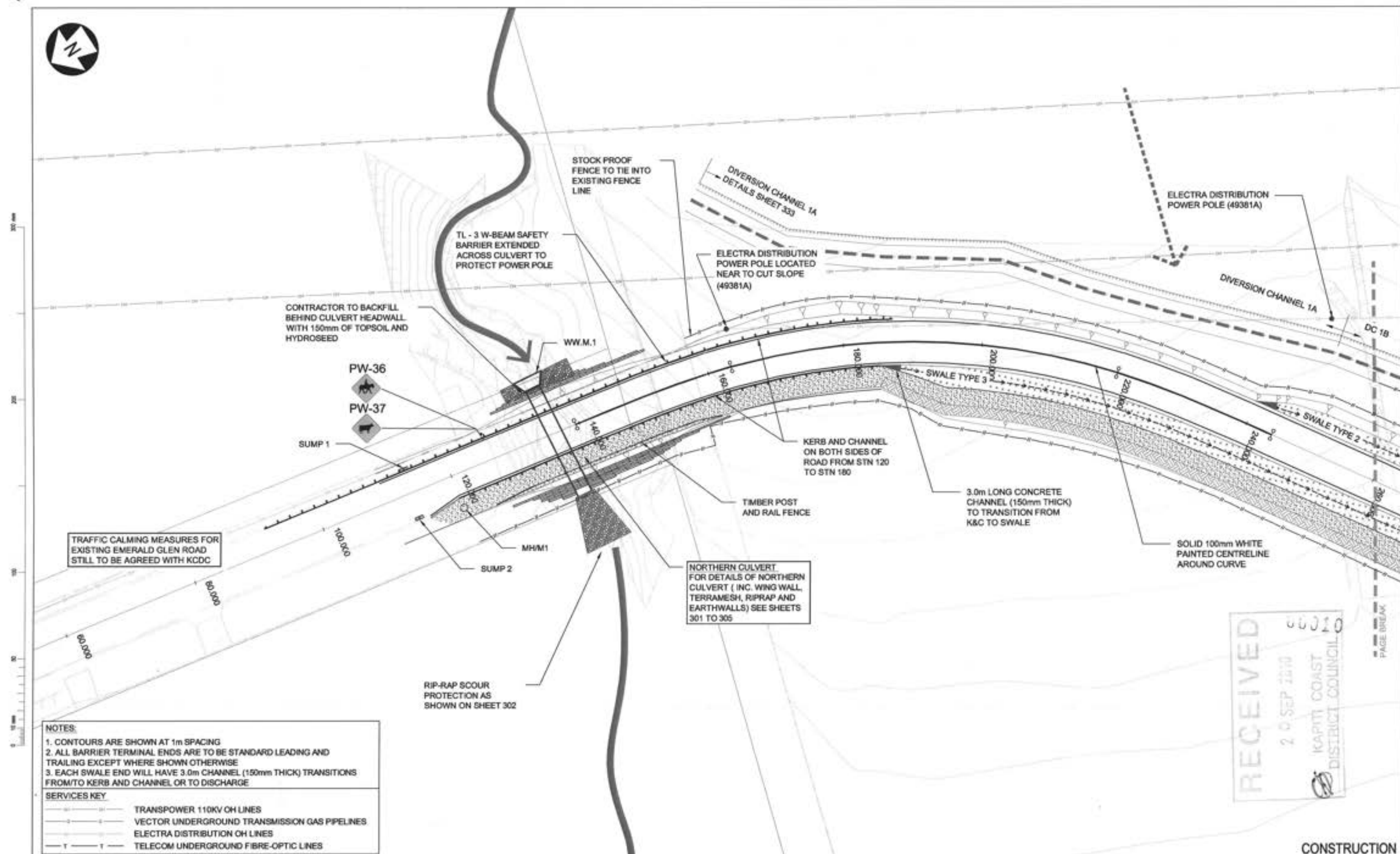
- KEY**
- POSSIBLE ARCHAEOLOGICAL SITE
 - PROPOSED FOOTPATH LOCATION
 - SWALES
 - STOCK PROOF FENCE
 - PROPOSED STOCK CROSSINGS
 - CULVERTS



RECEIVED
25 SEP 2010
KAPITI COAST
DISTRICT COUNCIL

CONSTRUCTION

Project: Emerald Glen Link Road, Kapiti Issue: 1.0 Scale: 1:1000 Author: [Name] Check: [Name] Drawn: [Name] Reviewed: [Name]	OPUS Wellington Office P.O. Box 12-000 Wellington 6144, New Zealand +64 4 471 7000	NEW ZEALAND TRANSPORT AGENCY EMERALD GLEN LINK ROAD, KAPITI ROAD ALIGNMENT PRELIMINARY ALIGNMENT - FOR CONSTRUCTION SHEET 2 OF 2 5-C1744.01 1:1000 (A1) 5 / 2573 / 1 / 7504 102 R2
---	---	---



NOTES:

1. CONTOURS ARE SHOWN AT 1m SPACING
2. ALL BARRIER TERMINAL ENDS ARE TO BE STANDARD LEADING AND TRAILING EXCEPT WHERE SHOWN OTHERWISE
3. EACH SWALE END WILL HAVE 3.0m CHANNEL (150mm THICK) TRANSITIONS FROM TO KERB AND CHANNEL OR TO DISCHARGE

SERVICES KEY

- T — T — TRANSPOWER 110KV OH LINES
- V — V — VECTOR UNDERGROUND TRANSMISSION GAS PIPELINES
- E — E — ELECTRA DISTRIBUTION OH LINES
- F — F — TELECOM UNDERGROUND FIBRE-OPTIC LINES

LEGEND

MINOR DRAINAGE CHANNEL	GRASSED BRIDLEWAY	STOCK CROSSING
MINOR STREAM	PROPOSED ROAD ALIGNMENT	SIGHT RAILS
SIGNIFICANT STREAM	EXISTING ROAD SIGNS & LIGHTING	MANHOLE
DIVERSION CHANNEL	NEW ROAD SIGNS & LIGHTING	SCRUFFY DOME MH
SWALES	TIMBER POST & RAIL FENCE	WINGWALL
FOOTPATH / BRIDLEWAY	STOCK PROOF FENCING	CULVERT

NO.	REVISION	DATE	BY	CHKD
1	ISSUED FOR CONSTRUCTION	02/05/2010		



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+64 4 471 7000

NO.	REVISION	DATE	BY	CHKD
1	ISSUED FOR CONSTRUCTION	02/05/2010		

NEW ZEALAND TRANSPORT AGENCY
EMERALD GLEN CONNECTION, KAPITI
ROAD ALIGNMENT

PLAN LAYOUT
Sta 60 to Sta 260

5/2573/1/7504

CONSTRUCTION

111 R2





RIP RAP SCOUR
PROTECTION AS SHOWN ON
SHEET 102

FOR DETAILS OF NORTHERN CULVERT
(INC. WING WALL, TERRACE, RIPRAP
AND EARTH WALLS) SEE SHEETS
300.311, 312 AND 313

225 DIA. HDPE STORMWATER PIPE

300 DIA. HDPE STORMWATER PIPE

WWM1

WWM2

1757438m
346336mm
(ED D/B 1)

SUMP 2

SUMP 1

120.000

140.000

1757438m
346336mm

WWM1



CONSTRUCTION

Revised	Description	Author	Check
R1	ISSUED FOR INFO	C.L.	MDP
R1	ISSUED FOR CONSTRUCTION	C.L.	MDP



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PO Box 12400, Brandon
Wellington 6144, New Zealand
+64 4 471 7500

NZ Transport Agency
Emerald Glen Connection Road, Kapiti
Road Alignment

Drawn: K. Mearns
Checked: R. Collier
Approved: C. Lee

Issue Date
SEP 2010

Culverts - Emerald Glen Culvert
Plan Layout

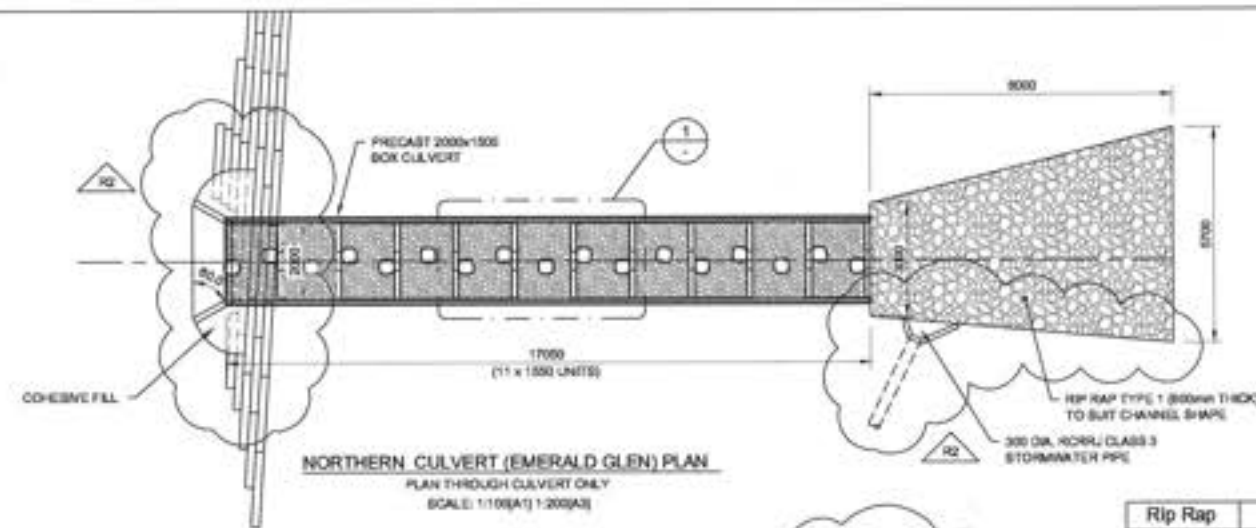
Sheet No.
5-C1744.01

Scale
1:100(A1), 1:200(A3)

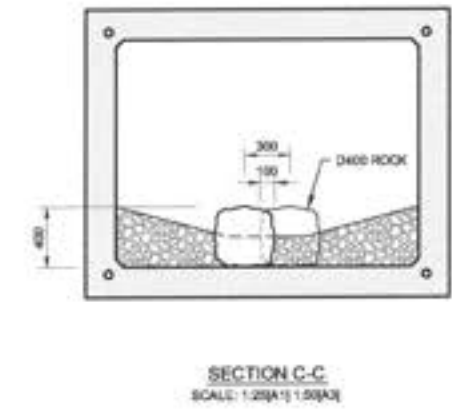
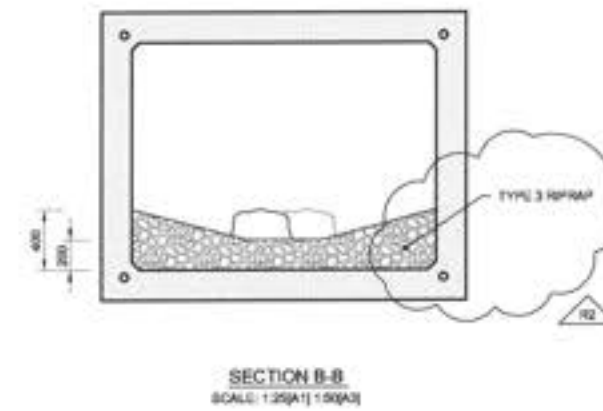
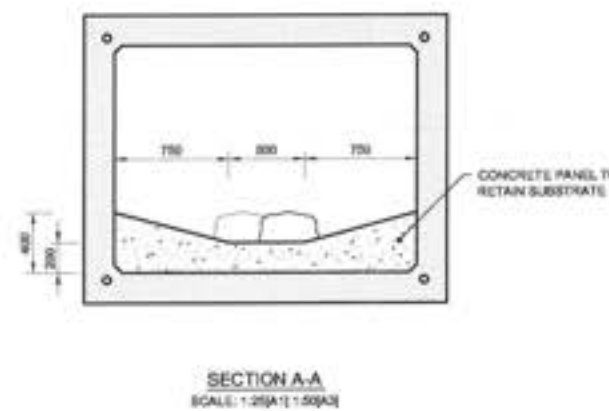
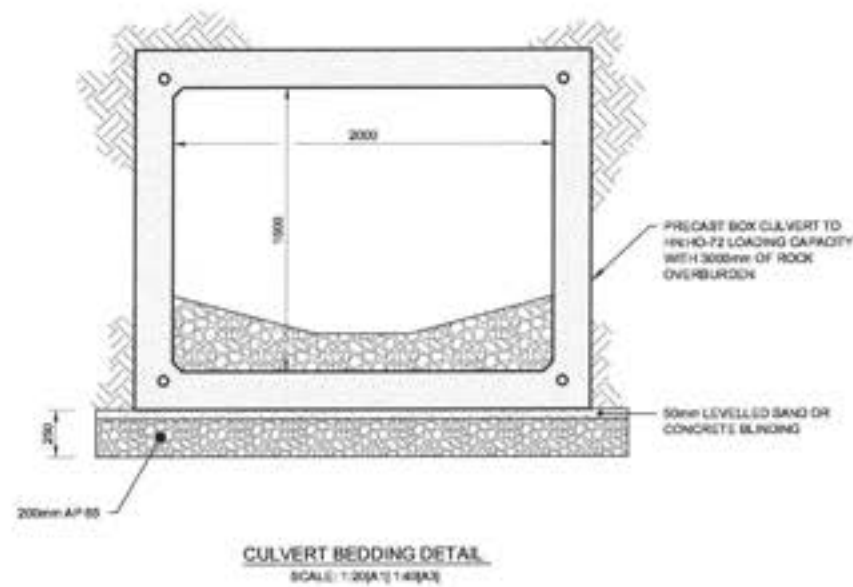
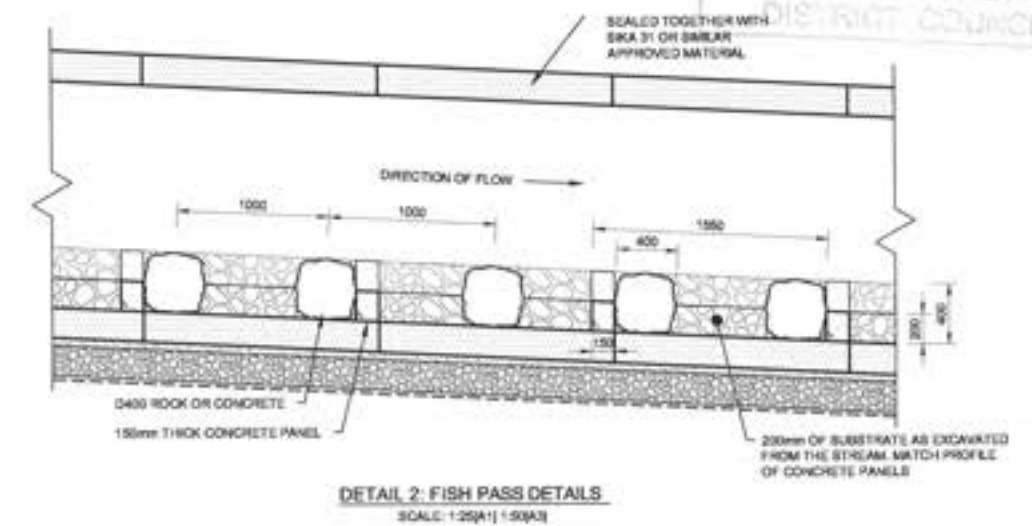
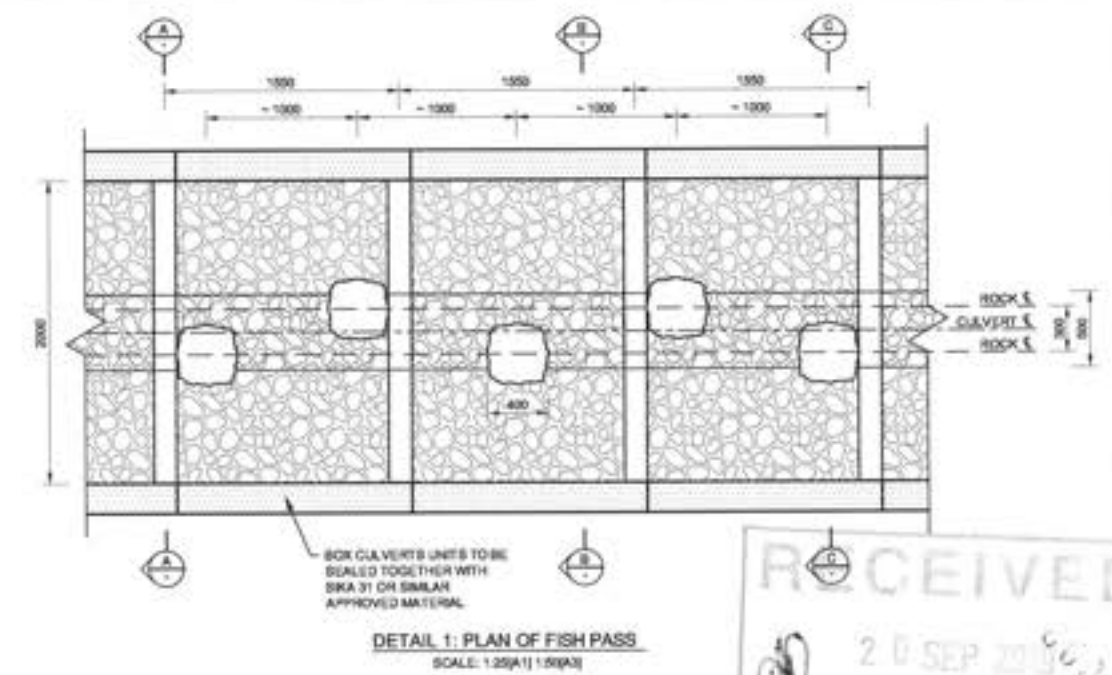
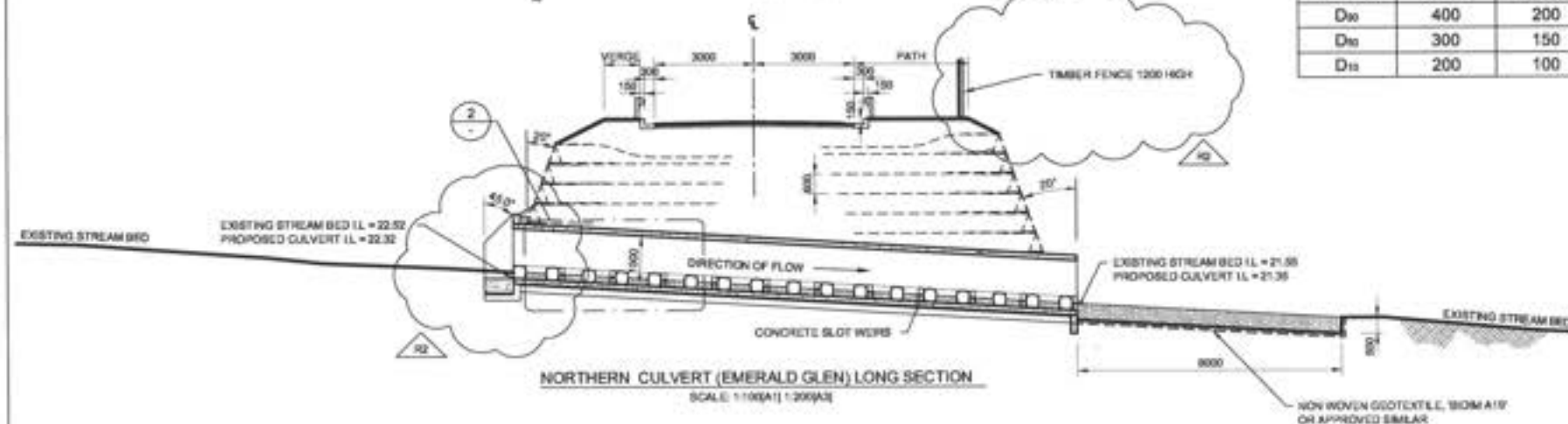
Drawing No.
5 / 2573 / 1 / 7504

Sheet No.
301

Revision
R1



Rip Rap	Type 1	Type 2
D ₅₀	400	200
D ₃₀	300	150
D ₁₅	200	100



REVISIONS
5/2573/1/7504/01 TO 305 CULVERTS-EMERALD GLEN
5/2573/1/7504/01 TO 313 CULVERTS-AMOR
5/2573/1/7504/021 AND 322 OPEN CHANNELS
5/2573/1/7504/031 ROAD DRAINAGE

Rev	Description	Author	Check
R2	ISSUED FOR CONSENT		
R1	ISSUED FOR INFO	C.L.	19/07/10
R0	ISSUED FOR CONSTRUCTION	C.L.	10/07/10



Wellington Office
PO Box 12 023, Thorndon
Wellington 6144, New Zealand
+64 4 471 7000

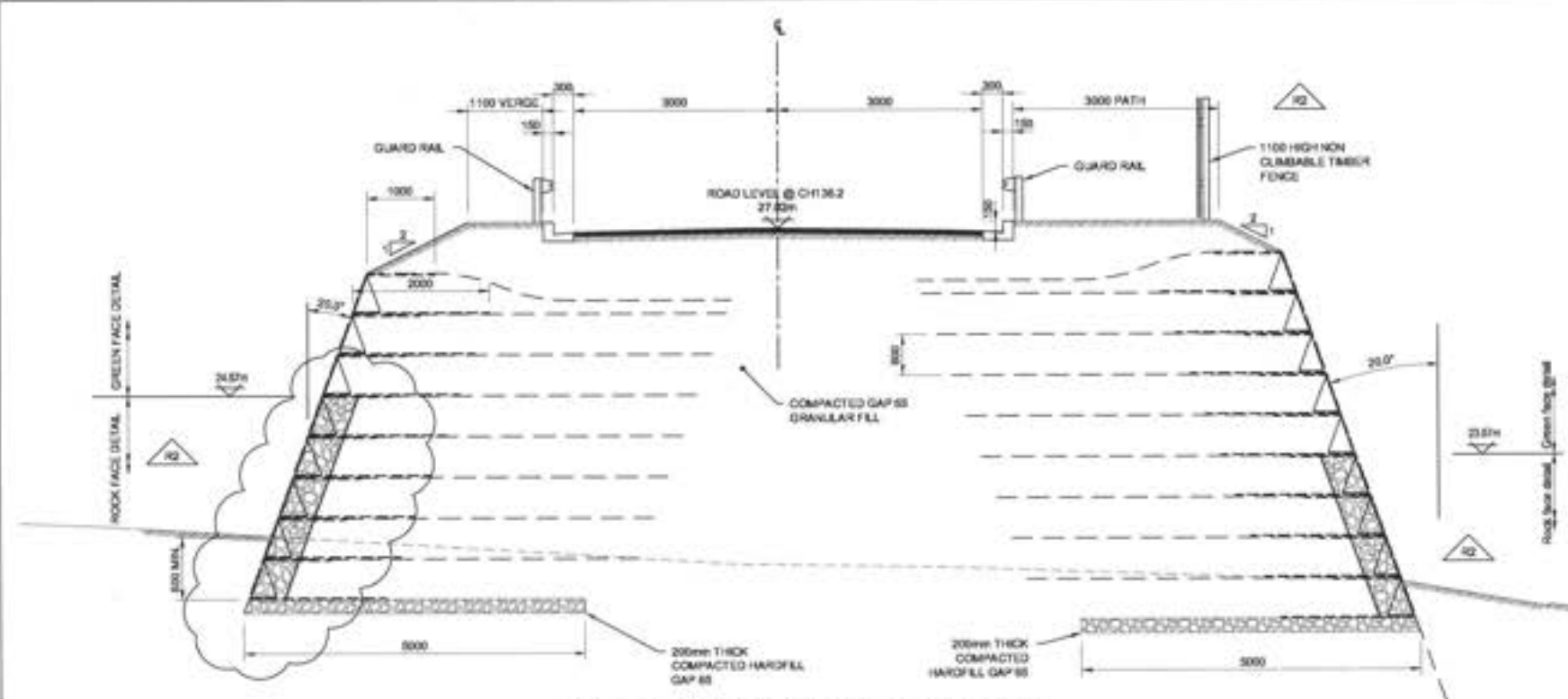
NZ Transport Agency
Emerald Glen Connection Road, Kapiti
Road Alignment

Culvert Details - Emerald Glen Culvert
Culvert Section and Fish Pass Details

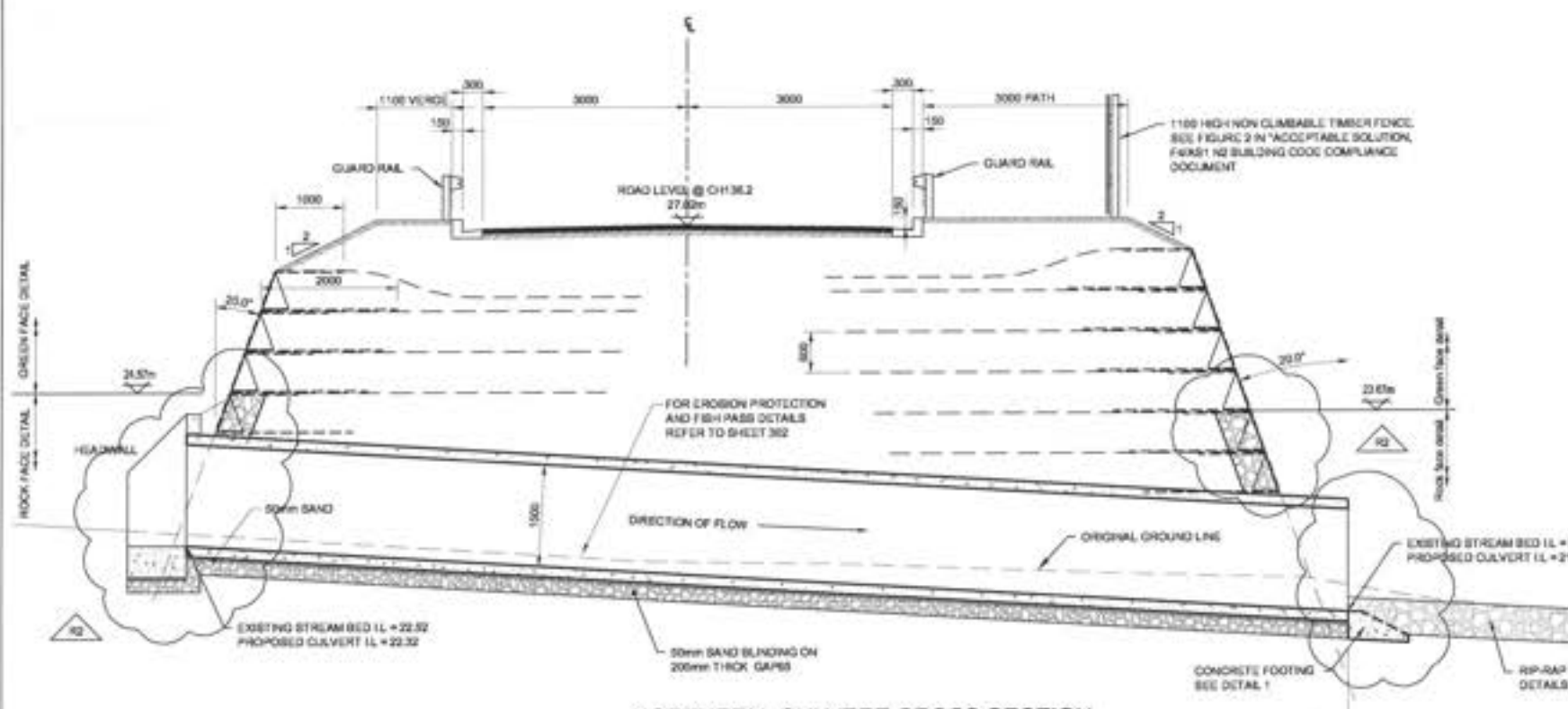
Drawn	Checked	Approved	Revision
H. Clark	R. Clark	C. Lee	SEPT 2010
Project No.	5-C1744.01	Scale	As Shown

Sheet No. 5 / 2573 / 1 / 7504

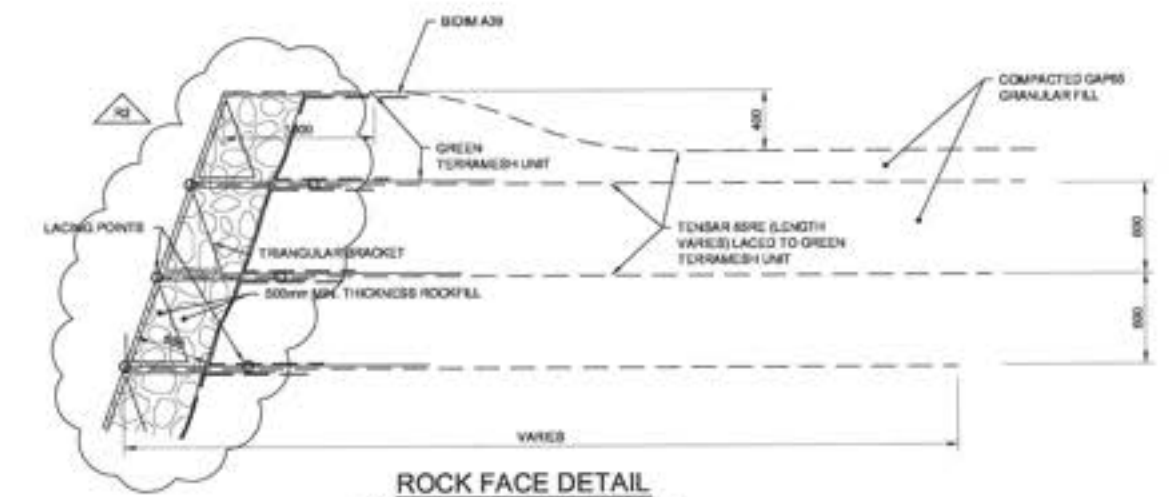
302 R2



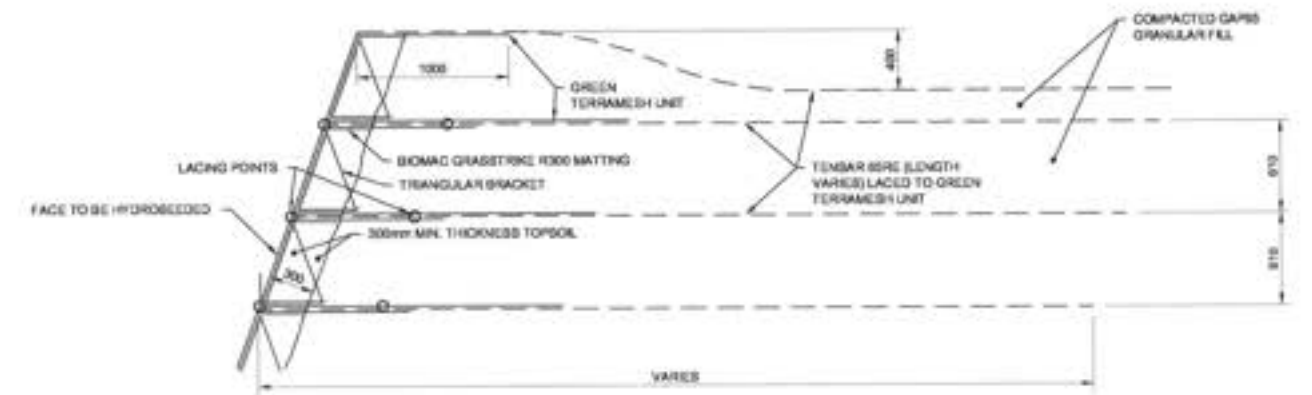
TYPICAL EMBANKMENT CROSS SECTION
(South of Northern Culvert, chainage 138m)
SCALE: 1:50[A1] 1:100[A3]



NORTHERN CULVERT CROSS SECTION
(by Emerald Glen Road, Chainage 136m)
SCALE: 1:50[A1] 1:100[A3]



ROCK FACE DETAIL
(Below R.L. of water level during flood)
SCALE: 1:25[A1] 1:50[A3]



GREEN FACE DETAIL
(Above R.L. of water level during flood)
SCALE: 1:25[A1] 1:50[A3]



CONCRETE FOOTING DETAIL 1
SCALE: 1:20[A1] 1:40[A3]



- Notes:**
1. Reinforcement shall be Tensar 65RE or 65VE Geogrid.
 2. The reinforcement shall be connected to the Terramesh unit by using at 2 points.
 3. The reinforcement shall extend from the face of the Terramesh unit into the 50 embankment (lengths shown).
 4. The reinforcement shall be oriented with the highest strength axis perpendicular to the wall face alignment.
 5. Reinforcement shall be continuous throughout their embankment lengths and placed side by side to provide 100% coverage. Spliced connections between shorter pieces of Geogrid or gaps between adjacent pieces of reinforcement are not permitted.
 6. The reinforcement shall be pulled tight prior to backfill placement on the reinforcement.

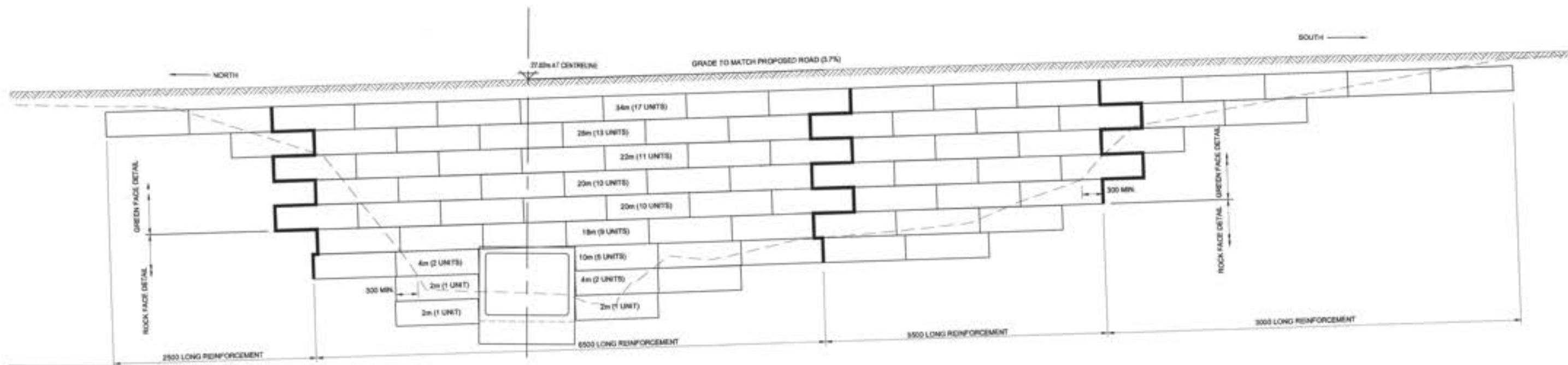
REFERENCE DRAWINGS:

50573/1/7504/01 TO 306 CULVERTS (EMERALD GLEN)
50573/1/7504/01 TO 313 CULVERTS (MINOR)
50573/1/7504/02 AND 322 OPEN CHANNELS
50573/1/7504/03: ROAD CHAINAGE

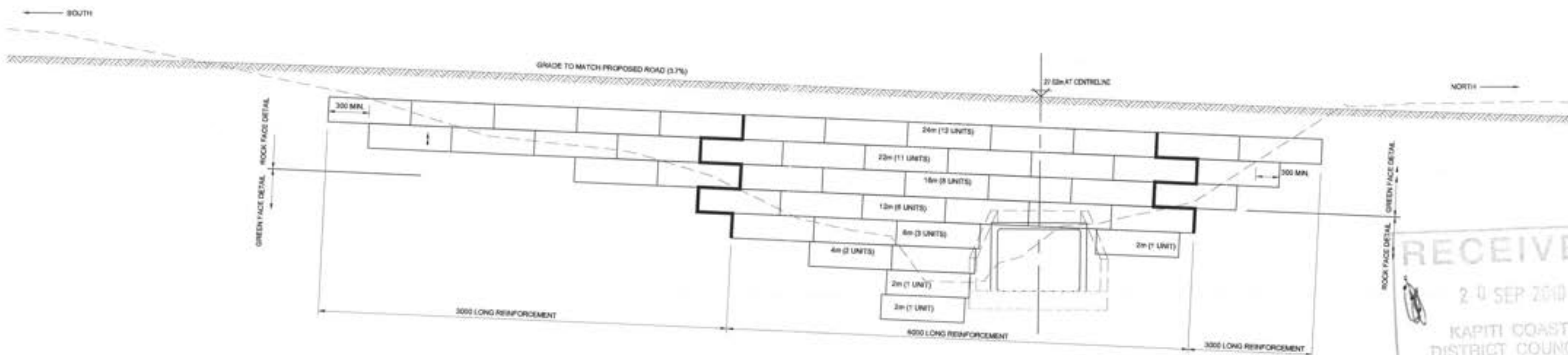
Stage	Approved	Approved	Revised
R2	ISSUED FOR MFD		
R1	RENUMBERED AND REVISED	C.L.	190910
R2	ISSUED FOR CONSTRUCTION	C.L.	50910



				Project: NZ Transport Agency Emerald Glen Connection Road, Kapiti Road Alignment	
Wellington Office PO Box 12 020, Thorndon Wellington 6144, New Zealand +64 4 471 7000				Sheet: Culverts - Emerald Glen Culvert Section Showing Terramesh Earth Wall Details	
Drawn: H. Clarke Checked: R. Oles & J. O. LAM Project: 5-C1744.01 Scale: As Shown	Date: 19/09/10 Revision: 1 Date: 19/09/10	Date: 19/09/10 Revision: 1 Date: 19/09/10	Date: 19/09/10 Revision: 1 Date: 19/09/10	Sheet No.: 303 Total: 303	Designer: R2



8.4m
DOWNSTREAM ELEVATION (LOOKING UPSTREAM)



24.2m
UPSTREAM ELEVATION (LOOKING DOWNSTREAM)

- Notes:
1. Reinforcement shall be Tensar/BSR or BSR Geogrid.
 2. The reinforcement shall be connected to the Terramesh unit by using at 2 points.
 3. The reinforcement shall extend from the face of the Terramesh unit into the fill embankment (lengths shown).
 4. The reinforcement shall be oriented with the highest strength axis perpendicular to the wall face alignment.
 5. Reinforcement shall be continuous throughout their embedment lengths and placed side by side to provide 100% coverage. Spliced connections between shorter pieces of Geogrid or gaps between adjacent pieces of reinforcement are not permitted.
 6. The reinforcement shall be pulled tight prior to backfill placement on the reinforcement.

REFERENCE DRAWINGS

5/2573/1/7504/001 TO 305 CULVERTS-EMERALD GLEN
5/2573/1/7504/011 TO 313 CULVERTS-MINOR
5/2573/1/7504/021 AND 322 OPEN CHANNELS
5/2573/1/7504/031 ROAD DRAINAGE

Revision	Description	Author	Checked
R1	ISSUED FOR INFO		
R2	RE-ALIGNED AND REVISED	C.L.	15/09/10
R3	ISSUED FOR CONSTRUCTION	C.L.	05/09/10



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PO Box 12-003, Thorndon
Wellington 6144, New Zealand
+64 4 671 7200

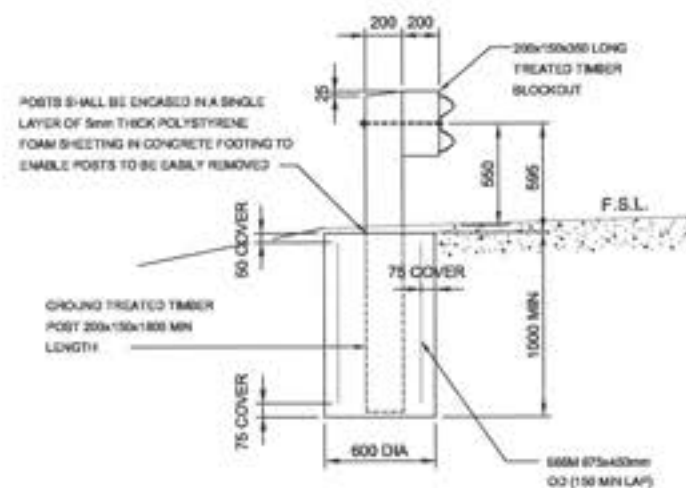
NZ Transport Agency
Emerald Glen Connection Road, Kapiti
Road Alignment

Culverts - Emerald Glen Culvert
End Elevations Showing Terramesh Earth Wall Details

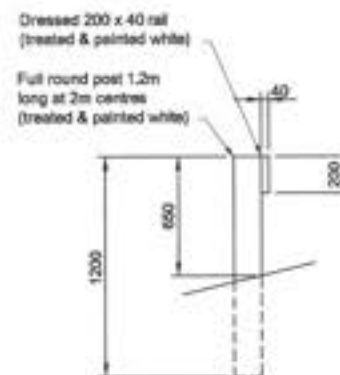
Project No. 5-C1744.01
Scale 1:50(A1), 1:100(A3)
Drawing No. 5 / 2573 / 1 / 7504

Sheet No. 304
Revision R2

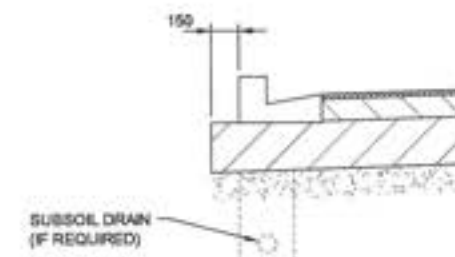




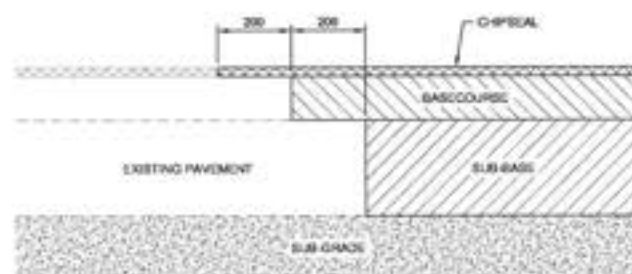
**TYPICAL GUARDRAIL POST DETAIL
IN CONCRETE FOOTING**
1:20



SIGHTRAIL
1:20



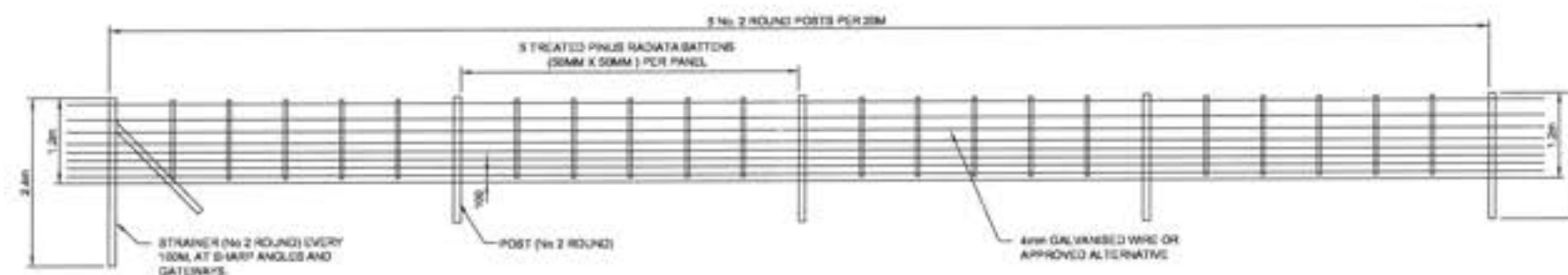
KERB & CHANNEL DETAIL
1:20



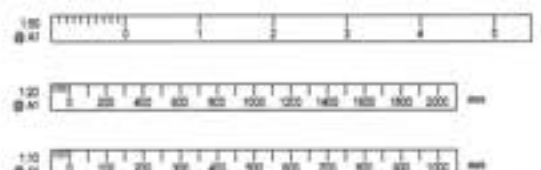
**TRANSVERSE JOINT BETWEEN
NEW & EXISTING PAVEMENT**
1:50



KERB AND CHANNEL - TYPE 1
SCALE 1:10



PERMANENT POST AND 8-WIRE FENCE DETAIL
SCALE 1:50



Revised	Description	Author	Checked	Date
42	ISSUED FOR CONSTRUCTION	C.L.	12/09/10	



OPUS		Wellington Office P.O. Box 10-401 Wellington 6142, New Zealand +64 4 471 7300	
Drawn	Designed	Approved	Issued Date
G.J. YOUNG	R.Q.		
Project No.	Drawn No.	Scale	Sheet No.
5C1744.01	AS SHOWN	5/2573/1/7504	122

NEW ZEALAND TRANSPORT AGENCY EMERALD GLEN CONNECTION, KAPITI ROAD ALIGNMENT	
MISCELLANEOUS DETAILS	
Sheet No.	Revision
122	R2

CONSTRUCTION

GUIDANCE ON USE OF PRODUCER STATEMENTS

Producer statements were first introduced with the Building Act 1992. The producer statements were developed by a combined task committee consisting of members of the New Zealand Institute of Architects, Institution of Professional Engineers New Zealand, Association of Consulting Engineers New Zealand in consultation with the Building Officials Institute of New Zealand. The original suite of producer statements has been revised at the date of this form as a result of enactment of the Building Act (2004) by these organisations to ensure standard use within the industry.

The producer statement system is intended to provide Building Consent Authorities (BCAs) with reasonable grounds for the issue of a Building Consent or a Code Compliance Certificate, without having to duplicate design or construction checking undertaken by others.

PS1 Design	Intended for use by a suitably qualified independent design professional in circumstances where the BCA accepts a producer statement for establishing reasonable grounds to issue a Building Consent;
PS2 Design Review	Intended for use by a suitably qualified independent design professional where the BCA accepts an independent design professional's review as the basis for establishing reasonable grounds to issue a Building Consent;
PS3 Construction	Forms commonly used as a certificate of completion of building work are Schedule 6 of NZS 3910:2003 ¹ or Schedules E1/E2 of NZIA's SCC 2007 ²
PS4 Construction Review	Intended for use by a suitably qualified independent design professional who undertakes construction monitoring of the building works where the BCA requests a producer statement prior to issuing a Code Compliance Certificate.

This must be accompanied by a statement of completion of building work (Schedule 6).

The following guidelines are provided by ACENZ, IPENZ and NZIA to interpret the Producer Statement.

Competence of Design Professional

This statement is made by a Design Firm that has undertaken a contract of services for the services named, and is signed by a person authorised by that firm to verify the processes within the firm and competence of its designers.

A competent design professional will have a professional qualification and proven current competence through registration on a national competence-based register, either as a Chartered Professional Engineer (CPEng) or a Registered Architect.

Membership of a professional body, such as the Institution of Professional Engineers New Zealand (IPENZ) or the New Zealand Institute of Architects (NZIA), provides additional assurance of the designer's standing within the profession. If the design firm is a member of the Association of Consulting Engineers New Zealand (ACENZ), this provides additional assurance about the standing of the firm.

Persons or firms meeting these criteria satisfy the term "suitably qualified independent design professional".

* Professional Indemnity Insurance

As part of membership requirements, ACENZ requires all member firms to hold Professional Indemnity Insurance to a minimum level.

The PI insurance minimum stated on the front of this form reflects standard, small projects. If the parties deem this inappropriate for large projects the minimum may be up to \$500,000.

Professional Services during Construction Phase

There are several levels of service which a Design Firm may provide during the construction phase of a project (CM1-CM5)³ (OL1-OL4)². The Building Consent Authority is encouraged to require that the service to be provided by the Design Firm is appropriate for the project concerned.

Requirement to provide Producer Statement PS4

Building Consent Authorities should ensure that the applicant is aware of any requirement for producer statements for the construction phase of building work at the time the building consent is issued as no design professional should be expected to provide a producer statement unless such a requirement forms part of the Design Firm's engagement.

Attached Particulars

Attached particulars referred to in this producer statement refer to supplementary information appended to the producer statement.

Refer Also:

¹ Conditions of Contract for Building & Civil Engineering Construction NZS 3910: 2003

² NZIA Standard Conditions of Contract SCC 2007 (1st edition)

³ Guideline on the Briefing & Engagement for Consulting Engineering Services (ACENZ/IPENZ 2004)

www.acenz.org.nz
www.ipenz.org.nz
www.nzia.co.nz



PRODUCER STATEMENT – PS1 – DESIGN

(Guidance notes on the use of this form are printed on the reverse side*)

 ISSUED BY: Opus International Consultants Limited
(Design Firm)

 TO: New Zealand Transport Authority
(Owner/Developer)

 TO BE SUPPLIED TO: Kapiti Coast District Council
(Building Consent Authority)

 IN RESPECT OF: Terramesh Green Wall
(Description of Building Work)

 AT: Emerald Glen Connection, Northern Culvert
(Address)
N/A

 LOT N/A

 DP N/A

 SO N/A

 We have been engaged by the owner/developer referred to above to provide design of retaining wall
at the northern culvert

 Clause(s) B1 and B2 (Extent of Engagement) services in respect of the requirements of
 of the Building Code for
☐ All or ☒ Part only (as specified in the attachment to this statement), of the proposed building work.

The design carried out by us has been prepared in accordance with:

☒ Compliance Documents issued by Department of Building & Housing B1/VM4
(verification method / acceptable solution)
☐ Alternative solution as per the attached schedule

 The proposed building work covered by this producer statement is described on the drawings titled Culverts - Emerald
Glen Culvert Section Showing and numbered S/2573/117504/303R2 & 304R2
Terramesh Earth Wall details
 together with the specification, and other documents set out in the schedule attached to this statement.

On behalf of the Design Firm, and subject to:

- (i) Site verification of the following design assumptions refer to the Specification
- (ii) All proprietary products meeting their performance specification requirements;

I believe on reasonable grounds the building, if constructed in accordance with the drawings, specifications, and other documents provided or listed in the attached schedule, will comply with the relevant provisions of the Building Code.

 I, Alexei Murasher am: ☒ CPEng 178533 #
(Name of Design Professional)
☐ Reg Arch #

 I am a Member of: ☒ IPENZ ☐ NZIA and hold the following qualifications:

 The Design Firm issuing this statement holds a current policy of Professional Indemnity Insurance no less than \$200,000*.
 The Design Firm is a member of ACENZ ☒ YES ☐ NO

 SIGNED BY Alexei Murasher ON BEHALF OF Opus International Consultants
(Design Firm)

 Date 14/09/10 (signature) [Signature]

Note: This statement shall only be relied upon by the Building Consent Authority named above. Liability under this statement accrues to the Design Firm only. The total maximum amount of damages payable arising from this statement and all other statements provided to the Building Consent Authority in relation to this building work, whether in contract, tort or otherwise (including negligence), is limited to the sum of \$200,000*.

This form is to accompany Form 2 of the Building (Forms) Regulations 2004 for the application of a Building Consent.

[illegible]

lot 1	200-433	lot 7	200-441	lot 13	200-449
lot 2	200-434	lot 8	200-442	lot 14	200-450
lot 3	200-435	lot 9	200-443	lot 15	200-451
lot 4	200-436	lot 10	200-444	lot 16	200-452
lot 5	200-437	lot 11	200-445	lot 17	200-453
lot 6	200-438	lot 12	200-446	lot 18	200-454
lot 19	200-440	lot 24	200-452	lot 30	200-456
lot 20	200-441	lot 25	200-453	lot 31	200-457
lot 21	200-442	lot 26	200-454	lot 32	200-458
lot 22	200-443	lot 27	200-455	lot 33	200-459
lot 23	200-444	lot 28	200-456	lot 34	200-460

negative the β_2 is the

Journal of Management Inquiry 22(1)

Admission on this plan was on August 11, 1945.

All figures shown herein are of good & true construction.

See also 10100, 10101, 10102, 10103, 10104, 10105, 10106, 10107, 10108, 10109, 10110, 10111, 10112, 10113, 10114, 10115, 10116, 10117, 10118, 10119, 10120, 10121, 10122, 10123, 10124, 10125, 10126, 10127, 10128, 10129, 10130, 10131, 10132, 10133, 10134, 10135, 10136, 10137, 10138, 10139, 10140, 10141, 10142, 10143, 10144, 10145, 10146, 10147, 10148, 10149, 10150, 10151, 10152, 10153, 10154, 10155, 10156, 10157, 10158, 10159, 10160, 10161, 10162, 10163, 10164, 10165, 10166, 10167, 10168, 10169, 10170, 10171, 10172, 10173, 10174, 10175, 10176, 10177, 10178, 10179, 10180, 10181, 10182, 10183, 10184, 10185, 10186, 10187, 10188, 10189, 10190, 10191, 10192, 10193, 10194, 10195, 10196, 10197, 10198, 10199, 10200, 10201, 10202, 10203, 10204, 10205, 10206, 10207, 10208, 10209, 10210, 10211, 10212, 10213, 10214, 10215, 10216, 10217, 10218, 10219, 10220, 10221, 10222, 10223, 10224, 10225, 10226, 10227, 10228, 10229, 10230, 10231, 10232, 10233, 10234, 10235, 10236, 10237, 10238, 10239, 10240, 10241, 10242, 10243, 10244, 10245, 10246, 10247, 10248, 10249, 10250, 10251, 10252, 10253, 10254, 10255, 10256, 10257, 10258, 10259, 10260, 10261, 10262, 10263, 10264, 10265, 10266, 10267, 10268, 10269, 10270, 10271, 10272, 10273, 10274, 10275, 10276, 10277, 10278, 10279, 10280, 10281, 10282, 10283, 10284, 10285, 10286, 10287, 10288, 10289, 10290, 10291, 10292, 10293, 10294, 10295, 10296, 10297, 10298, 10299, 10300, 10301, 10302, 10303, 10304, 10305, 10306, 10307, 10308, 10309, 10310, 10311, 10312, 10313, 10314, 10315, 10316, 10317, 10318, 10319, 10320, 10321, 10322, 10323, 10324, 10325, 10326, 10327, 10328, 10329, 10330, 10331, 10332, 10333, 10334, 10335, 10336, 10337, 10338, 10339, 10340, 10341, 10342, 10343, 10344, 10345, 10346, 10347, 10348, 10349, 10350, 10351, 10352, 10353, 10354, 10355, 10356, 10357, 10358, 10359, 10360, 10361, 10362, 10363, 10364, 10365, 10366, 10367, 10368, 10369, 10370, 10371, 10372, 10373, 10374, 10375, 10376, 10377, 10378, 10379, 10380, 10381, 10382, 10383, 10384, 10385, 10386, 10387, 10388, 10389, 10390, 10391, 10392, 10393, 10394, 10395, 10396, 10397, 10398, 10399, 10400, 10401, 10402, 10403, 10404, 10405, 10406, 10407, 10408, 10409, 10410, 10411, 10412, 10413, 10414, 10415, 10416, 10417, 10418, 10419, 10420, 10421, 10422, 10423, 10424, 10425, 10426, 10427, 10428, 10429, 10430, 10431, 10432, 10433, 10434, 10435, 10436, 10437, 10438, 10439, 10440, 10441, 10442, 10443, 10444, 10445, 10446, 10447, 10448, 10449, 10450, 10451, 10452, 10453, 10454, 10455, 10456, 10457, 10458, 10459, 10460, 10461, 10462, 10463, 10464, 10465, 10466, 10467, 10468, 10469, 10470, 10471, 10472, 10473, 10474, 10475, 10476, 10477, 10478, 10479, 10480, 10481, 10482, 10483, 10484, 10485, 10486, 10487, 10488, 10489, 10490, 10491, 10492, 10493, 10494, 10495, 10496, 10497, 10498, 10499, 10500, 10501, 10502, 10503, 10504, 10505, 10506, 10507, 10508, 10509, 10510, 10511, 10512, 10513, 10514, 10515, 10516, 10517, 10518, 10519, 10520, 10521, 10522, 10523, 10524, 10525, 10526, 10527, 10528, 10529, 10530, 10531, 10532, 10533, 10534, 10535, 10536, 10537, 10538, 10539, 10540, 10541, 10542, 10543, 10544, 10545, 10546, 10547, 10548, 10549, 10550, 10551, 10552, 10553, 10554, 10555, 10556, 10557, 10558, 10559, 10560, 10561, 10562, 10563, 10564, 10565, 10566, 10567, 10568, 10569, 10570, 10571, 10572, 10573, 10574, 10575, 10576, 10577, 10578, 10579, 10580, 10581, 10582, 10583, 10584, 10585, 10586, 10587, 10588, 10589, 10590, 10591, 10592, 10593, 10594, 10595, 10596, 10597, 10598, 10599, 10600, 10601, 10602, 10603, 10604, 10605, 10606, 10607, 10608, 10609, 10610, 10611, 10612, 10613, 10614, 10615, 10616, 10617, 10618, 10619, 10620, 10621, 10622, 10623, 10624, 10625, 10626, 10627, 10628, 10629, 10630, 10631, 10632, 10633, 10634, 10635, 10636, 10637, 10638, 10639, 10640, 10641, 10642, 10643, 10644, 10645, 10646, 10647, 10648, 10649, 10650, 10651, 10652, 10653, 10654, 10655, 10656, 10657, 10658, 10659, 10660, 10661, 10662, 10663, 10664, 10665, 10666, 10667, 10668, 10669, 10670, 10671, 10672, 10673, 10674, 10675, 10676, 10677, 10678, 10679, 10680, 10681, 10682, 10683

REMARKS ON EVIDENCE

Participant	Condition	Pretest Score	Posttest Score
Participant 1	Baseline	2.00	2.00
Participant 1	Baseline	2.00	2.00
Participant 1	Baseline	2.00	2.00

4500 4500 4500

SUBDIVISION OF PT

SEC 73 & SECS 74,75,76,84,85,86
& 96 BLK II, PAKAKARKI S. D.
PT SECS 8,20,23 & 24 WAINU DIST
PART WHAREROA 1B & 2A &
CLOSED ROAD

BLK II PAKAKARIKI SURVEY DIST.
WELLINGTON LAND DIST. HUTI COUNTY
Scale: 5 chains to an inch. Surveyed by Foster & Foster.
July 1971 - Feb. 1972.

Rural Lots. 685 - 1 - 29
Road To Vest. 4 - 3 - 20 - 6
Severance Lot 24. 4 - 3 -
TOTAL AREA. 686 - 1 - 13 - 9

33688
Sheet 1 of 2 Sheet

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KAPITI COAST
DISTRICT COUNCIL

Calculation Sheet

$$\frac{\phi}{2} = 150,149$$

Project/Task/File No:

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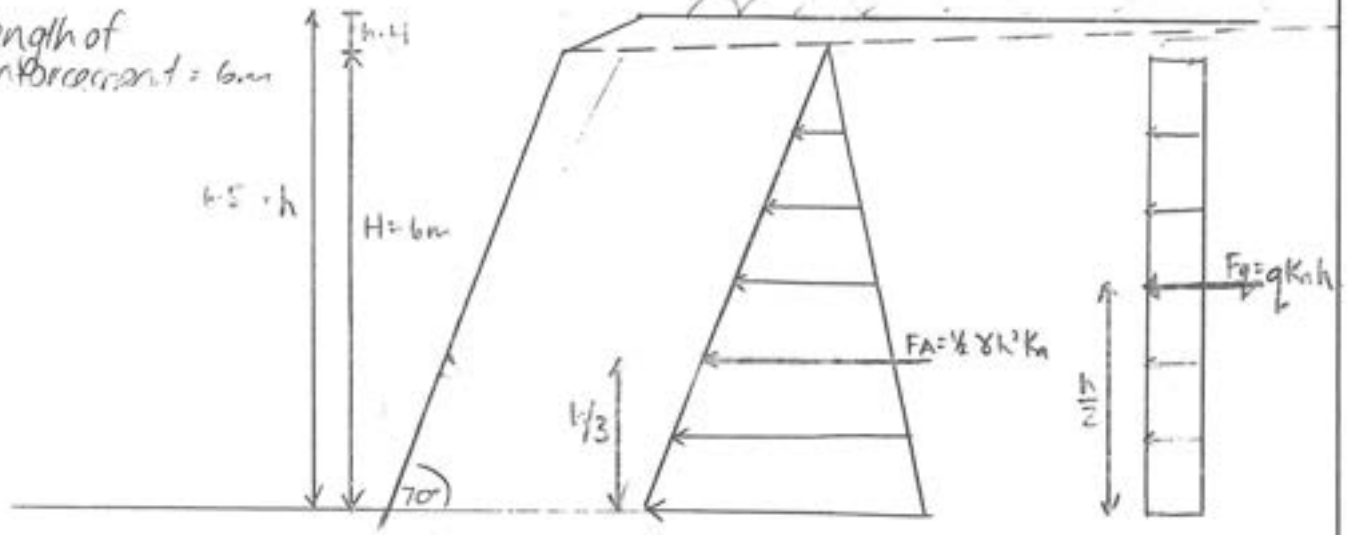
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Checked:

External stability under static conditions

Length of reinforcement = 6m



Minimum Embedment:

$$\text{Slope in front of wall} = 3H:1V \Rightarrow H/10 = 600\text{mm}$$

* note: assume flat backfill

$$K_{af} = \frac{\sin^2(\theta + \phi)}{\sin^2\theta \sin(\theta - \delta) \left[1 + \frac{\sin(\theta + \delta) \sin(\phi - \beta)}{\sin(\theta - \delta) \sin(\theta + \beta)} \right]^2}$$

$$\phi = 35^\circ$$

$$\beta = 3^\circ \text{ (ignore)}$$

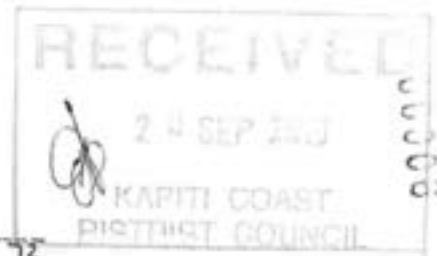
$$\delta = \phi/2 = 17.5^\circ$$

$$\theta = 110^\circ$$

$$K_{af} = \frac{\sin^2(145)}{\sin^2 110 \sin 92.5 \left[1 + \frac{\sin(52.5) \sin(32)}{\sin 92.5 \sin 113} \right]^2} \quad (35)$$

$$= \frac{0.329}{0.877 \cdot [1 + 0.676]^2} \quad (115)$$

$$K_{af} = 0.133 \quad (0.130)$$



Calculation Sheet

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$$F_A = 0.5 K_A \gamma_f h^2 = 0.5 \times 0.133 \times 19 \times 6^2$$

$$= 45.5 \text{ kN}$$

$$\text{Weight of wall} = \gamma h L$$

$$= 19 \times 6 \times 6$$

$$= 684 \text{ kN}$$

$$F_q = q K_{af} h = 10 \times 0.133 \times 6$$

$$= 7.98 \text{ kN}$$

1. Forward Sliding

$$F_h^* \leq \Phi F_v^* \tan \phi \quad (A/B)$$

$$F_h^* \leq T_{d,i,z}^* \quad (C)$$

$$F_h^* \leq \Phi c_u L \quad (\text{cohesive soil})$$

$$F_h^* = (45.5 + 7.98) \times 1.5$$

$$= 80.22 \text{ kN}$$

$$F_v^* = 684 \times 1.0$$

$$= 684 \text{ kN}$$

$$\text{where } \Phi = 1.0$$

$$\Phi F_v^* \tan \phi = 1.0 \times 684 \times \tan 35$$

$$= 478 \text{ kN}$$

$$> F_h^*$$

OK!!

$$T_{d,i,z}^* = T_{i,z} \Phi \Phi_n$$

$$\text{where } \Phi = 0.8 \quad \text{Table A6}$$

$$\Phi_n = 1.0 \quad \text{Table A5}$$

$$T_{d,i,z}^* = (\tan \phi (\alpha \sigma_v' L_o)) \Phi \Phi_n$$

$$= (\tan 35 \times 0.9 \times 19 \times 6 \times 6) \times 0.8 \times 1.0$$

$$= 345 \text{ kN}$$

$$> F_h^*$$

OK!!

$$\alpha_s = 0.9 \quad \text{Macraferri interaction factor}$$



Calculation Sheet

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2. Bearing Capacity Failure

$$\sigma_v^* = F_v^* / (L - 2e)$$

$$e = 700 \text{ mm (refer to Slope/W diagram)}$$

$$\sigma_v^* = \frac{684}{(6 - 2 \times 0.7)}$$

$$= 148.7 \text{ kN/m}$$

$$\sigma_v^* \leq \Phi q_u$$

$$q_{ult} = c N_c + p_0 N_q + 1/2 \gamma B N_\gamma$$

$$= 19 \times 6 \times 33.3 + 1/2 \times 19 \times 6 \times 33.9$$

$$= 6,050 \text{ kPa}$$

$$\phi = 35$$

$$N_c = 46.1$$

$$N_q = 33.3$$

$$N_\gamma = 33.9$$

$$\Phi q_u = 0.6 \times 6050$$

$$\Phi = 0.6$$

$$= 3,630.3 \text{ kN/m}$$

$$> \sigma_v^* \quad \underline{\underline{OK!!}}$$

Reduced Foundation Strength
~~1200 kPa~~

$$\phi = 28 \quad \gamma = 17$$

$$N_c = 25.8$$

$$N_q = 14.7$$

$$N_\gamma = 10.9$$

3. Overturning

$$M_d^* = 1.5 \times [45.5 \times 2 + 7.98 \times 3]$$

$$= 172.4 \text{ kNm}$$

$$M_r^* = 1.0 \times [684 \times 3]$$

$$= 2052 \text{ kNm}$$

$$172 \leq 2052 \times 0.75$$

$$\leq 1539 \text{ kNm}$$

$$\underline{\underline{OK!!}}$$

$$\therefore q_{ult} = 2,150 \text{ kN/m}$$

$$0.6 q_{ult} = 1288.8$$

$$\gg \sigma_v^*$$

$$\underline{\underline{OK!!}}$$

References: Opus Retaining Wall Design Guide
Notes



Calculation Sheet

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Internal stability under seismic conditions

~~1. Reinforce~~

$$K_{ar} = \frac{\sin^2(\theta + \phi_r)}{\sin^3 \theta \left[1 + \frac{\sin \phi_r}{\sin \theta} \right]^2}$$

$\theta = 110^\circ$
 $\phi_r = 35^\circ$

$$= \frac{\sin^2(145)}{\sin^3 110 \left[1 + \frac{\sin 35}{\sin 110} \right]^2} = \frac{0.329}{0.8298 \times 2.593}$$

$$= 0.153$$

1. Internal sliding

Use 6m long Tensor GSRE @ 0.6m c/s vert

$$F_{h2}^* = 0.5 K_{ar} \gamma_r H^2$$

$$= 0.5 \times 0.153 \times 19 \times 6^2$$

$$= 52.3 \text{ kN}$$

$$T_{d1,2}^* = 345 \text{ kN (from External stability)}$$

$$T_{fr}^* =$$

$$F_{h2}^* < T_{d1,2}^* + T_{fr}^*$$

2 Connection Failure.

$$F_j^* \leq \Phi_c T_c$$

$$\Phi = 1.0 \text{ (static conditions)}$$

$$F_j^* = \sigma_{hj}^* S_{uj}$$

$$= 19 \times 0.6 \times 0.6$$

$$= 6.84 \text{ kN/m}$$

$$\Phi T_c = 15 \text{ kN/m}$$

$$\geq F_j^*$$

OK!!

	Normal load	Connection Strength
At top	11.4 kN	15 kN/m
At bottom	114 kN	50 kN/m

(refer to table in
N/A 03/09/00 brochure)



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$$C(T) = C_h(T) \cdot Z \cdot R \cdot N(T, D) \cdot S_p$$

Subsoil
Class = C

$$S_p = 0.8$$

$$C_h(T) = 1.33$$

$$N(T, D) = 1$$

$$\leftarrow D \leq 2 \text{ km.} \Rightarrow N(T, D) = N_{max}'(T)$$

Importance = 2 (Permanent)

$$APE = 1/1000$$

$$\Rightarrow R = 1.3$$

$$Z = 0.4$$

$$C(T) = 1.33 \times 0.8 \times 1.3 \times 0.4$$

$$= 0.553$$

For stability

$$C(T) = 1.33 \times 1.3 \times 0.4$$

$$= 0.699$$



Calculation Sheet

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Seismic Loading

NZ Bridge Manual =

$$\text{Design PGA} = 0.55g$$

* Based on Guidelines for Design and construction of Geosynthetic-Reinforced soil structures in NZ - Alexi, Muzlov

$$q_h = q_{h \text{ ext}} = 0.6 \times \text{PGA} \\ = 0.33g$$

$$q_h = q_{h \text{ int}} = (1.3 - \text{PGA}) \text{PGA} \\ = 0.41g$$

$$\begin{aligned} I &= 0 & \varepsilon &= 0 \\ \theta &= 110 \\ \phi &= 35 \\ \alpha_v &= 0 \\ \alpha_n &= 0.55g \end{aligned}$$

$$\begin{aligned} KAE &= \frac{\cos^2(35 - 90 + 110)}{\cos^2(90 - 110) \cos(90 - 110)} \times \left[1 + \frac{\sin(35) \sin(35)}{\cos(90 - 35) \cos(-90 + 110)} \right]^2 \\ &= \frac{0.329}{[0.830][1 + 0.781]}^2 \\ &= 0.125 \end{aligned}$$

$$\begin{aligned} FAE &= 0.5 \times KAE \times h^2 \\ &= 0.5 \times 0.125 \times 19 \times 6^2 \\ &= 42.75 \Rightarrow 0.0625 \text{ } \nu h^2 \end{aligned}$$



Calculation Sheet

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External Stability under seismic conditions

$$\begin{aligned} \text{Inertia Force} = F_{ir} &= 0.5 a_{h, \text{ext}} \gamma h^2 \\ &= 0.5 \times 0.33 \times (19 \times 6^2) \\ &= 112.86 \end{aligned}$$

$$F_A (\text{static}) = 45.5 \text{ kN}$$

$$F_{AE}' = F_A + 0.5 \Delta F_{AE}$$

$$K_A = K_{af}$$

$$\begin{aligned} K_{AR} &= \frac{\sin^2(\theta + \phi_r)}{\sin^2 \theta \left[1 + \frac{\sin \phi_r}{\sin \theta} \right]^2} = \frac{\sin^2(146)}{\sin^2 110 \left[1 + \frac{\sin 36}{\sin 110} \right]^2} \quad \theta = 110^\circ \\ &= \frac{0.329}{0.830 \times 2.593} = 0.153 \end{aligned}$$

$$\begin{aligned} \Delta K_{AE} &= K_{AE} - K_A = 0.125 - 0.153 \\ &= -0.028 \end{aligned}$$

$$\begin{aligned} \Delta F_{AE} &= 0.5 \times -0.028 \times 19 \times 6^2 \\ &= 9.58 \text{ kN} \end{aligned}$$

$$\begin{aligned} F_{AE}' &= F_A + 0.5 \Delta F_{AE} \\ &= 45.5 + 0.5 \times 9.576 \\ &= 40.712 \text{ kN} \end{aligned}$$

$$\theta (\text{surchage}) = 8 \text{ kN}$$

Forward Sliding

$$\begin{aligned} i) \quad \phi F^* \tan \phi &= 0.9 \times 684 \times \tan 35 \\ &= 431 \text{ kN} \end{aligned}$$

$$\phi = 0.9 \text{ (seismic)}$$

$$F_h^* = 1.0 \times (112.9 + 40.7 + 8) = 161.6 \text{ kN} < 431 \text{ kN}$$

OK!!



Forward Sliding

$$F_h \leq F_v \tan \phi$$

$$F_h \leq 7.00$$

$$F_h \leq \phi \text{ cul}$$

$$F_h = (45.5 + 7.98) \times 1.0$$

$$= 53.5 \text{ kN}$$

i) $F_v = 684 \text{ kN}$

$$\Phi F_v \tan \phi = 0.9 \times 684 \times \tan 35$$

$$= 431 \text{ kN}$$

$$F_h = F_R + F_{ve} + F_q$$

$$= 1.0 (112.9 + 40.7 + 8)$$

$$= 161.6 \text{ kN} < 431 \text{ kN} \quad \text{OK!!}$$

ii) $T_{d,2} = 345 \text{ kN} > F_h = 161.6 \text{ kN} \quad \text{OK!!}$

Bearing Failure

$$e = 700 \text{ mm}$$

$$\sigma_v = F_v / (11 - 2e)$$

$$= 684 \text{ kN} / (11 - 2 \times 0.7)$$

$$= 148.7 \text{ kN/m}$$

$$\sigma_v \leq \Phi q_u$$

$$q_{ult} = 6050 \text{ (high strength parameters)}$$

$$\Phi q_u = 0.7 \times 6050$$

$$= 4235 > \sigma_v$$

OK!!

$$0.7 q_u = 902.1 \text{ kN/m} > \sigma_v$$

$$q_{ult} = 2150 \text{ kN/m (lower strength parameters)}$$

$$\Phi = 0.7 \text{ (seismic)}$$

$$\Phi = 0.9 \text{ (seismic)}$$



Calculation Sheet

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Overturning

$$M_d^* = \Phi M_r^*$$

Φ -reduction factor
= 0.9

$$M_d^* = 1.0 \times [45.5 \times 2 + 7.98 \times 3]$$

$$= 106.92 \text{ kN/m}$$

$$\Phi M_r^* = 0.9 \times [6848 \times 3]$$

$$= 1847 \text{ kN/m} > M_d^* \quad \text{OK!!}$$



Northern Culvert - Circular Failure

FOS (Static) = 1.558

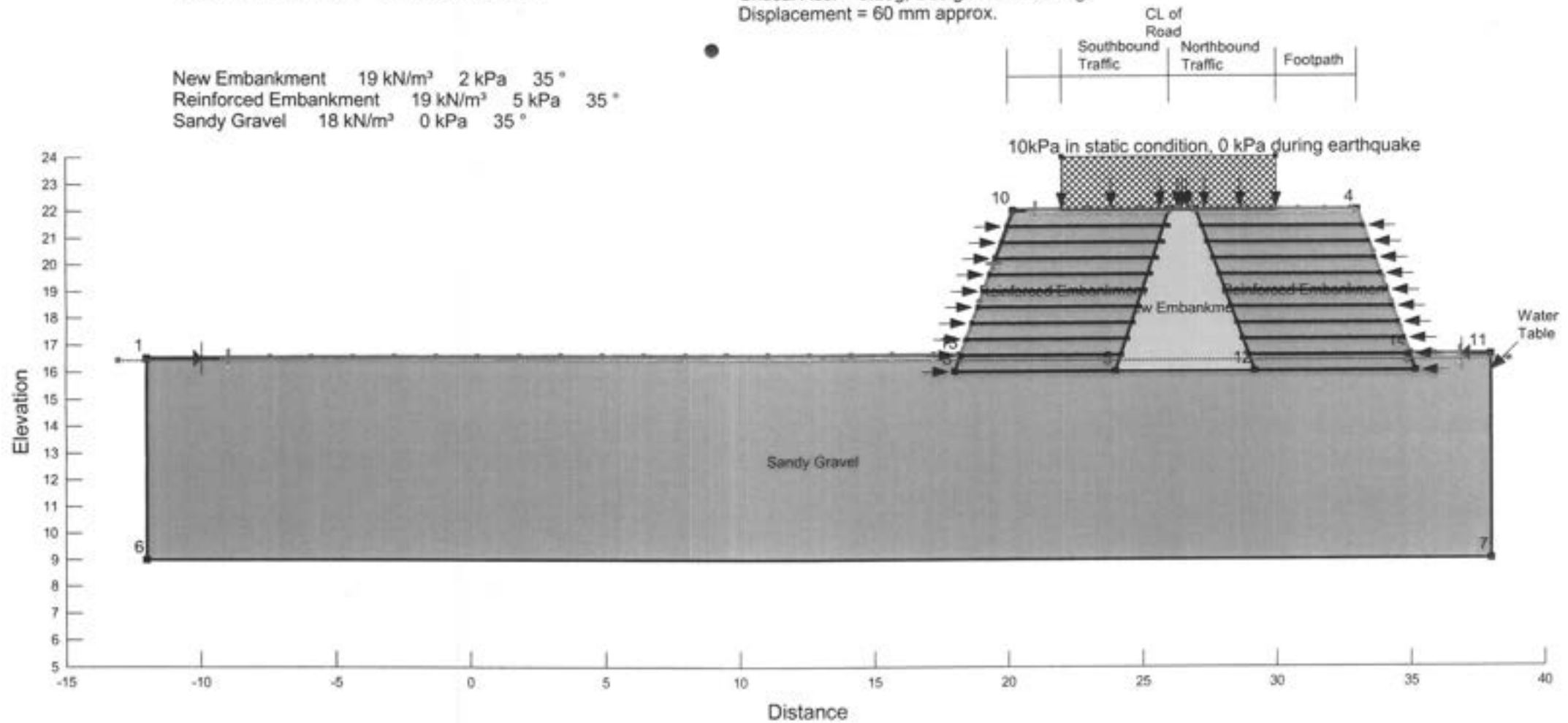
Critical Acc. = 0.23g, Design PGA = 0.55g

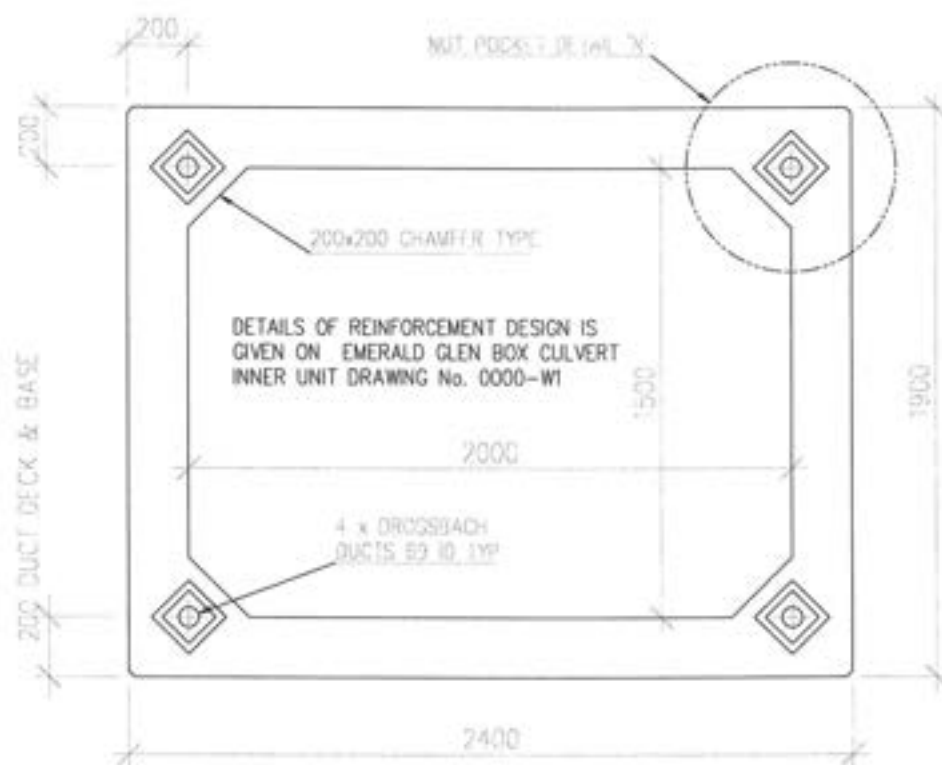
Displacement = 60 mm approx.

New Embankment 19 kN/m³ 2 kPa 35°

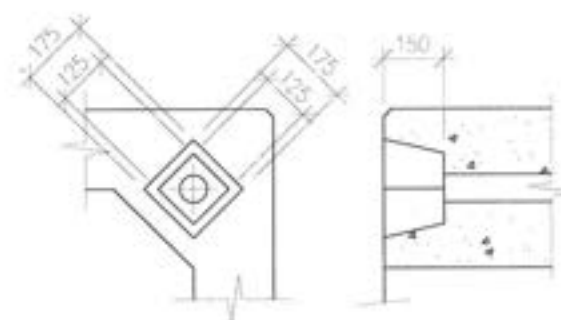
Reinforced Embankment 19 kN/m³ 5 kPa 35°

Sandy Gravel 18 kN/m³ 0 kPa 35°





- 1 MANUFACTURE IN ACCORDANCE WITH
NZS 3109:1997 (PRECAST UNITS)
- 2 CONCRETE STRENGTH @ 28 DAYS 40MPa,
MIN. STRIPPING STRENGTH 15MPa
- 3 SURFACE FINISHES TO NZS 3114:1987,
FORMED SURFACE - F4 FINISH
& TROWELLED SURFACE - U2 FINISH
- 4 BOX UNITS ARE NOT SUITABLE FOR USE
IN SEA WATER OR TIDAL SITES
- 5 CONCRETE COVER TO REINFORCEMENT IS
40mm INSIDE BOX SURFACE,
35mm OUTSIDE BOX SURFACE
- 6 REINFORCEMENT DENOTED 'DH' IS
DEFORMED GRADE 500E STEEL
- 7 DESIGN:- LOADING HN-H0-72
DEPTH OF FILL 1000-4000mm
DESIGN LIFE 100 YEARS
EXPOSURE B1\B2
- 8 BOX UNITS ARE TO BE CURED



NUT POCKET DETAIL 'N'

FOR APPROVAL ONLY
NOT FOR CONSTRUCTION



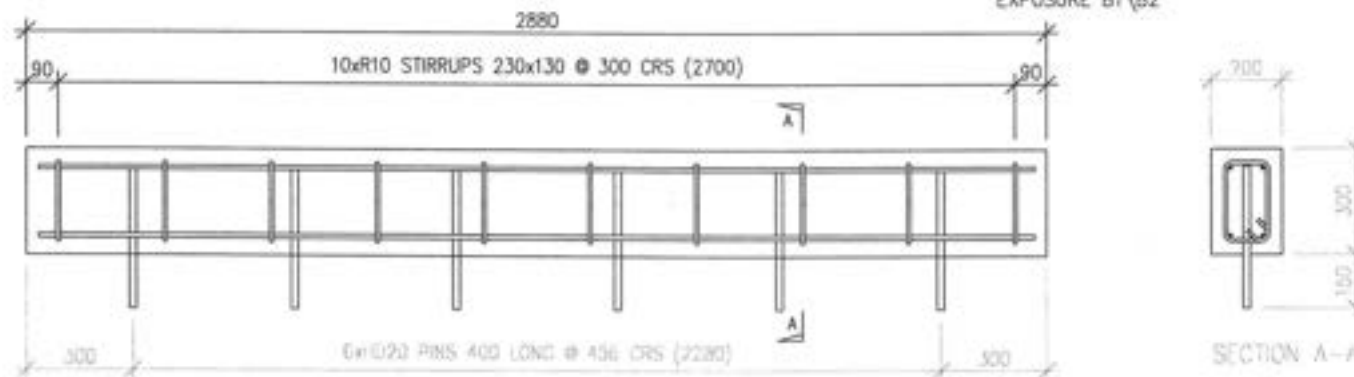
Humes Pipeline Systems
Private Bag 92817
Auckland
Freephone 0800 502 112
www.humes.co.nz

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VOLUME:- 3.88m³
WEIGHT:- 9.71Tonne
— REQUIRED

Rev	Description	Date	By	Check	Product CULVERT	Detailed On SHEET 2/3	Status NTS
A	FOR APPROVAL	8-06-10	JD	-	Code SPC00000	LAYOUT ON	Drawn GB
					Project Name		Date 9-06-2010
					EMERALD GLEN BOX CULVERTS		Checked By -
					WINCWall END UNIT		
					Doc Title ELEVATIONS		00000-W2A

- 1 MANUFACTURE IN ACCORDANCE WITH NZS 3109:1997 (PRECAST UNITS)
- 2 CONCRETE STRENGTH @ 28 DAYS 40MPa, MIN. STRIPPING STRENGTH 15MPa
- 3 SURFACE FINISHES TO NZS 3114:1987, FORMED SURFACE - F4 FINISH
& TROWELLED SURFACE - U2 FINISH
- 4 BOX UNITS ARE NOT SUITABLE FOR USE IN SEA WATER OR TIDAL SITES
- 5 CONCRETE COVER TO REINFORCEMENT IS 35mm
- 6 REINFORCEMENT DENOTED 'DH' IS DEFORMED GRADE 500E STEEL
& THAT DENOTED 'R' IS ROUND GRADE 300E STEEL
- 7 DESIGN:- LOADING HN-HO-72
DEPTH OF FILL 1000-4000mm
DESIGN LIFE 100 YEARS
EXPOSURE B1\B2



VOLUME:- 0.17m³
WEIGHT:- 0.43Tonne
— REQUIRED

Rev	Description	Date	By	Check	Product SPC	Details On SHEET 3/4	Status NTS
1	FOR APPROVAL	1-28-10	DB		Code 00000	Layout On --	Drawn GB
					Project Name		Date 9-09-2010
					EMERALD GLEN BOX CULVERTS		Checked By --
					HEAD WALL		
					Orig Title ELEVATIONS		00000-W3A



Emerald Glen Road
Stream and Culvert Hydraulics Report



Emerald Glen Road Extension Stream and Culvert Hydraulics Report

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Date: July 2010
Reference: S-C1744.00-02EE-012
Status: Final

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Appendix A Hydrology report 'Emerald Glen Flow Regimes'

Appendix B Hydrology Report (Emerald Glen Flow Regimes) Addendum – Maps Showing Minor Catchments

Appendix C Drawings Showing Location and Details of Culverts for Consent Application

Appendix D Culvert Sizing Calculations

Appendix E MacKays Crossing flow Extrapolation

Appendix F MacKays Crossing Backwater effect assessment

1 Introduction

The level crossing for the railway between Waterfall Road and State Highway 1 (SH1) is to be closed due to safety concerns. In order to prevent serious inconvenience to the residents who live in the Waterfall Road area, the New Zealand Transport Agency (NZTA) is planning to extend Emerald Glen Road to connect with the existing SH1 connection at Mackays Crossing as shown in Figure 1.

The proposed road alignment crosses predominantly gently sloping (less than 10%) land in the lee of the foothills of the Taranui Range. The alignment crosses two perennial watercourses and one which we consider intermittent. There are also two crossing places for ephemeral land drainage as shown in Figure 2.

Hydraulic analysis has been performed to determine the required sizes of these culverts. Due to the close proximity of the rail tracks and SH1 downstream of the Emerald Glen extension, the influence from downstream culverts was investigated as part of the analysis.



Figure 1 Location of Emerald Glen Road Extension



Figure 2 Major & minor waterway crossings along Emerald Glen Road extension

2 Summary of Culvert Sizes

The following table summarises the size of culvert required for each waterway crossing based on the design process described in Section 4.

Culvert	Type	Size (m)	Design Storm (yr)
MacKays roundabout Crossing	Box	Twin 2.5 x 2.5	100
Emerald Glen Crossing	Box	2 x 1.5	100
Tramway Crossing	Circular	1.050	10
Minor Culvert 1	Circular	0.525	10
Minor Culvert 2	Circular	0.600	10

The water way crossing at MacKays roundabout, was initially proposed to be a culvert, however it is now proposed to be a bridge.

3 Hydrology (Design Flows)

Opus' specialist hydrologists have isolated the catchments for both the perennial streams and the land drainage areas. Multiple methods have been used to determine the flow for each catchment. Experience has then been used to judge which values are the most realistic portrayal of actual flows experienced within the watercourses. A summary of the flows is given below in Table 1. For a full explanation and analysis of flows, please see the hydrology report 'Emerald Glen Flow Regimes' (2010) located in Appendix A.

The expected increase in rainfall intensity due to climate change has been included in this analysis. The Ministry for the Environment's report 'Climate Change Effects and Impacts Assessment – A guidance manual for local government in New Zealand 2nd Edition' May 2008, was used to determine the expected increased rainfall in 2090.

Table 1 Flows For Each New Culvert

Culvert Name	Contributing Sub-catchments ¹	Catchment Area (ha)	10 year Flow Q_{10} 2090 (m ³ /s)	100 year Flow Q_{100} 2090 (m ³ /s)
Emerald Glen	Crossing 3	96	3.47	5.20
Mackays Crossing	Mackays Crossing	568	14.80	22.15
Tramway	Sub-catchment 1	8	0.48	0.71
Tramway	Sub-catchment 2	14	0.74	1.11
Tramway	Total	22	1.22	1.82
Minor Culvert 2	Sub-catchment 3	3	0.22	0.33
Minor Culvert 2	Sub-catchment 4	5	0.33	0.49
Minor Culvert 2	Sub-catchment 5	1	0.09	0.14
Minor Culvert 2	Total	9	0.64	0.96
Minor Culvert 1	Sub-catchment 6	2	0.16	0.23
Minor Culvert 1	Sub-catchment 7	3	0.22	0.33
Minor Culvert 1	Total	5	0.38	0.56

For a map of the major catchments in the area, see the hydrology report in Appendix A. For a map of the minor catchments, see Appendix B.

¹ These refer to the catchments/sub-catchments of the Hydrology Report

3.1 Bridge Considerations for MacKays Crossing

For the design of bridges, flows greater than the 100 year flow need to be considered. For a minor road, such as this, the 1000 year flow has been considered.

The flow record that we used to construct our synthetic flow record fits very closely to a natural log scale. Thus we have extrapolated the data we have, to a 1000 year return period on this basis.

For the MacKays Crossing stream, we have assessed the 1 in 1000 year flow to be 31m³/s. See Appendix E.

4 Selecting a level of service

4.1 For culverts

Kapiti Coast District Council's (KCDC) guiding document Subdivision and Development Principles and Requirements (2005), states that culverts must be designed to cater for the 10% Annual Exceedance Probability (AEP) (10 year return period) storm event. However it also states "culverts may need to be designed up to the 1% AEP (100 year return period) event depending on circumstances. This will be determined on a case by case basis."

The Emerald Glen Road extension is a local purpose road expected to have very low traffic volumes. In normal circumstances designing culverts to the 10 year event on such a road would be sufficient. However, due to the particular circumstances of the area, larger events have been used in some cases, as outlined below.

A house is located on the true right bank of the stream at the end of the existing Emerald Glen Road. In our opinion, it is important to ensure that the culvert to be installed on the Emerald Glen stream does not cause flooding to this property. We have therefore designed the Emerald Glen Crossing culvert to pass the 100 year flow.

The Mackays Crossing culvert is to be located within a constructed channel where the stream was diverted in 2004 during the construction of the Mackays Crossing intersection on SH1. A twin 2.5m x 2.5m box culvert allows the stream to pass beneath SH1. Both the box culvert and the constructed channel were designed to pass the 100 year flow and keep the flow within the stream banks. The secondary overflow path at this point has been estimated to be across the Mackays Crossing roundabout and through the underpass to Whareroa Road. To protect Mackays crossing roundabout to state highway standard (as it is an integral part of SH1), the new Mackays Crossing culvert has also been designed for a 100 year flow event, keeping all the flow within the stream banks.

The Tramway culvert has been designed to the 10 year event as we consider localised upstream ponding and the overtopping of the road as a secondary flow path during larger events as acceptable (and in line with KCDC standards). The depth of flow across the road during a 100 year event is expected to be less than 100mm, so emergency vehicles should be able to pass. This culvert is not considered to cause an increase in flooding or other significant impacts to property downstream.

The two minor land drainage culverts have also been sized to pass the 10 year flow without any significant upstream effects. However in larger events, as with the Tramway culvert, some localised upstream ponding or overtopping of the road may occur.

4.2 For Bridges

The waterway crossing at Mackays Crossing, was initially proposed to be a culvert. It is now proposed to be a bridge.

From the NZTA Bridge manual, the free board from the surface of the 100 year event should be 600mm (or 1200mm if the debris load is expected to be high) to the bottom of the

bridge deck. The 1000 year event should also be considered, such that the bridge deck does not get damaged.

We have assessed the land uses of the upstream catchment and consider the debris load to be relatively low. The upstream catchment consists of approximately 65% exotic grassland, 30% broad leaved indigenous hardwoods, 5% pine forest and a small amount of gorse and broom. The exotic grassland, indigenous hardwoods and gorse and broom, are not expected to generate a significant amount of large debris items that will block the culvert. The pine forest is a potential source of debris if the area is harvested however the percentage area covered by pine is very small, so we would not expect it to generate a sufficient quantity of large debris.

Thus the level of service for this bridge is 100 year plus 600mm to the underside of the bridge deck. It is likely that the 1000 year event will also pass under the bridge deck.

5 Hydraulic Methodology

5.1 Hydraulic Methodology for Major Culverts

Design Process

The three major culverts have been sized based on detailed hydraulic analysis. This process involved the following steps:

- Determining the flow regime (sub or super critical),
- Investigating the effects on the new culvert from downstream conditions,
- Based on these downstream conditions, finding the headwater depth assuming the culvert is inlet and outlet controlled,
- The greater headwater value between the inlet / outlet control is used as the expected headwater depth for the given flow.

This process was carried out for a variety of culvert sizes. The smallest culvert size which produced a reasonable headwater depth was then selected.

In the hydraulic analysis of these culverts, we assumed that the bottom 0.3 metres is to be filled with suitably sized gravel and baffles, the spacing of which is dependent on the grade. This is to provide a flow environment within the culvert which native fish can navigate.

Determination of Downstream Conditions

Due to the close proximity of the Emerald Glen Road extension to the main trunk railway and SH1, the downstream flow conditions of the new major waterway crossing culverts have been assessed. This assessment was undertaken to estimate the tailwater depth for the new culverts during the design storm events.

We used the computer programme *CivilTools V 1.1s*, to estimate the headwater depth produced at downstream culverts. We also estimated the depth of flow within the stream channel based on its geometry. The greater depth at the new culvert outlet between the flow depth and the headwater depth produced from downstream culverts was then used as the tailwater depth in hydraulic calculations for the new culvert. See Figure 3.

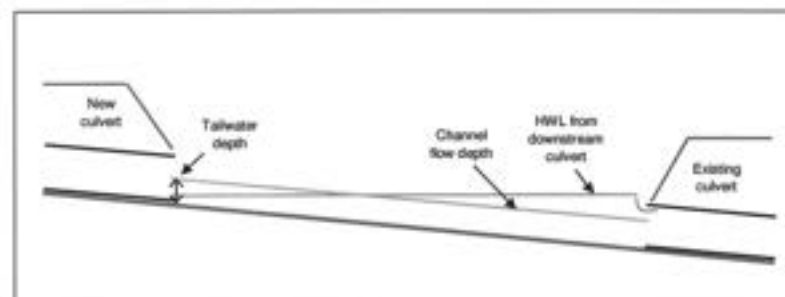


Figure 3 Finding tailwater depth from downstream conditions

5.2 Hydraulic Methodology of the Minor Land Drainage Culverts

The programme *CivilTools V 1.1s* has been used to determine a minimum culvert size to pass the 10 year flow. This size has then been increased by approximately 200 mm to account for the gravel in the base. The net effect is a conservative culvert size with a hydraulic capacity greater than the 10 year flow.

Although these culverts are not expected to be wet all the time, we are still proposing to place gravel in the bottom 200 mm of the culvert. This is to ensure that ecological connectivity can be maintained across the road.

5.3 Hydraulic Methodology for Bridge(s)

To determine the water level in the channel for several design events, we considered the channel hydraulics and the back water effect of the downstream structures. In this case, to determine the back water effect, we used the programme *hydrochan*. This allow us to analyse the levels as a gradually verging flow, rather than in a steady state condition. See Appendix F.

6 Results of Major Waterway Crossing Sizing

6.1 Mackays Crossing Bridge

The existing downstream box culvert has a back water effect on the water level at the bridge location. Approximate water levels including this backwater effect and climate change to 2090, are shown here (these water levels assume a manning roughness of 0.06).

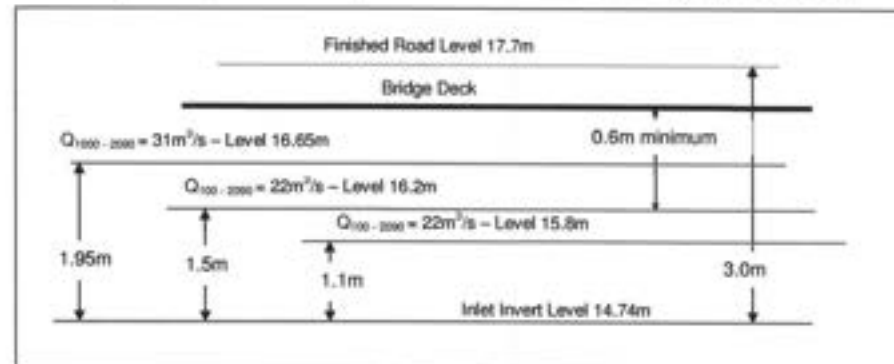


Figure 4 Water Levels in Mackay's Crossing channel at bridge location

6.2 Emerald Glen Culvert

A 2m wide and 1.5m high box culvert has been selected for the Emerald Glen crossing.

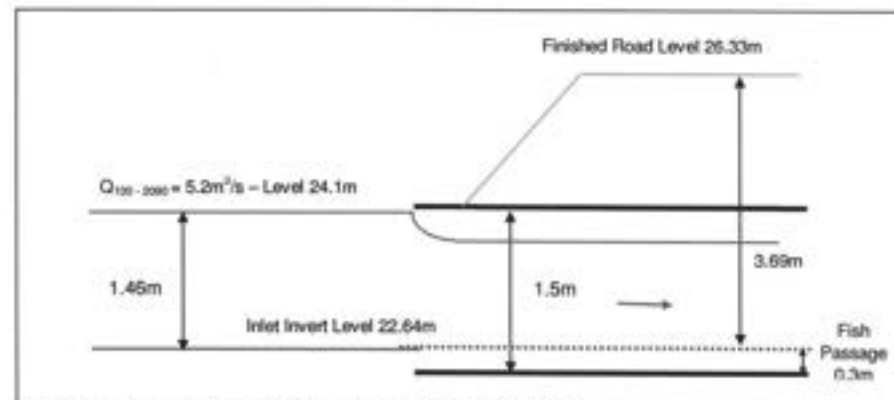


Figure 5 Head Water Level for $Q_{100}-2090$ for Emerald Glen Culvert

6.3 Tramway Culvert

A 1050 mm diameter round culvert has been selected for the Tramway crossing. The flow conditions during the 10 year and 100 year events are shown below in Figure 6.

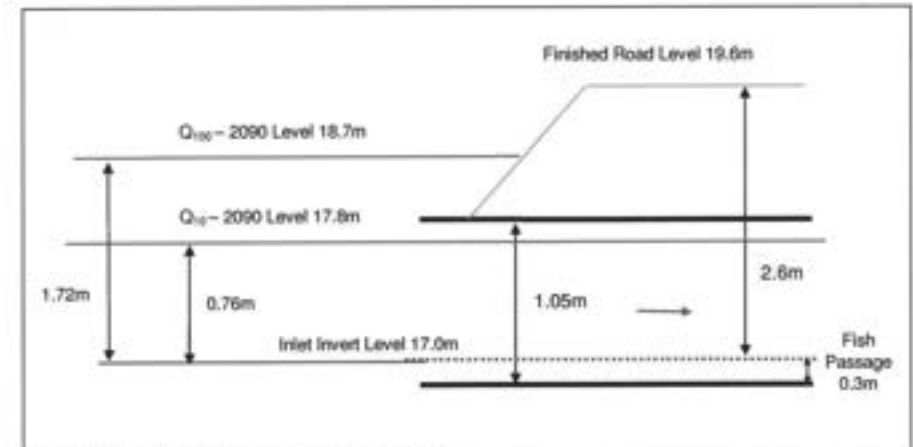


Figure 6 Head Water Level for Tramway Culvert

7 Physical works

See Appendix C for draft drawings showing location and details of the culverts.

7.1 Fish pass design

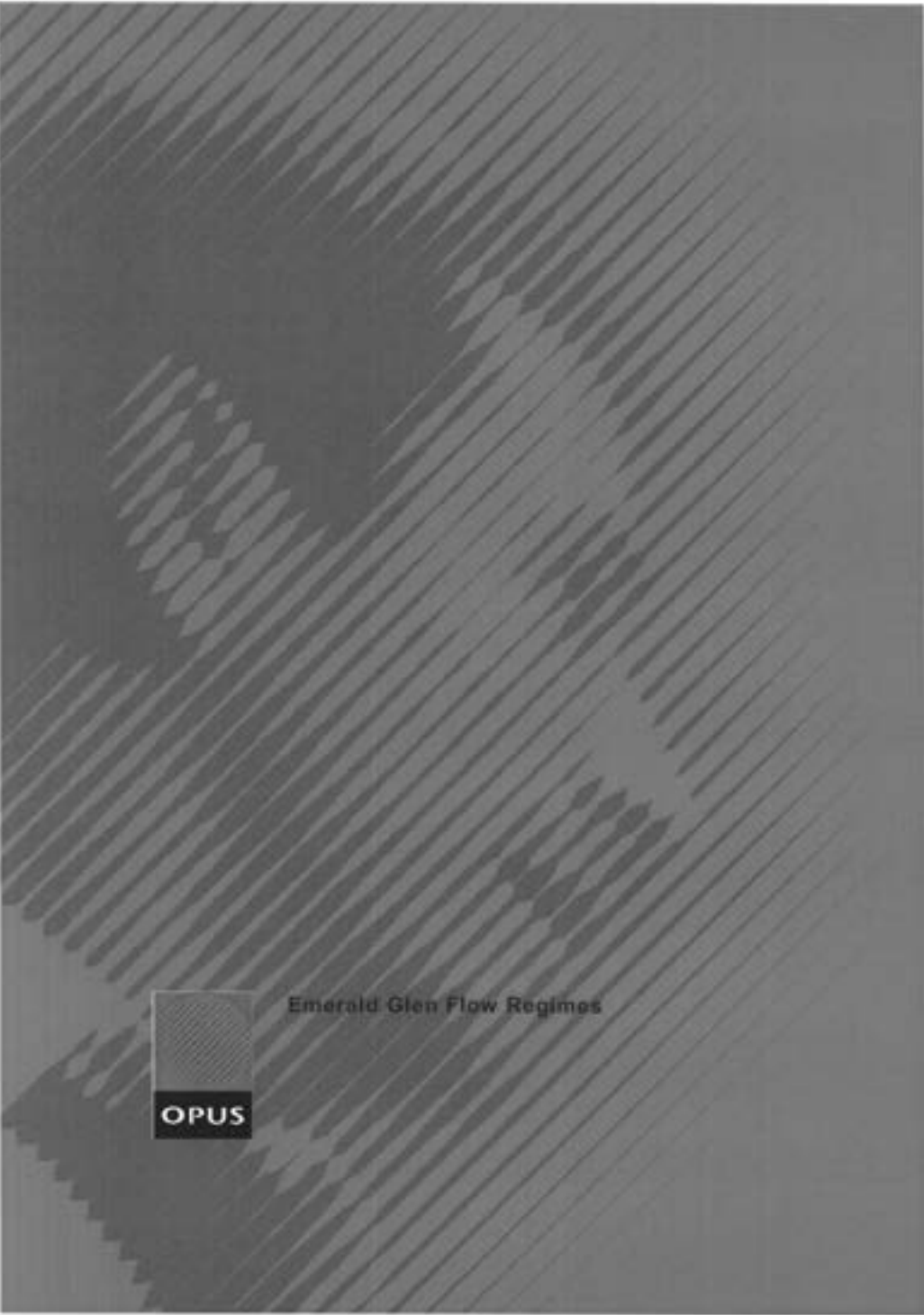
For most of the culverts we will follow the general GWRC (and other general) guidelines to design the fish pass through them.

The Northern culvert (Emerald Glen culvert) is at a grade of approximately 5%. As such we intend to deviate from the standard approach (of burying the invert of the culvert by 200mm). The approach we intend to use is a slot weir system where we can calculate the velocity and depth of the flow through the slot weir for a given flow.

In our opinion a reasonable flow range to work with is long term average flow for the mid 300 days in a typical year. As there is no flow record for this stream, we will use the flow record from an adjacent (or nearby) catchment and scale the flows to our catchment.

The flow depth and velocities we will design the slot weir system to, will depend on which fish species GWRC include in the consent conditions.

Appendix A - Hydrology report 'Emerald Glen Flow Regimes'



Emerald Glen Flow Regimes





Emerald Glen Flow Regimes

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Date: February 2010
Reference: 5c1744.00
Status: Final

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1 Introduction

NZTA is developing a strategy for designing and upgrading roading infrastructure on the Kapiti Coast. The local hydrology has the potential to affect the design of some of this infrastructure. Therefore an understanding of the flow regimes, particularly the frequency and magnitude of flood events of the streams and rivers in close proximity to any development is essential.

This report characterises the expected peak flows of the various streams in the vicinity of Emerald Glen Road (Figure 1). As there was no specific flow data from the study sites the REC database, Rational Method, and Regional Flood Frequency Method were used to derive peak flows for specific reaches of interest i.e., the Waterfall Road Crossing, Mackays Crossing, Crossings 2 and 3, and the smaller sub-catchments. The REC database obtains its values using the Regional Flood Frequency Method so it is expected that these two methods would give similar results. The catchments extend below Emerald Glen road as the REC values relate to the whole reach. The values provided in this report are therefore likely to be conservative i.e., slightly higher than expected.



Figure 1: Location of the study catchments near Emerald Glen Road.

2 River Environment Classification

The national River Environment Classification (REC) groups and classifies rivers, or parts of rivers, at six hierarchical levels. The location of each REC class is mapped so that the class of any reach of a river in New Zealand can be identified. REC classes discriminate variation in physical and biological characteristics at a range of spatial scales. Characteristics that are important for management such as hydrology, hydraulics, water quality, and biological communities are similar within classes; and significantly different between classes.

Each of the REC's six hierarchical classification levels is defined by a controlling factor: climate, source-of-flow, geology, land-cover, network-position and valley-landform. The REC represents rivers as networks of sections, including their upstream catchments. Each level of the REC hierarchy is based on differences in a set of processes that directly affect the physical and biological characteristics of rivers.

The REC can therefore be used to assess the spatial distribution of river resources and values. In particular, it allows the estimation of various flow parameters and attributes of catchments and river reaches for which no flow data exist. It is therefore possible to extract from the REC estimates of various flow parameters for the study streams. It is also possible to extract data for those sites for which there are flow data. This allows comparison of the accuracy and appropriateness of the data contained in the REC for a particular study area.

Estimated flow parameters for the sites of interest were therefore extracted from the REC database (Table 1). The small sub-catchments between Mackays Crossing and Crossing 3 cannot be estimated using the REC database as they do not contain a recognised waterway, and are therefore not included in the database.

Table 1: Flow characteristics predicted from the REC database.

Name	Area (km ²)	Q ₁₀₀ (m ³ /s)	Mean flow (l/s)	7-day MALF (l/s)
Waterfall Rd Crossing	2.78	8.33	57.06	6.08
Crossing 2	0.38	1.73	7.82	1.12
Crossing 3	0.87	3.66	17.98	2.44
Mackays Crossing	5.72	18.05	123.09	16.16

3 The Rational Method

The Rational Method is a probabilistic or statistical method and is used to estimate the peak flow for a given return period from the average rainfall intensity of the same return period.

The Rational Method is based upon the following formula:

$$QT = C I T A$$

where:

- QT = estimate of the peak rate of runoff for some recurrence interval, T
 C = runoff coefficient; fraction of rainfall, expressed as a dimensionless decimal that appears as surface runoff from the contributing drainage area
 IT = average rainfall intensity (mm/hr) for some recurrence interval, T, during that period of time equal to Tc
 A = the contributing tributary drainage area to the point of design
 Tc = time of concentration. The time it takes runoff from the furthest part of the catchment to reach the point of interest.

The runoff coefficient (C) represents the integrated effects of infiltration, evaporation, retention, flow routing, and interception; all of which affect the time distribution and peak rate of runoff.

Estimated flows for the sites of interest were therefore obtained using the Rational Method. The results are listed in Table 2.

Table 2: Flow characteristics predicted using the Rational Method.

Name	Area (km ²)	Q ₂ (m ³ /s)	Q ₁₀ (m ³ /s)	Q ₂₀ (m ³ /s)	Q ₅₀ (m ³ /s)	Q ₁₀₀ (m ³ /s)
Waterfall Rd Crossing	2.60	16.3	25.1	29.6	34.2	37.2
Crossing 2	0.29	3.6	5.5	6.5	7.3	8.2
Crossing 3	0.96	7.9	13.2	15.6	17.6	19.7
Mackays Crossing	5.88	33.8	51.8	61.2	70.5	76.9
Sub-catchment 1	0.08	0.7	1.1	1.3	1.5	1.6
Sub-catchment 2	0.14	0.7	1.4	1.6	2.0	2.2
Sub-catchment 3	0.03	0.3	0.5	0.6	0.7	0.8
Sub-catchment 4	0.05	0.5	0.8	1.0	1.1	1.3
Sub-catchment 5	0.01	0.1	0.2	0.2	0.2	0.3
Sub-catchment 6	0.02	0.2	0.4	0.4	0.5	0.5
Sub-catchment 7	0.03	0.3	0.5	0.6	0.7	0.8

These results appear to be significantly higher than can be realistically expected, especially at Mackays Crossing. It was therefore decided to provide an independent calibration or alternative approach. This is detailed in section 4.

4 Scaled Design Peak Floods

As the study sites do not have any flow data, the general relationship between catchment area and flow was considered. This assumes that given the same rainfall-runoff relationship a bigger catchment produces higher flows and vice versa. The flood peak estimates used for scaling were obtained by the Regional Flood Frequency Method during a previous investigation (Webby, 2004; Appendix A) (Table 3).

Table 3: Flows for Whareroa Stream taken at Mackays Crossing (6.13km²) predicted using the Regional Flood Frequency Method (Webby, 2004).

Return Period (yrs)	Flow (m ³ /s)
2.33	8.5
5	11.1
10	13.1
20	14.9
50	17.6
100	19.6

The 100-year design flood peaks were obtained for the study catchments by scaling the flood flows from the Whareroa Stream as a function of catchment area (Model 1, Table 4). A second technique was also used which raises the ratio of the two catchment areas to the power of 0.8 (Model 2, Table 4). Appendix B lists both the models for all the return periods. This is the approach recommended in the McKerchar and Pearson (1989) Regional Flood Frequency methodology when analysing flood frequencies.

Table 4: Peak Floods predicted using Model 1 and Model 2 to scale flows.

Name	Area (km ²)	Q ₁₀₀ (m ³ /s) Model 1	Q ₁₀₀ (m ³ /s) Model 2
Waterfall Rd Crossing	2.60	8.31	9.87
Crossing 2	0.29	0.93	1.71
Crossing 3	0.96	3.07	4.45
Mackays Crossing	5.88	18.80	18.96
Sub-catchment 1	0.08	0.26	0.61
Sub-catchment 2	0.14	0.45	0.95
Sub-catchment 3	0.03	0.10	0.28
Sub-catchment 4	0.05	0.16	0.42
Sub-catchment 5	0.01	0.03	0.12
Sub-catchment 6	0.02	0.06	0.20
Sub-catchment 7	0.03	0.10	0.28

5 Comparisons

Comparisons of the REC 'scaled flows', and Rational Methods show a significant difference in flood peaks (Table 5). The scaled flows from the Regional Flood Frequency Method are consistent with those from the REC database as expected. They appear much more realistic than the flows derived from the Rational Method. Scaling by A^{0.8} provides a little more conservatism to the estimated peak discharge.

Table 5: Comparison of the 1-in-100-year REC, Rational Method and scaled floods.

	REC (Q_{100})	Rational Method (Q_{100})	Synthetic Flows (Q_{100}) - Model 1	Synthetic Flows (Q_{100}) - Model 2
Mackays Crossing	18.10	76.90	18.80	18.96
Waterfall Rd	8.30	37.20	8.31	9.87
Crossing 2	1.70	8.20	0.93	1.71
Crossing 3	3.70	19.70	3.07	4.45

6 Concluding Comment

There is no instrumental flow record for Mackays Crossing or any of the other study sites. Scaling as a simple function of the ratio of catchment areas provides an estimate of the peak discharge of the 100-year event of 18.80m³/s. This is very close to the magnitude of this extreme event derived from the REC (18.10m³/s). However, the Rational Method predicted flows are 4 times higher than the REC predicted flood and 4 times greater than the scaled flood peaks.

The recommended methodology for scaling catchments as a function of area for flood frequency analysis is to raise area to the power of 0.8 (i.e., $A^{0.8}$). When applied to the current situation this would appear to give reasonable but still conservative estimates of peak flows. This is expected as the REC database uses the Regional Flood Frequency Method to estimate the flows.

Assuming that 18.96m³/s is a realistic estimate of the peak discharge for the 100-year event at Mackays Crossing, this can be scaled by 7.2% and 18.8% to allow for predicted climate change effects in 2040s and 2090s respectively (MfE, 2008). If a percentage increase in rainfall generates the same percentage in peak discharge, this would give peak discharges of 20.33 and 22.15m³/s for the 2040s and 2090s. Table 5 lists the predicted increase in flood peaks for the 2040s and 2090s for all the study catchments. Appendix B lists all the return periods and the associated values scaled for climate change.

Table 5: Flood peaks scaled to allow for climate change.

Name	Q_{100} (m ³ /s)	2040	2090
Waterfall Rd Crossing	9.87	10.58	11.53
Crossing 2	1.71	1.83	2.00
Crossing 3	4.45	4.77	5.20
Mackays Crossing	18.96	20.33	22.15
Sub-catchment 1	0.61	0.65	0.71
Sub-catchment 2	0.95	1.02	1.11
Sub-catchment 3	0.28	0.30	0.33
Sub-catchment 4	0.42	0.45	0.49
Sub-catchment 5	0.12	0.13	0.14
Sub-catchment 6	0.20	0.21	0.23
Sub-catchment 7	0.28	0.30	0.33

7 References

- McKerchar, A.I. and Pearson, C.P. (1989). Flood frequency in New Zealand, Christchurch, New Zealand.
- MfE (Ministry for the Environment), 2008. Climate change effects and impacts assessment, Wellington, New Zealand.
- Webby, G. (2004). Whareroa Stream at State Highway 1 – Flood flow estimation.

Whareroa Stream at State Highway 1 – Flood Flow Estimation

Flood frequency estimates for the catchment of the Whareroa Stream at SH1 (Mackay's Crossing) have been calculated by two methods.

1. Nearby flow data with area reduction ($A^{0.8}$) method using nearby catchments with long term flow records.
2. The Regional Flood Frequency Method.

1. Nearby Flow Data with Area Reduction

The nearby flow data with area reduction method applied here is a commonly used hydrological technique in New Zealand. Catchment areas for the Whareroa Stream and the nearby flow recorder catchments are factored down by a ratio of $A^{0.8}$ (area to the power of 0.8). Table 1 details the $A^{0.8}$ values for the recorder sites and the Whareroa catchment.

Table 1. Catchment Details

	Catchment Area (km ²)	Catchment Area ^{0.8}
Whareroa at SH1	6.13	4.27
Waikanae at WTP*	124.5	47.44
Wharemakau at Coastlands*	7.77	5.16
Pauatahanui at Gorge*	38.4	18.51

*Nearby gauging station

Flood frequency estimates for the three nearby recorder sites have been calculated as detailed in Table 2.

Table 2. Flood Frequency Estimates of Nearby Catchments (m³/s)

Catchment	Wharemakau Strm at Coastlands	Waikanae River at WTP	Pauatahanui Strm at Gorge
Area (km ²)	7.77	124.5	38.4

Return Period (yrs)	Flow (m ³ /s)		
2.33	6.7	151	40.3
5	9.9	199	51.4
10	12.7	238	60.5
2	15.5	274	69.2
50	19.0	323	80.4
100	21.7	359	88.8

Appendix A Flood Flow Estimation (Webby, 2004)

The ratios of $A^{0.2}$ between the Whareroa at SH1 catchments and the nearby recorder catchments are applied to the return period flows for the recorder catchments (Table 2) to derive a corresponding return period flow for each of the study catchments. Table 3 shows resulting return period flows derived for the Whareroa Stream at SH1.

2. Regional Flood Frequency Method

The Regional Flood Frequency Method is commonly applied to determine flood estimates in areas where there is no long-term flow record.

The method used is that described in *Flood Frequency in New Zealand* (McKerchar and Pearson 1989). Contour maps based on measured flow data have been derived that cover New Zealand. The catchments being investigated in this study are located off the contour maps and the respective coefficients are therefore used in equations involving catchment area to determine the design flood frequency estimates as detailed in column six of Table 3.

Table 3. Summary of Whareroa Stream at SH1 Flood Frequency Estimates

Return Period (yrs)	Based on Wharemauku Stream	Based on Waikanae River	Based on Pauatahanui Stream	Average of Nearby Catchments	Regional Flood Frequency Method
2.33	5.5	13.6	9.3	9.5	8.5
5	8.2	17.9	11.9	12.7	11.1
10	10.5	21.4	14.0	15.3	13.1
20	12.8	24.7	16.0	17.8	14.9
50	15.7	29.1	18.5	21.1	17.6
100	18.0	32.3	20.5	23.6	19.6

3. Recommended Flood Estimates

The Whareroa Stream does not entirely fit into one hydrological region and so a range of nearby gauging stations were selected.

The Waikanae River catchment rises in the Taranaki Ranges in a higher rainfall area than the Whareroa catchment. It is considered to over estimate the flood frequency for the Whareroa Stream.

The Wharemauku catchment is of a similar size to the Whareroa catchment and is geographically the closest of those analysed. The Wharemauku is also located in similar foothill topography and within the same rainfall band as the Whareroa catchment. Of the three nearby catchments analysed it is the most appropriate to use in calculation of flood frequency estimates for the Whareroa Stream.

However, the Wharemauku catchment has a long narrow shape with the main channel following a gently sloping floor. The elongated nature means the peak flows from tributaries, although peaking at the same time, will not concentrate in the main channel at the same place. This is opposed to the Whareroa catchment, which is more of a rounded shape, where the peak flows from all tributaries will concentrate in the main channel at the same time and place giving a higher flood peak.

The Pauatahanui catchment is further away but tends to closely confirm the average results.

Because of the variation in catchment shape between the Whareroa and Wharemauku streams a slightly conservative approach to using flood frequency estimates derived from the Wharemauku is to adopt the results from the Regional Flood Frequency Method. The recommended flood frequency estimates for the Whareroa Stream at SH1 are detailed in Table 4.

Table 4. Whareroa stream at SH1 recommended Flood Frequency Estimates

Return Period (years)	Flow (m ³ /s)
2.33	8.5
5	11.1
10	13.1
20	14.9
50	17.6
100	19.6

It should be noted, however, that there were major storms at nearby Paekakariki on 3 October 2003 and 17/18 February 2004. These events mobilised a lot of bed material and although the proposed State Highway 1 bridge over Whareroa Stream is not adjacent to the hills there may still be a debris load which may need factoring into the design. This needs verification by field inspection.

Appendix B Peak Floods and Climate Change

Emerald Glen Flow

Table B-1: Peak floods predicted using Model 1 and Model 2 to scale flows.

Name	Area (km ²)	Q _{1.10} (m ³ /s) Model 1	Q _{2.10} (m ³ /s) Model 2	Q ₅ (m ³ /s) Model 1	Q ₅ (m ³ /s) Model 2	Q ₁₀ (m ³ /s) Model 1	Q ₁₀ (m ³ /s) Model 2	Q ₂₀ (m ³ /s) Model 1	Q ₂₀ (m ³ /s) Model 2	Q ₅₀ (m ³ /s) Model 1	Q ₅₀ (m ³ /s) Model 2	Q ₁₀₀ (m ³ /s) Model 1	Q ₁₀₀ (m ³ /s) Model 2
Waterfall Rd Crossing	2.60	3.61	4.28	4.71	5.59	5.56	6.60	6.32	7.50	7.46	8.86	8.31	9.87
Crossing 2	0.29	0.40	0.74	0.53	0.97	0.62	1.14	0.70	1.30	0.83	1.53	0.93	1.71
Crossing 3	0.96	1.33	1.93	1.74	2.52	2.05	2.97	2.33	3.38	2.76	3.99	3.07	4.45
Mackays Crossing	5.88	8.15	8.22	10.65	10.74	12.57	12.67	14.29	14.41	16.88	17.02	18.80	18.96
Sub-catchment 1	0.06	0.11	0.26	0.14	0.35	0.17	0.41	0.19	0.46	0.23	0.55	0.26	0.61
Sub-catchment 2	0.14	0.19	0.41	0.25	0.54	0.30	0.64	0.34	0.72	0.40	0.86	0.45	0.95
Sub-catchment 3	0.03	0.04	0.12	0.05	0.16	0.06	0.19	0.07	0.21	0.09	0.25	0.10	0.28
Sub-catchment 4	0.05	0.07	0.18	0.09	0.24	0.11	0.28	0.12	0.32	0.14	0.38	0.16	0.42
Sub-catchment 5	0.01	0.01	0.05	0.02	0.07	0.02	0.08	0.02	0.09	0.03	0.10	0.03	0.12
Sub-catchment 6	0.02	0.03	0.09	0.04	0.11	0.04	0.13	0.05	0.15	0.06	0.18	0.06	0.20
Sub-catchment 7	0.03	0.04	0.12	0.05	0.16	0.06	0.19	0.07	0.21	0.09	0.25	0.10	0.28

Table B-2: Peak floods scaled to allow for climate change.

Name	$Q_{1\%}$ (m ³ /s)	2040	2090	Q_5 (m ³ /s)	2040	2090	Q_{10} (m ³ /s)	2040	2090	Q_{20} (m ³ /s)	2040	2090	Q_{50} (m ³ /s)	2040	2090	Q_{100} (m ³ /s)	2040	2090
Waterfall Rd Crossing	4.28	4.59	5.00	5.59	5.99	6.53	6.80	7.07	7.70	7.50	8.04	9.39	8.86	9.50	10.35	9.87	10.58	11.53
Crossing 2	0.74	0.79	0.86	0.97	1.04	1.13	1.14	1.22	1.33	1.30	1.39	1.62	1.53	1.64	1.79	1.71	1.83	2.00
Crossing 3	1.93	2.07	2.25	2.52	2.70	2.94	2.97	3.19	3.47	3.38	3.62	4.23	3.99	4.28	4.66	4.45	4.77	5.20
Mackays Crossing	8.22	8.81	9.60	10.74	11.51	12.54	12.67	13.58	14.80	14.41	15.45	18.05	17.02	18.25	19.88	18.96	20.33	22.15
Sub-catchment 1	0.26	0.28	0.31	0.35	0.37	0.40	0.41	0.44	0.48	0.46	0.50	0.58	0.55	0.59	0.64	0.61	0.65	0.71
Sub-catchment 2	0.41	0.44	0.48	0.54	0.58	0.63	0.64	0.68	0.74	0.72	0.78	0.91	0.86	0.92	1.00	0.95	1.02	1.11
Sub-catchment 3	0.12	0.13	0.14	0.16	0.17	0.18	0.19	0.20	0.22	0.21	0.23	0.26	0.25	0.27	0.29	0.28	0.30	0.33
Sub-catchment 4	0.18	0.19	0.21	0.24	0.25	0.28	0.28	0.30	0.33	0.32	0.34	0.40	0.38	0.40	0.44	0.42	0.45	0.49
Sub-catchment 5	0.05	0.05	0.06	0.07	0.07	0.08	0.08	0.08	0.09	0.09	0.09	0.11	0.10	0.11	0.12	0.12	0.13	0.14
Sub-catchment 6	0.09	0.09	0.10	0.11	0.12	0.13	0.13	0.14	0.16	0.15	0.16	0.19	0.18	0.19	0.21	0.20	0.21	0.23
Sub-catchment 7	0.12	0.13	0.14	0.16	0.17	0.18	0.19	0.20	0.22	0.21	0.23	0.26	0.25	0.27	0.29	0.28	0.30	0.33

**Appendix B – Hydrology Report (Emerald Glen Flow Regimes) Addendum – Maps Showing
Minor Catchments**

Figure 1 - Showing Minor Catchments as Hydrology Report.

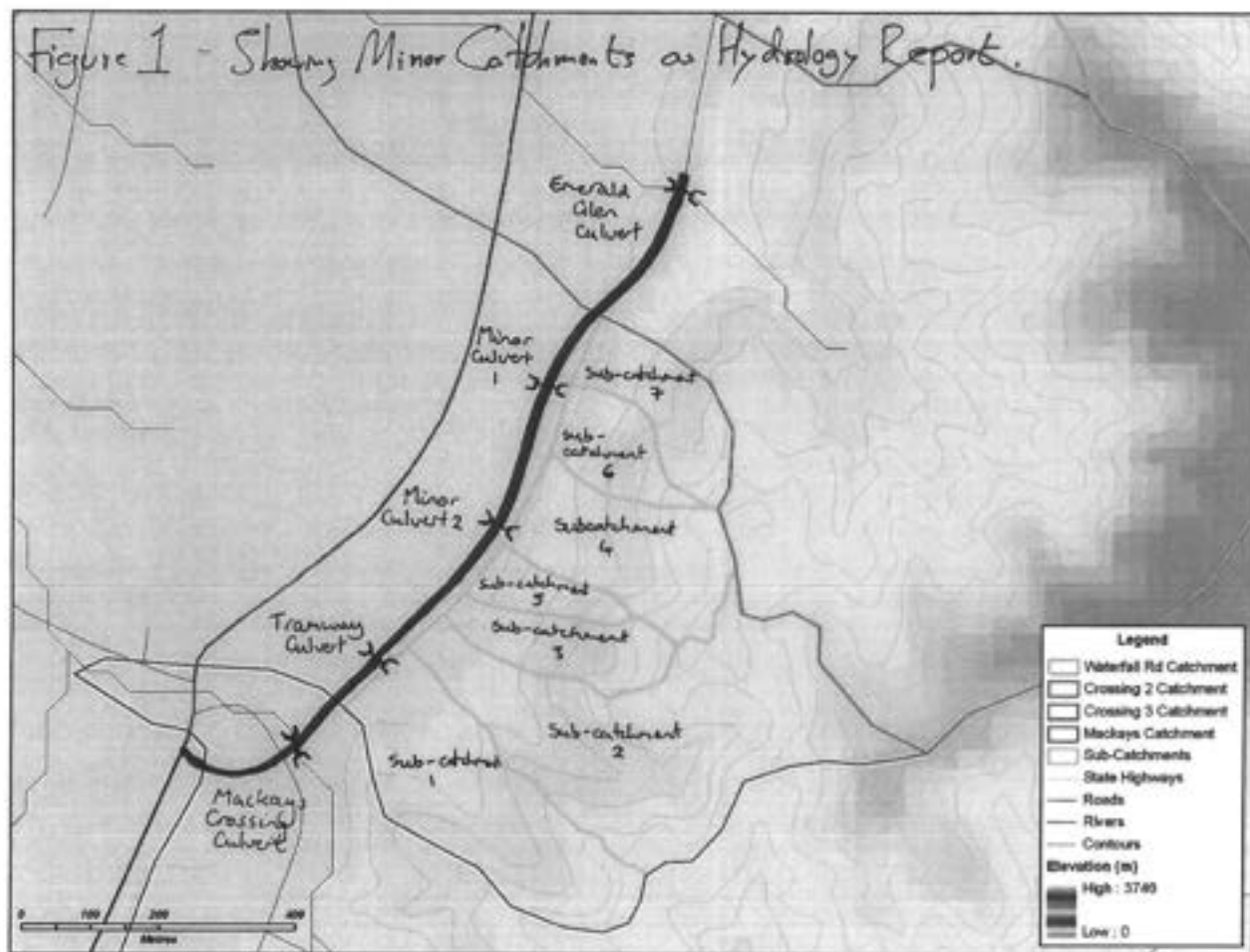
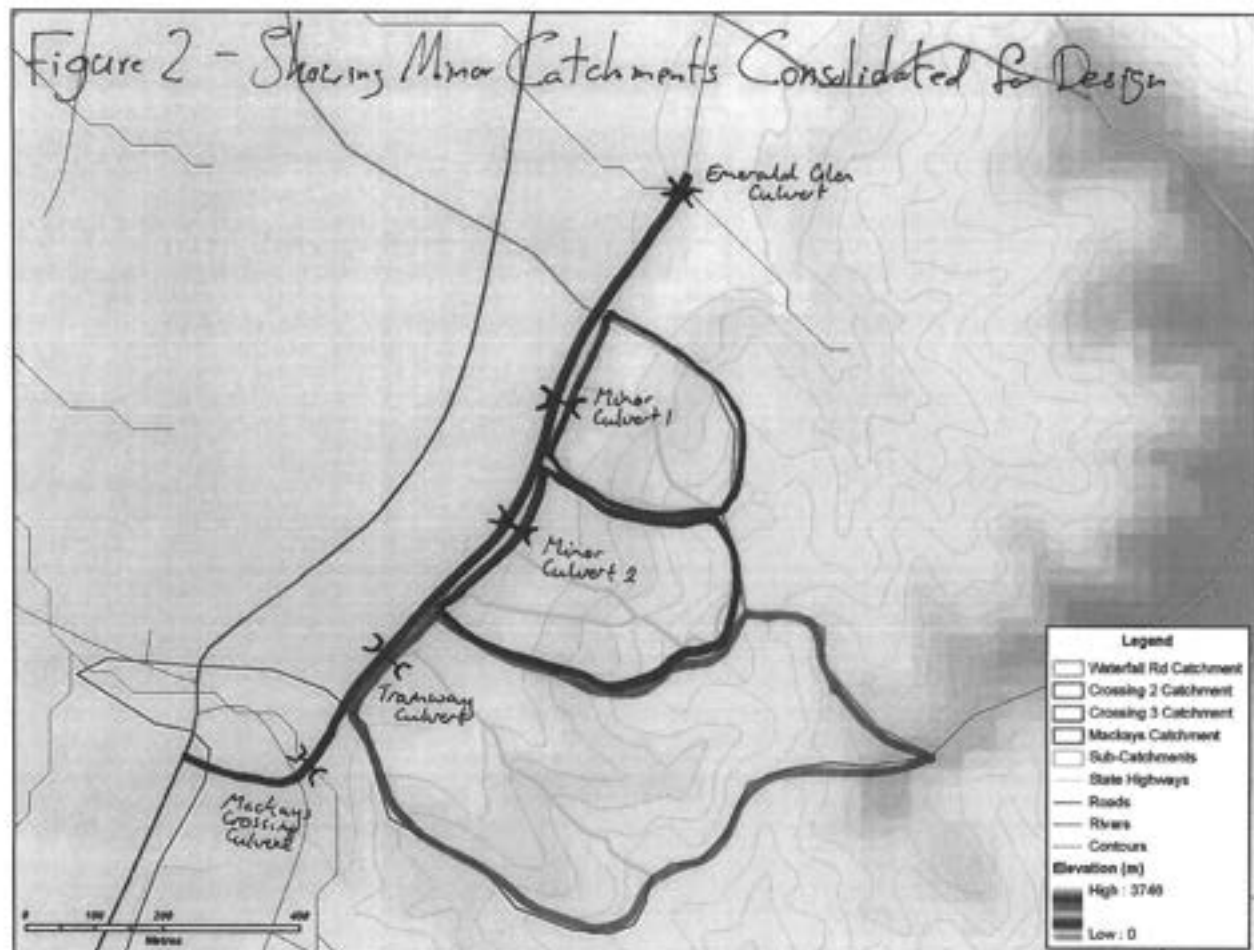
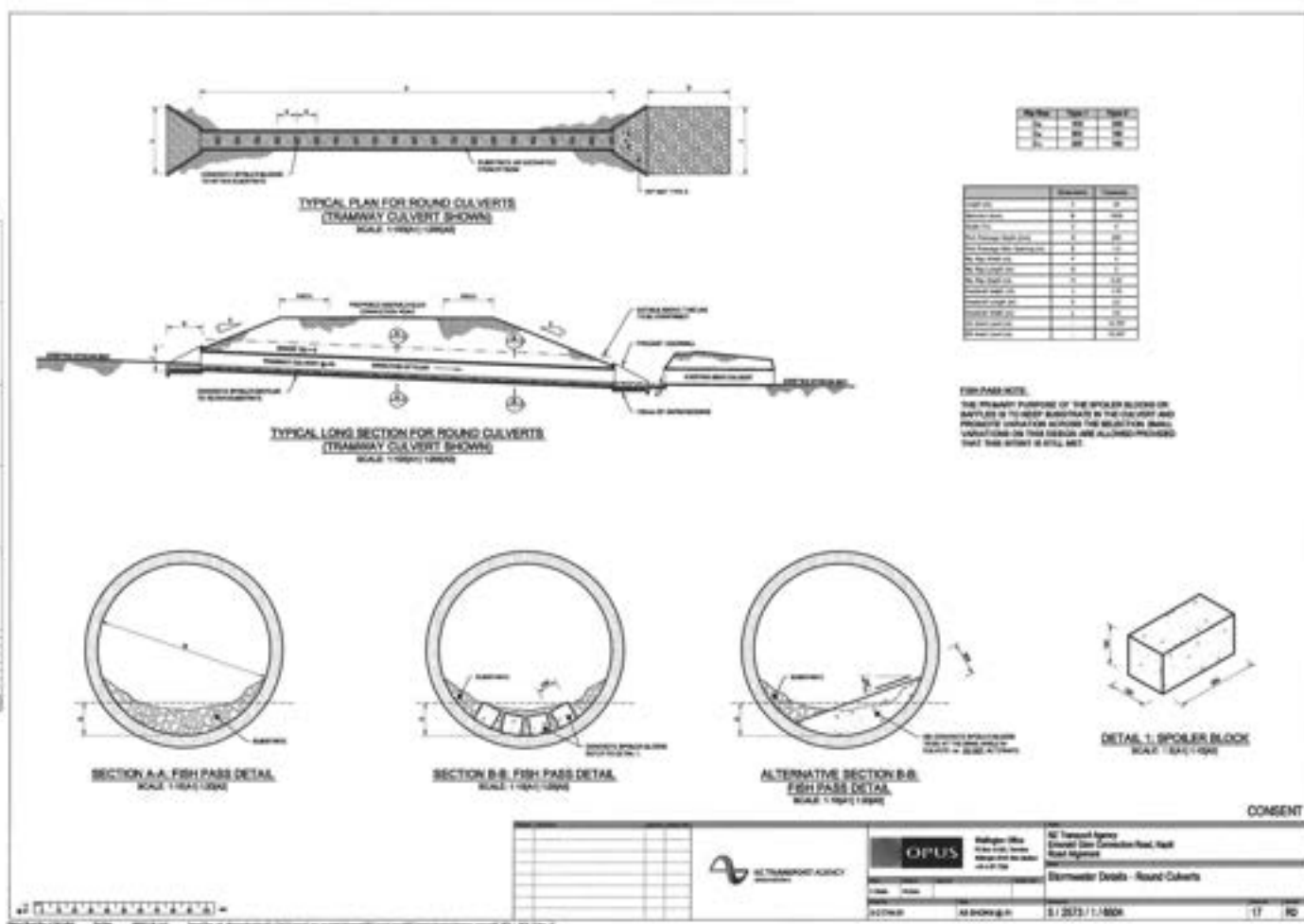
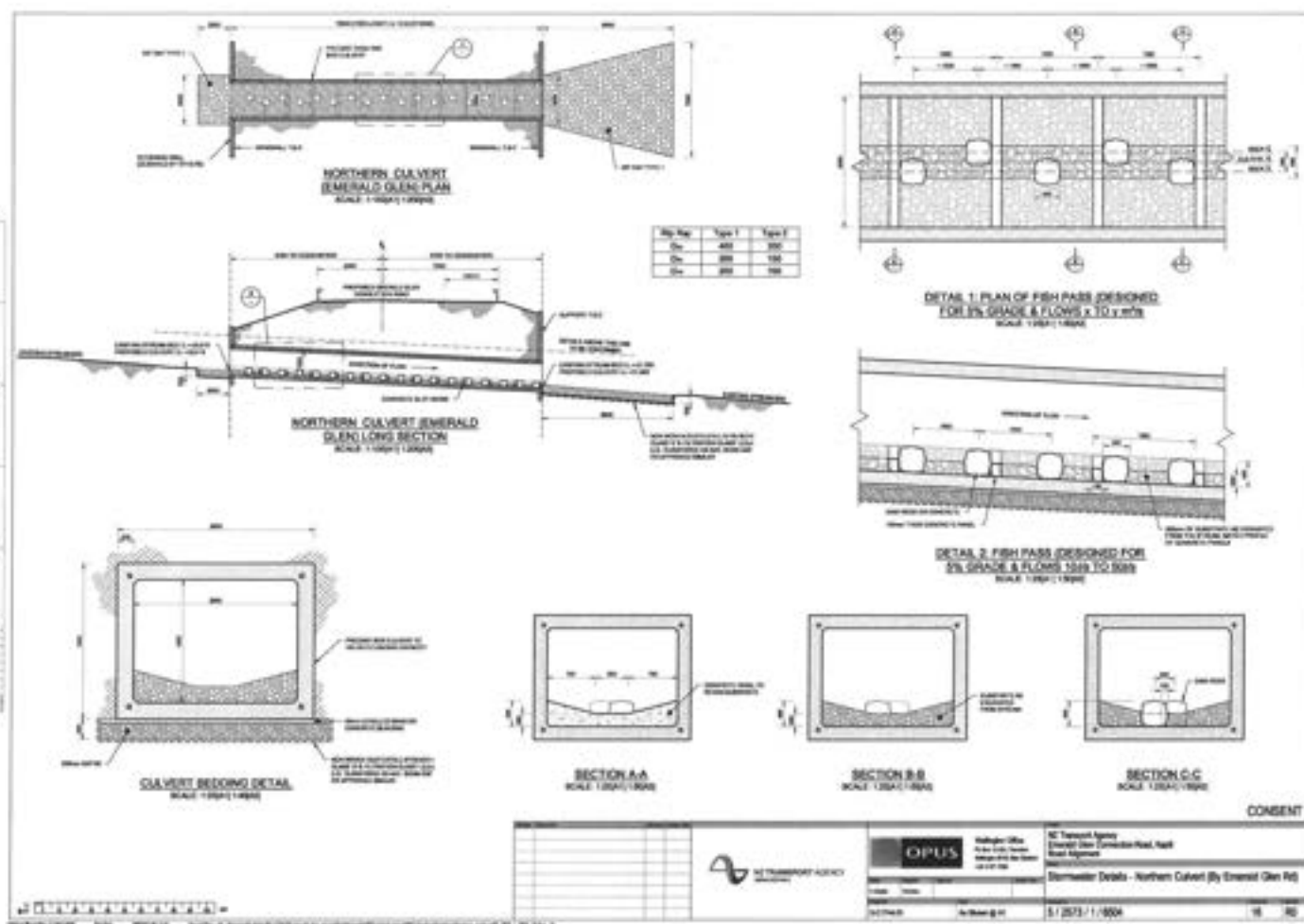


Figure 2 - Showing Minor Catchments Consolidated for Design



Appendix C – Drawings Showing Location and Details of Culverts for Consent Application



Appendix D – Culvert Sizing Calculations

Summary of Culvert Sizing

Culvert	Size	Downstream influence?	Inlet Invert RL (m)	Finished Road level RL (m)	Left Bank RL (m)	Right Bank RL (m)	Inlet Control WL RL (m)		Outlet Control WL RL (m)		Inlet or Outlet Control?
							10 yr	100 yr	10 yr	100 yr	
Emerald Glen	2mW x 1.5mD	No	22.64	26.7	24.1	24.2	23.72	24.10	NA	NA	Inlet
Mackays Crossing	5mW x 2.5mD	Yes	14.74	17.7	17.7	17.7	16.30	16.78	15.46	16.16	Inlet
Tramway	1050mm	Yes	17	19.6	18.4	18.46	17.76	17.91	17.66	18.72	Inlet 10yr

Culvert Capacity for Inlet Control Box Culvert Emerald Glen Culvert

Where: $Q = 2.3 \cdot C \cdot B \cdot H^{3/2} \cdot \sqrt{2 \cdot g \cdot H}$
(Based on orifice equation)

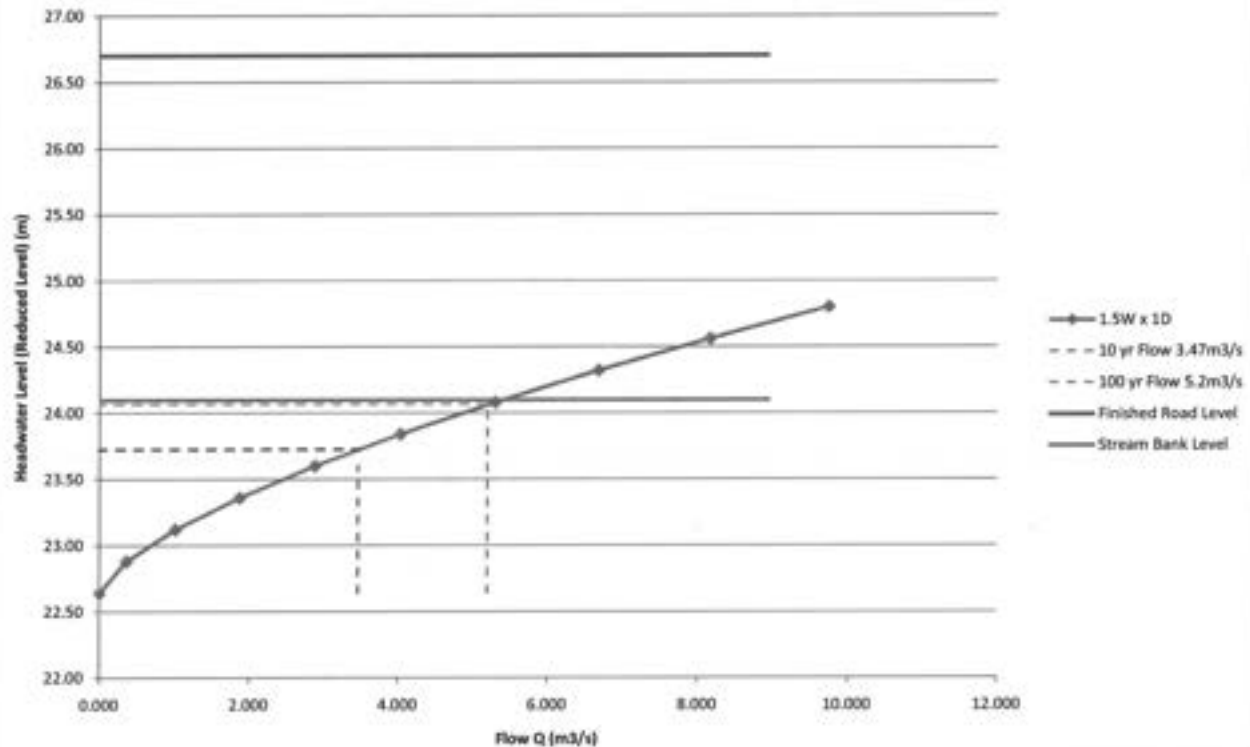
Note: as there are no culverts in the immediate area downstream of the new culvert and the grade is relatively steep (~8%), it was assumed that this culvert is inlet controlled

Procedure	
1	A range of values for H/D is first assigned where H is headwater level and D is culvert depth
2	Find headwater level H by multiplying H/D by the culvert depth D
3	Find the reduced level of headwater level by adding the reduced level of the Invert to H
4	Find the inlet capacity flow Q for the headwater level (H) using the equation above.
5	Plot the inlet capacity flow against the headwater level to determine where head water level is in relation to the road level and stream banks.

Dimensions	
Culvert 2m wide x 1.5m deep	
B (width)	9.61 m/s2
D (depth)	2m
Deckfall	1.5m
Ch (coefficient)	1.2m
RL Invert level	0.9
	22.64m
*10.3m of bottom assumed to be used for fish passage)	

Working			
H/D	H (m)	RL (m)	Q (m3/s)
0.00	0.00	22.64	0.000
0.20	0.24	22.88	0.361
0.40	0.49	23.12	1.021
0.60	0.72	23.36	1.875
0.80	0.96	23.60	2.897
1.00	1.20	23.84	4.034
1.20	1.44	24.08	5.303
1.40	1.68	24.32	6.692
1.60	1.92	24.56	8.194
1.80	2.16	24.80	9.742

Emerald Glen Inlet Capacity (Inlet Control)



Culvert Capacity for Outlet Control Box Culvert Mackays Crossing Culvert

Procedure

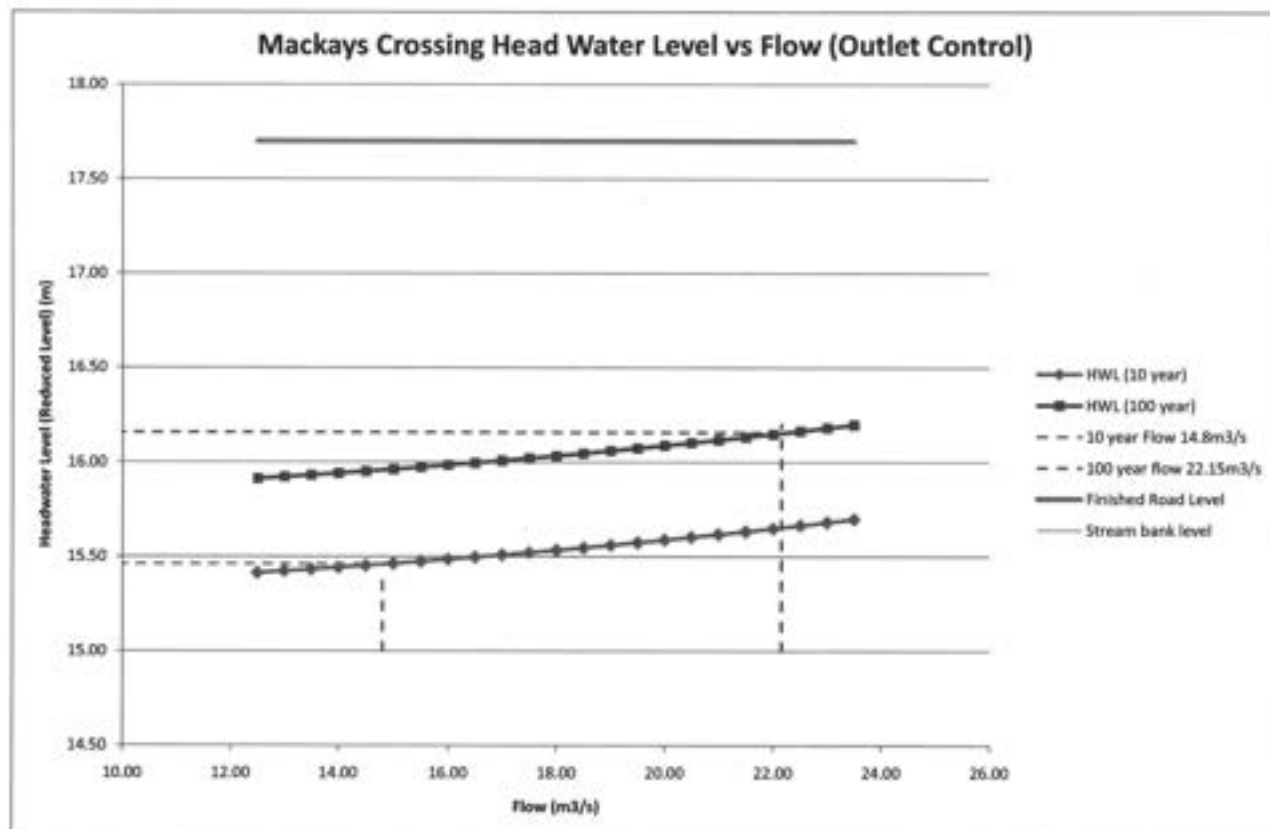
- 1 A range of values for inlet flow Q is first assigned
- 2 The flow velocity is then determined by $V=Q/A$ where A is cross sectional area of culvert
- 3 Determine velocity head $= V^2/(2 \times g)$ where $g=9.81 \text{ m}^2/\text{s}$
- 4 Determine losses due to friction $h_f = f \times (L / (4 \times R)) \times V_{\text{head}}$
- 5 Determine inlet losses $H_i = V_{\text{head}} \times K_i$
- 6 Determine outlet losses $H_o = V_{\text{head}} \times K_o$
- 7 Determine total head losses $H_{\text{tot}} = h_f + H_i + H_o$
- 8 Determine the headwater level HWL for the 100 yr and 10 yr flows where $\text{HWL} = \text{Tailwater level} + \text{Total losses}$

Culvert Dimensions

5m wide x 2.5m deep

		Notes/Assumptions
Width B	5 m	
Depth D	2.5 m	
Depth actual D_{actual}	2.2 m	Assuming bottom 0.3m is filled with gravel for fish passage
Pipe Area A	11.00 m ²	Cross sectional area $A=B \times D_{\text{actual}}$
Wetted Perimeter P	14.40 m	Assuming pipe is flowing full
Hydraulic Radius R	0.76 m	$R=A/P$
Coef. of friction f	0.06	
K_i	0.2	(inlet loss coefficient)
K_o	1	(outlet loss coefficient)
Length L	26 m	
Manning's n	0.035	
RL Mackays Outlet	14.25 m	
10 yr Tailwater	1.05 m	(Civil Tools used to estimate downstream flow depth from downstream culvert)
100 yr Tailwater	1.55 m	(Civil Tools used to estimate downstream flow depth from downstream culvert)
RL Mackays TW 100yr	15.80 m	
RL Mackays TW 10yr	15.30 m	

Working								
Flow Q (m ³ /s)	Velocity v (m/s)	Velocity Head V_{head} (m)	Friction Loss h_f (m)	Inlet Loss H_i (m)	Outlet loss H_o (m)	Total Loss H_{tot} (m)	HWL 100yr (m)	HWL 10yr (m)
12.50	1.14	0.066	0.03	0.013	0.066	0.11	15.91	15.41
13.00	1.18	0.071	0.04	0.014	0.071	0.12	15.92	15.42
13.50	1.23	0.077	0.04	0.015	0.077	0.13	15.93	15.43
14.00	1.27	0.083	0.04	0.017	0.083	0.14	15.94	15.44
14.50	1.32	0.089	0.05	0.018	0.089	0.15	15.95	15.45
15.00	1.36	0.095	0.05	0.019	0.095	0.16	15.96	15.46
15.50	1.41	0.101	0.05	0.020	0.101	0.17	15.97	15.47
16.00	1.45	0.108	0.06	0.022	0.108	0.18	15.98	15.48
16.50	1.50	0.115	0.06	0.023	0.115	0.20	16.00	15.50
17.00	1.55	0.122	0.06	0.024	0.122	0.21	16.01	15.51
17.50	1.59	0.129	0.07	0.026	0.129	0.22	16.02	15.52
18.00	1.64	0.136	0.07	0.027	0.136	0.23	16.03	15.53
18.50	1.68	0.144	0.07	0.029	0.144	0.25	16.05	15.55
19.00	1.73	0.152	0.08	0.030	0.152	0.26	16.06	15.56
19.50	1.77	0.160	0.08	0.032	0.160	0.27	16.07	15.57
20.00	1.82	0.168	0.09	0.034	0.168	0.29	16.09	15.59
20.50	1.86	0.177	0.09	0.035	0.177	0.30	16.10	15.60
21.00	1.91	0.186	0.09	0.037	0.186	0.32	16.12	15.62
21.50	1.95	0.195	0.10	0.039	0.195	0.33	16.13	15.63
22.00	2.00	0.204	0.10	0.041	0.204	0.35	16.15	15.65
22.50	2.05	0.213	0.11	0.043	0.213	0.36	16.16	15.66
23.00	2.09	0.223	0.11	0.045	0.223	0.38	16.18	15.68
23.50	2.14	0.233	0.12	0.047	0.233	0.40	16.20	15.70



Culvert Capacity for Inlet Control Box Culvert Mackays Crossing Culvert

Where: $Q = 2/3 \cdot C \cdot B \cdot H \cdot \sqrt{2/3 \cdot g \cdot H}$
(Based on orifice equation)

Procedure

- 1 A range of values for H/D is first assigned where H is headwater level and D is culvert depth
- 2 Find headwater level H by multiplying H/D by the culvert depth D
- 3 Find the reduced level of headwater level by adding the reduced level of the invert to H
- 4 Find the inlet capacity flow Q for the headwater level (H) using the equation above.
- 5 Plot the inlet capacity flow against the headwater level to determine where head water level is in relation to the road level and stream banks.

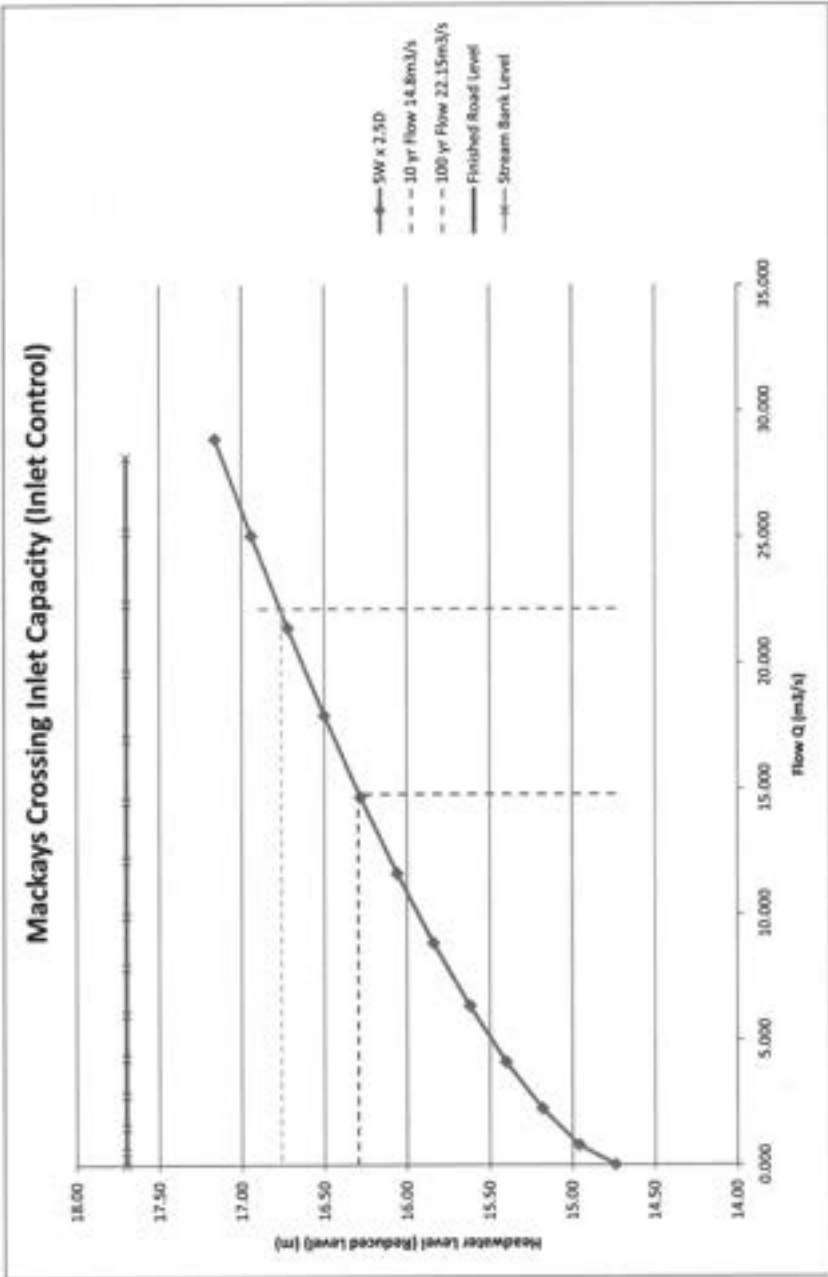
Dimensions

Culvert 5m wide x 2.5m deep

g	9.81	m/s ²	Assuming 0.3m at base is used for fish passage
B	5	m	
D	2.5	m	
Dactual	2.2	m	
Cb	0.9		
RL invert inlet	14.74	m	

Working

H/D	H (m)	RL (m)	Q (m ³ /s)
0.00	0.00	14.74	0.000
0.10	0.22	14.96	0.792
0.20	0.44	15.18	2.239
0.30	0.66	15.40	4.114
0.40	0.88	15.62	6.333
0.50	1.10	15.84	8.851
0.60	1.32	16.06	11.635
0.70	1.54	16.28	14.662
0.80	1.76	16.50	17.913
0.90	1.98	16.72	21.375
1.00	2.20	16.94	25.035
1.10	2.42	17.16	28.882



Culvert Capacity for Inlet Control Round Culvert **Tramway Culvert Inlet Control**

Where: $Q = C^*A*\sqrt{2*3*g*H}$
 (Orifice equation)

Procedure

- 1 A range of values for H/D is first assigned where H is headwater level and D is culvert depth
- 2 Find headwater level H by multiplying H/D by the culvert diameter D
- 3 Find the reduced level of headwater level by adding the reduced level of the invert to H
- 4 Find the inlet capacity flow Q for the headwater level (H) using the equation above.
- 5 Plot the inlet capacity flow against the headwater level to determine where head water level is in relation to the road level and stream banks.

Dimensions

1050 mm diameter

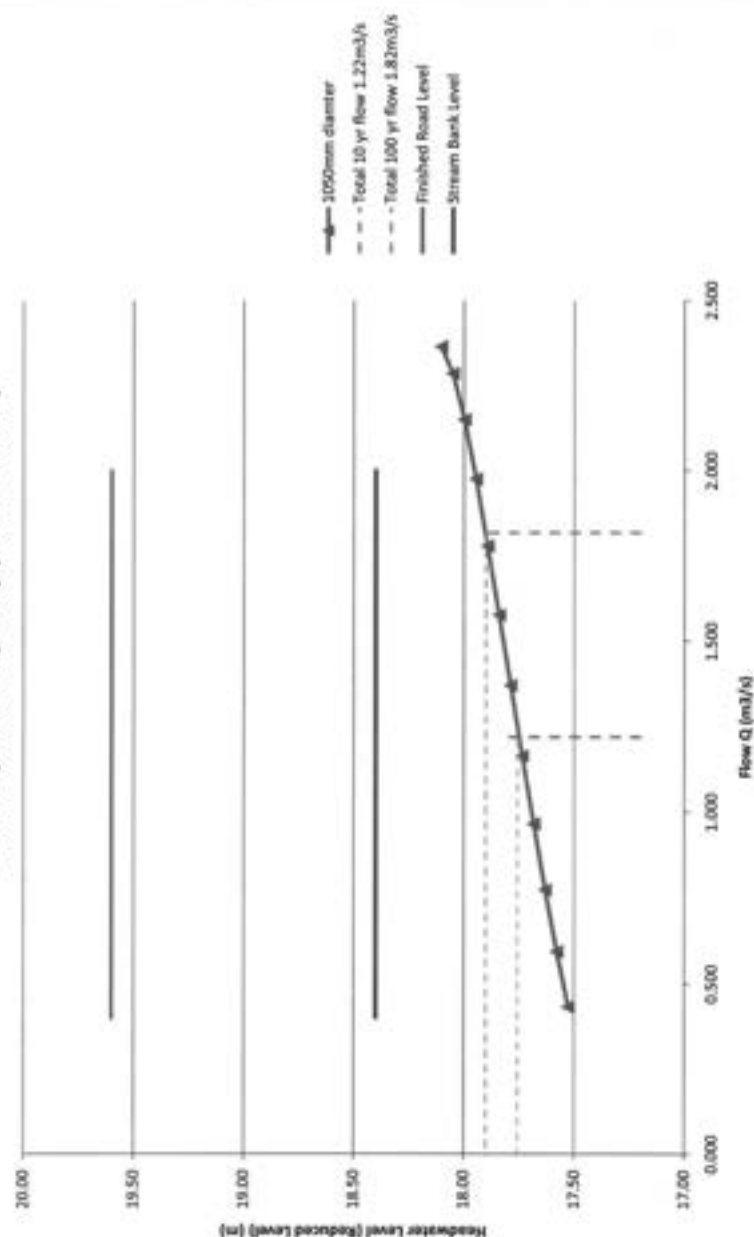
g	9.81	m/s ²	
Diameter D	1.05	m	
Fish Pass area	0.204	m ²	Assuming 0.3m of base filled used for fish passage
Total Area A	0.661	m ²	
Cd	0.9		
RL invert	17	m	
H	y2+y3	m	(y2 = 0.3m fish passage allowance)



Working

H/D	H (m)	Theta (rads)	y3 (m)	Area A (m ²)	RL (m)	Q (m ³ /s)
0.00	0.00	0.00	-0.30	-0.20	17.00	#NUM!
0.10	0.11	1.29	-0.20	-0.46	17.11	#NUM!
0.20	0.21	1.85	-0.09	-0.68	17.21	#NUM!
0.30	0.32	2.32	0.02	0.01	17.32	0.007
0.35	0.37	2.53	0.07	0.07	17.37	0.068
0.40	0.42	2.74	0.12	0.12	17.42	0.165
0.45	0.47	2.94	0.17	0.17	17.47	0.288
0.50	0.53	3.14	0.23	0.23	17.53	0.433
0.55	0.58	3.34	0.28	0.28	17.58	0.596
0.60	0.63	3.54	0.33	0.34	17.63	0.775
0.65	0.68	3.75	0.38	0.39	17.68	0.966
0.70	0.74	3.96	0.44	0.44	17.74	1.166
0.75	0.79	4.19	0.49	0.49	17.79	1.371
0.80	0.84	4.43	0.54	0.54	17.84	1.578
0.85	0.89	4.69	0.59	0.58	17.89	1.781
0.90	0.95	5.00	0.65	0.62	17.95	1.975
0.95	1.00	5.38	0.70	0.65	18.00	2.150
1.00	1.05	6.28	0.75	0.66	18.05	2.285
1.05	1.10	6.28	0.80	0.66	18.10	2.364

Tramway Inlet Capacity (Inlet Control)



Culvert Capacity for Outlet Control Round Culvert
Tramway Culvert Outlet Control

Procedure

1. A range of values for inlet flow Q is first assigned
2. The flow velocity is then determined by $V=Q/A$ where A is cross sectional area of culvert
3. Determine velocity head $= V^2/(2 \times g)$ where $g=9.81 \text{ m/s}^2$
4. Determine losses due to friction $h_f = f \times (L / (4 \times R)) \times V_{\text{head}}$
5. Determine inlet losses $H_i = V_{\text{head}} \times K_i$
6. Determine outlet losses $H_o = V_{\text{head}} \times K_o$
7. Determine total head losses $H_{\text{tot}} = h_f + H_i + H_o$
8. Determine the headwater level HWL for the 100 yr and 10 yr flows where $\text{HWL} = \text{Tailwater level} + \text{Total losses}$

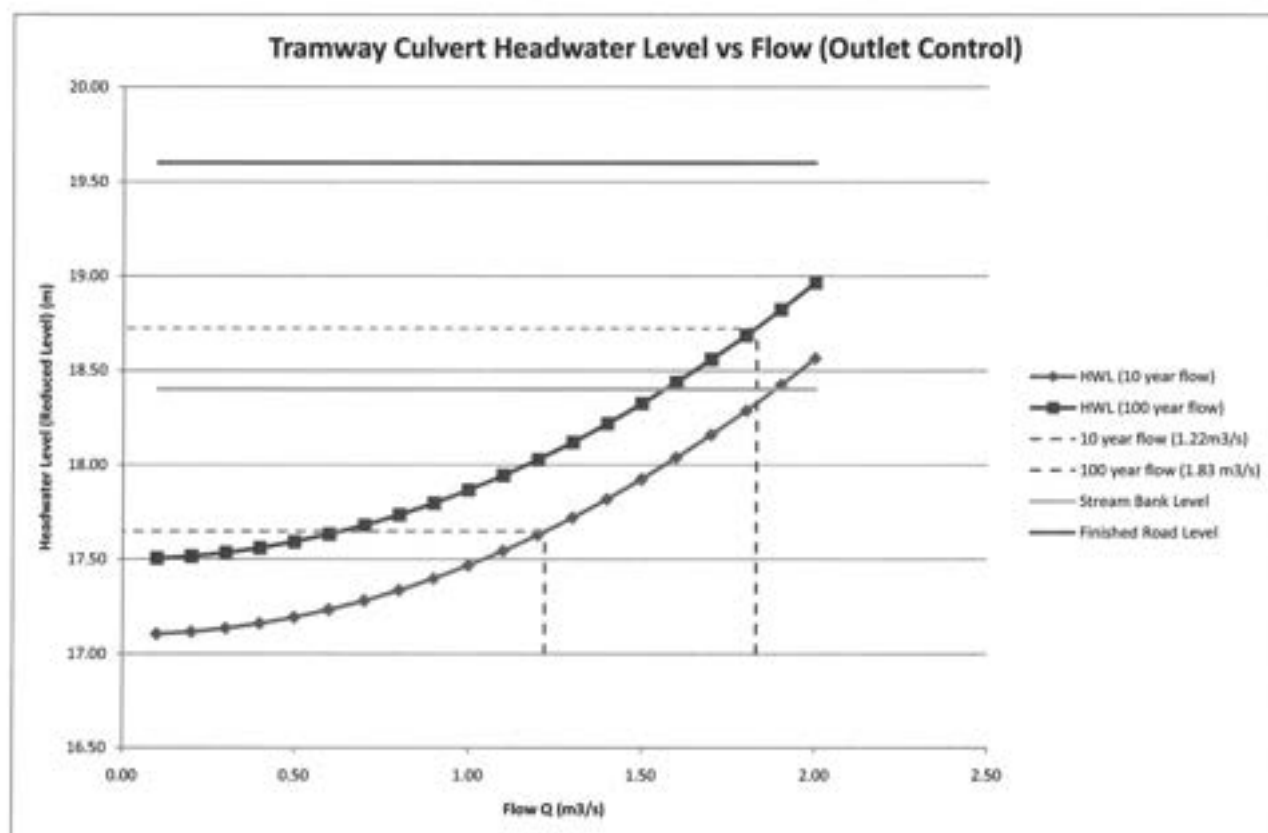
Culvert Dimensions

1050mm

Diameter	1.05 m	
Fish pass area a	0.204 m	Assuming bottom 0.3m filled for fish passage
Pipe Area A	0.66 m ²	Pipe area - fish passage area
Wetted Perimeter P	3.30 m	Assuming pipe flowing full
Hydraulic Radius R	0.20 m	
Coef. of friction f	0.06	
K_i	0.2	(inlet loss coefficient)
K_o	1	(outlet loss coefficient)
Length L	26 m	
Manning's n	0.025	
RL SubC 1 Outlet	15.90 m	
100 yr TW depth	1.60 m	Civil Tools used to estimate tailwater depth caused by downstream culvert
10 yr TW depth	1.20 m	Civil Tools used to estimate tailwater depth caused by downstream culvert
RL Tramway TW 100yr	17.50 m	
RL tramway TW 10yr	17.10 m	

Working

Flow Q (m ³ /s)	Velocity v (m/s)	Velocity Head V_{head} (m)	Friction Loss h_f (m)	Inlet Loss H_i (m)	Outlet loss H_o (m)	Total Loss H_{tot} (m)	HWL 100yr (m)	HWL 10yr (m)
0.10	0.15	0.001	0.00	0.000	0.001	0.00	17.50	17.10
0.20	0.30	0.005	0.01	0.001	0.005	0.01	17.51	17.11
0.30	0.45	0.010	0.02	0.002	0.010	0.03	17.53	17.13
0.40	0.60	0.019	0.04	0.004	0.019	0.06	17.56	17.16
0.50	0.75	0.029	0.06	0.006	0.029	0.09	17.59	17.19
0.60	0.91	0.042	0.08	0.008	0.042	0.13	17.63	17.23
0.70	1.06	0.057	0.11	0.011	0.057	0.18	17.68	17.28
0.80	1.21	0.075	0.14	0.015	0.075	0.23	17.73	17.33
0.90	1.36	0.094	0.18	0.019	0.094	0.30	17.80	17.40
1.00	1.51	0.116	0.23	0.023	0.116	0.37	17.87	17.47
1.10	1.66	0.141	0.27	0.028	0.141	0.44	17.94	17.54
1.20	1.81	0.168	0.33	0.034	0.168	0.53	18.03	17.63
1.30	1.97	0.197	0.38	0.039	0.197	0.62	18.12	17.72
1.40	2.12	0.228	0.44	0.046	0.228	0.72	18.22	17.82
1.50	2.27	0.262	0.51	0.052	0.262	0.82	18.32	17.92
1.60	2.42	0.298	0.58	0.060	0.298	0.94	18.44	18.04
1.70	2.57	0.337	0.65	0.067	0.337	1.06	18.56	18.16
1.80	2.72	0.377	0.73	0.075	0.377	1.19	18.69	18.29
1.90	2.87	0.421	0.82	0.084	0.421	1.32	18.82	18.42
2.00	3.02	0.468	0.91	0.093	0.468	1.46	18.96	18.56

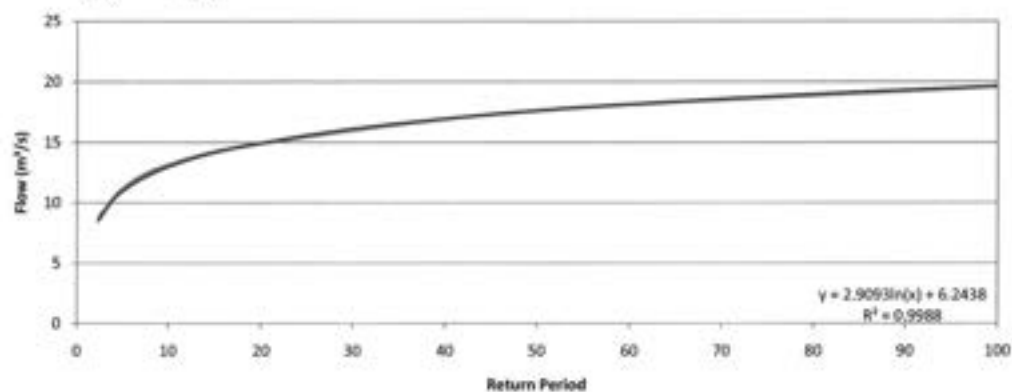


Appendix E Mackays Crossing flow Extrapolation

Determining the Q200, 500 and 1000 at Mackays Crossing

- Extrapolating the flow, from the Whareroa Stream flood report by Opus (Grant Webby) 2004

2.33	8.5
5	11.1
10	13.1
20	14.9
50	17.6
100	19.6



	ARI	Flow (m³/s)	Climate Change	
			2040	2090
Extrap	200	21.6	23.16	25.23
Extrap	500	24.32	26.07	28.41
Extrap	1000	26.34	28.24	30.77

Appendix F Mackays Crossing Backwater effect assessment

Results :

Q (cms)	15.000	22.000	31.000
TW El. (m)	15.500	16.000	16.500
Normal Depth (m)	0.977	1.211	1.462
Critical Depth (m)	0.722	0.914	1.125

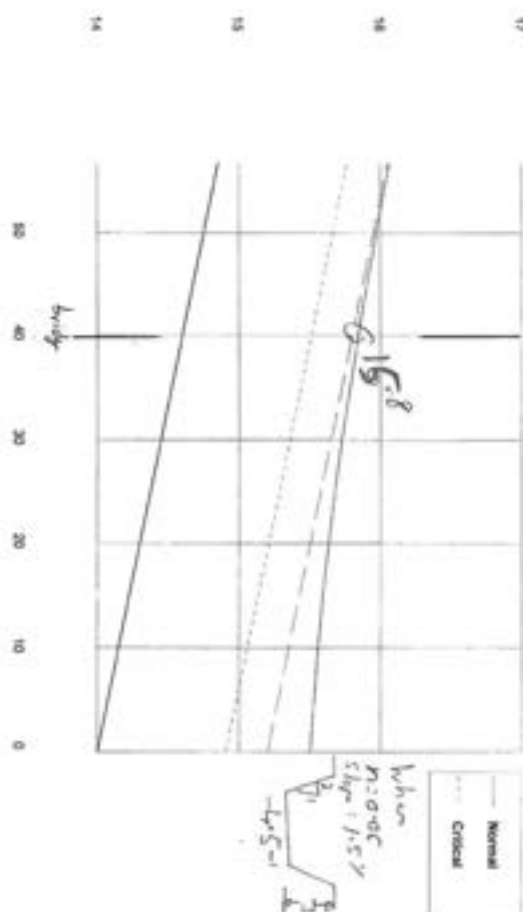
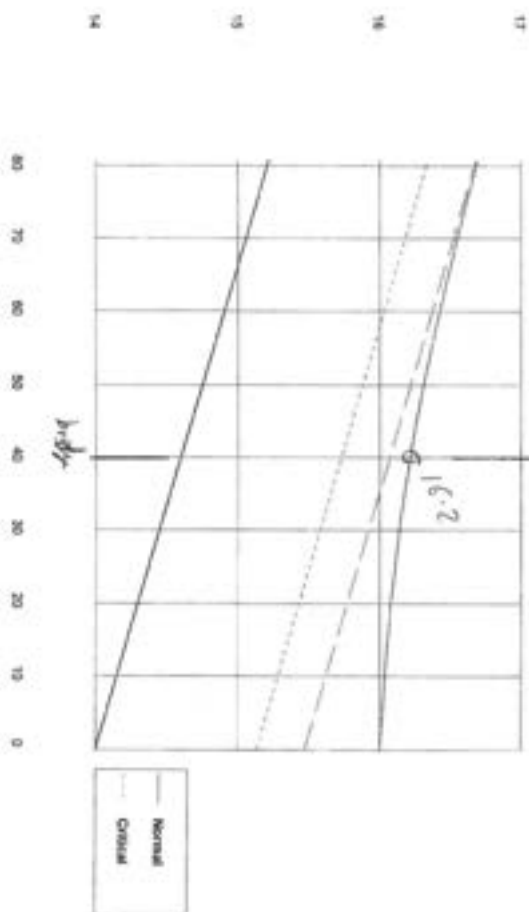
When

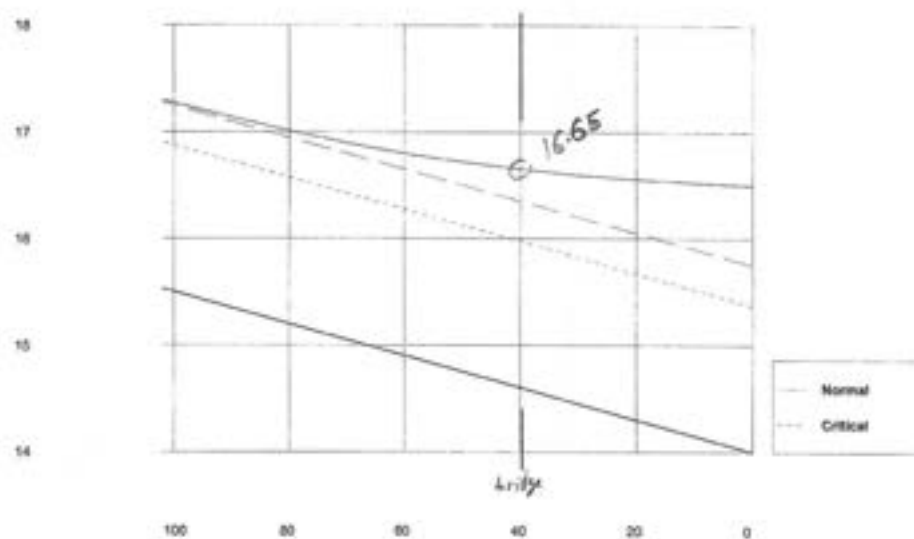
$$h = 0.00$$

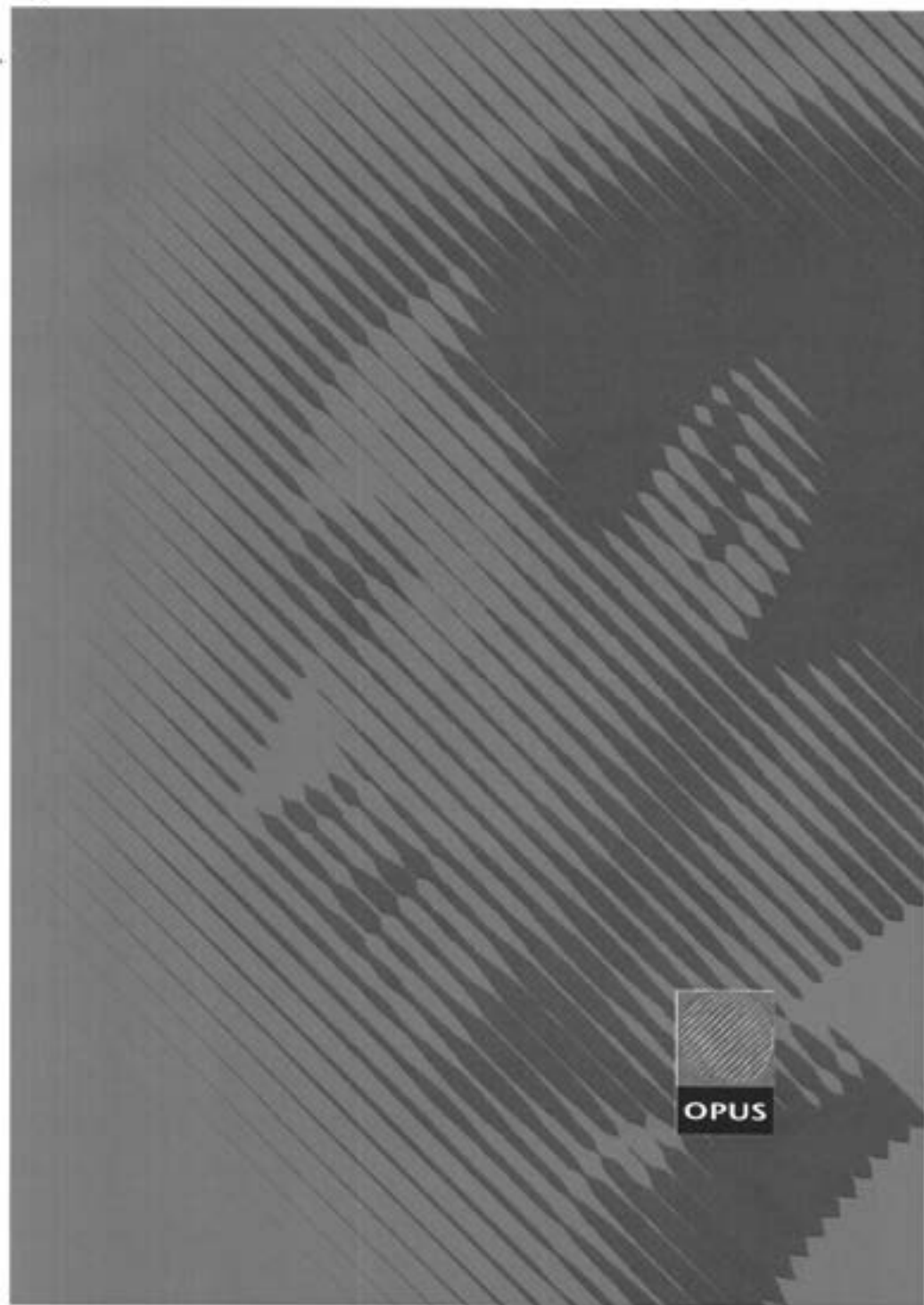
$$S_{hy} = 0.015$$

Profiles :

Q = 15.000		Q = 22.000		Q = 31.000	
X (m)	El. (m)	X (m)	El. (m)	X (m)	El. (m)
0.000	15.500	0.000	16.000	0.000	16.500
4.000	15.509	10.000	16.016	10.000	16.513
8.000	15.519	20.000	16.036	20.000	16.529
12.000	15.531	30.000	16.069	30.000	16.550
16.000	15.545	40.000	16.110	40.000	16.578
20.000	15.561	50.000	16.167	50.000	16.614
24.000	15.580	60.000	16.242	60.000	16.662
28.000	15.601	70.000	16.339	70.000	16.725
32.000	15.628	80.000	16.455	80.000	16.804
36.000	15.655	90.000	16.584	90.000	16.903
40.000	15.687	95.532	16.656	100.000	17.018
44.000	15.724			110.000	17.145
48.000	15.764			118.376	17.252
52.000	15.809				
56.000	15.854				
60.000	15.907				
64.000	15.960				
68.000	16.012				
71.627	16.062				







16th September, 2010

Leigh Sage
Building Consents Officer
Kapiti Coast District Council
175 Rimu Road
Private Bag 601
Paraparaumu 5254



5-C1744.03.WE02.65

Dear Leigh

NZTA: Emerald Glen Connection - Retaining Walls - Building Consent Approvals

Thank you for your time talking through the retaining wall designs proposed for NZTA's Emerald Glen Connection project.

General

There are two proposed retaining walls, which will support the proposed road above the proposed Emerald Glen Culvert, around Station 140.

Fence

From our preliminary meeting we have made alterations to the pedestrian fence, along the footpath, at the top of the retaining wall. The fence now complies with Clause F4 of the New Zealand Building Code (as can be seen on drawings sheet 303r2, and we have instructed the contractor to use the fence 'a' from figure 2 of Acceptable Solution F4/AS1.

Public access

During the works: The site will be part of a larger formal construction site. As such the public will not have access.

After the works: The public will be able to drive over the road, which is supported by the two retaining walls. The design includes road barriers either side of the road and a fence on the retaining wall side of the foot path. The public would need to climb over these barriers to gain access to the retaining walls.

Land ownership

A certificate of title has not been included in this application, as there is no certificate of title. This is because the proposed retaining walls are located within an existing KCDC road reserve (although there is no road there yet). There is also no Gazetted reference as this road was not acquired under the public works act but vested to Council in 1972, see survey drawing number SO33688 which is attached.

Supporting information

The attached information provided for the retaining walls includes the following:

- Completed building consent application (1 copy);
- Application Fee (1 copy);
- Producer statements PS1(2 copies);
- Locality plan and construction drawings (2 copies);
- Design calculations for the walls (2 copies);
- Specifications for the walls (2 copies);
- Hydraulics report for the culvert (2 copies);
- Shop drawings from Humes, for the manufacturing details (2 copies);
- Breakdown of construction costs from our engineers' estimate (2 copies);
- Copy of survey plan SO33688 (2 copies);
- E-mail to contractor regarding selection of pedestrian fence (2 copies).

Application fee

I have included a cheque for \$1,350.00 as advised by Michelle Salmon, based on our construction estimate of circa \$200,000.

Timing

Please can you process this consent at you earliest convenience. This would be greatly appreciated as the contractors need to start work very soon, in order that this road can be opened before the Waterfall Road railway crossing is closed, so the double tracking of the railway can operate safely.

Contact

If you have any inquiries or require further information about this application, please do not hesitate to contact me on DDI (04) 471 7120.

Yours sincerely



Richard Coles
Environmental Engineer.

Richard Coles

From: Curtis Lee [Curtis.Lee@opus.co.nz]
Sent: Thursday, 9 September 2010 15:00
To: 'Gerald Carty [FCC-CivEng]'
Cc: 'Wayne Holcroft'; 'Andrew Quinn'
Subject: FW: Emerald Glen - Fence on Northern Culvert
Attachments: F4-safety-from-falling.pdf

Gerald

I'd like to confirm that the type of timber fence along the edge of the footpath over the northern culvert, should be the same as shown on Figure 2 (a) in the Building Code attached.

If you need any further information just let me know. I shouldn't think we'd have to create a drawing for this fence.

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Attached is a copy of the building code which talks about these fence types.

When I get chance I will have a read through this code and let you know which fence we should go for.

But in the meantime, it might be worth you doing the same.

Thanks

Curtis



Curtis Lee
Team Leader Transportation Engineering Design

Opus International Consultants Ltd

Curtis.Lee@opus.co.nz

Tel +64 4 471 7012, Fax +64 4 471 1397

<http://www.opus.co.nz>

Level 7 Majestic Centre, 100 Willis St, PO Box 12 003, Wellington, New Zealand



Green Terramesh Retaining Wall

1.1 Scope

This specification covers the procedures and requirements for the construction of the Green Terramesh retaining wall at the northern culvert of the proposed Emerald Glen Road extension.

The Contractor shall supply all materials, plant, equipment, labour and supervision necessary to complete the specified work as detailed on the Drawings.

1.2 Materials

Green Terramesh

The Green Terramesh units are prefabricated from a single panel of wire mesh with an additional welded steel mesh at the facing and triangular supporting bracket. Opening and pivoting of the triangular supporting brackets will create a 70° face angle.

The material (including zinc coated wires, PVC coating, wire mesh gabions, lacing wire, internal stiffeners, steel mesh and fasteners) properties, requirements, tolerances, and fabrication of the Terramesh System and woven mesh gabions shall be in accordance with the latest product standard Specification issued by the manufacturer Maccaferri Inc.

Unit size of the gabions shall be in accordance with the construction Drawings.

HDPE Geogrid

Geogrid shall be Tensar HDPE geogrid as specified in the Drawings or approved equivalent.

Erosion Control Mat

Biomac Grasstrike 300 or equivalent approved shall be used as the erosion control mat at the wall face.

Reinforced Backfill Materials

(a) Rock Fill

The rock fill material in the gabion basket shall be hard, unweathered, free from fissures and flaking, angular to round, durable rock and of such quality that they shall not disintegrate on exposure to water or weathering during the life of the structure.

The material shall have unit weight of at least 24 kN/m³. The rocks shall range between 100 mm and 200 mm. The range in sizes shall allow for a variation of 5% oversize and/or 5% undersize rock, provided it is not placed on the gabion exposed surface. Rock fill shall not be more than 10% passing a 75 mm aperture sieve opening.

(b) Structural Fill Material

Structural fill material within the reinforced zone for the reinforced soil wall shall be obtained from natural sources. It shall be free from organic or otherwise deleterious materials (timber, metal, bricks, paper etc) and shall contain no material retained on a 75 mm sieve, and for which the material passing a 19 mm sieve when tested in accordance with NZS 4402: 1986 Test 6.1.1 has a soaked CBR greater than 20.

Compaction

The in situ density shall not be less than 98% of the maximum dry density (MDD) for that material as determined by Standard Compaction Test 4.1.1 of NZS 4402:1986 for each set of tests being the average of five separate field measurements. No separate field density measurement shall be less than 95%.

Internal Friction Angle

Material shall exhibit an effective angle of internal friction of not less than 36°, as determined by large diameter triaxial testing or approved equivalent on the portion of material finer than the 20 mm, utilizing a sample of material compacted to 98 percent of Heavy Compaction NZS 4402: 1988; Test 4.1.2. Sampling for triaxial testing shall be carried out in accordance with NZS4407: 1991.

Soundness

The material shall be free of shale or other soft, poor durability particles.

Drainage Filter Material

All material used for the filter around subsoil drain pipes (wrapped in geotextile) shall comply with TNZ F/6 unless otherwise specified.

When tested in accordance with NZS 4407:1991, Test 3.8.1, the drainage aggregate shall comply with the following gradings:

<u>Test Sieve Aperture</u>	<u>Percentage Passing</u>
20 mm	100
1.5 mm	5 max

Geotextile

The geotextile shall be a non-woven geotextile meeting the requirements of TNZ F/7 for Strength Class C and Filtration Class 1 (i.e. Bidim A29 or equivalent approved).

1.3 Acceptance of Material

The Contractor shall ensure that all the materials specified comply with the applicable contract specifications and Maccaferri Specifications.

1.4 Excavation for Green Terramesh

Excavation shall be carried out in such a manner that the surface is neatly cut, free draining over its length. The Contractor shall be responsible for the design and construction of any temporary cut and/or lateral support required during construction.

The final founding level shall be approved by a qualified Engineer.

1.5 Foundation Preparation

The foundations for the walls shall be graded level. Any surface irregularities, soft or loose materials, and vegetation shall be removed and replaced by compacted granular fill during the preparation of the foundation.

The final foundation platform shall be approved by a Geotechnical Engineer before construction of the Green Terramesh wall system.

1.6 Green Terramesh combined with Geogrid

- (i) Geogrid shall be oriented with the highest strength axis perpendicular to the wall face alignment.
- (ii) Geogrid reinforcement shall be placed at the lengths and elevations shown on the construction Drawings.
- (iii) The geogrid shall be attached to the Terramesh Green units by lacing or stainless steel rings. The geogrid shall be pulled tight prior to backfill placement on the geogrid.
- (iv) Geogrid reinforcements shall be continuous throughout their embedment lengths and placed side-by-side to provide 100% coverage at each level. Spliced connections between shorter pieces of geogrid or gaps between adjacent pieces of geogrid are not permitted.
- (v) Install the Green Terramesh units as described in Clause 1.7. Once the first row of Terramesh Green units are placed in position, the units shall be connected through the geogrid layer along the front reinforced bar using lacing or stainless steel rings. Refer to the manufacturer's specifications for ring connection spacing.

1.7 Green Terramesh System Wall Installation

- (i) Place the Green Terramesh units in their positions. Alignment and level shall be checked in all directions and insure that all gabion units are in full contact with the base and properly seated.
- (ii) Raise the unit's facing, turn the triangular stiffening brackets perpendicular to the facing and connect to base panel with lacing wire or steel fasteners.
- (iii) Roll out the erosion mat and place the erosion mat through the triangular brackets located behind the front face of the Green Terramesh unit and fix top and bottom to the mesh face. Slitting of the erosion mat in line with the brackets is required to ensure that it extends horizontally back against the horizontal mesh by at least 400 mm.
- (iv) Place approved backfill up to 300 mm of the face and compact to the required degree of compaction as stated in Clause 2.1. Compaction is to proceed parallel to the slope face, ensuring that the compacting machine does not come within 1 m of the face element. Lighter compaction equipment should be used towards the front face zone.
- (v) Topsoil with suitable seed mix shall be placed in the 300 mm zone of the structure, behind the Green Terramesh face. The top soil should be firmly compacted by plate or small machine compactor. Overfill topsoil zone by 5 % to allow for some local settlement of the top soil.
- (vi) The erosion mat and 1 m long Green Terramesh tieback is folded back into the slope ready for the next unit. The next row of Green Terramesh units shall be laid out as previously described directly on the lower units. The upper and lower units shall be all connected along the front reinforced bar using lacing or stainless steel rings.
- (vii) Adjacent panels shall be laced longitudinally to provide a homogeneous reinforcement layer. Each lacing loop shall be pulled tight to prevent the joint opening during backfilling.

1.8 Reinforced Backfill Placement

- (i) Reinforced backfill shall be placed, spread, and compacted in such a manner that minimizes the development of slack in the geogrid and installation damage.
- (ii) Reinforced backfill shall be placed and compacted in lifts not to exceed 150mm loose lift thickness where hand compaction is used, or 200mm loose lift thickness where heavy compaction equipment is used. Lift thickness shall be decreased to achieve the required density as required.
- (iii) Reinforced backfill shall be compacted to 98% of the maximum density as determined by Heavy Compaction in accordance with NZS 4402. The moisture

Green Terramesh Retaining Wall

content of the backfill material prior to and during compaction shall be uniformly distributed throughout each layer and shall be + 2%, and - 3% of optimum.

- (iv) Only lightweight hand-operated equipment shall be allowed within 1 meter from the tail of the gabion unit.
- (v) Tracked construction equipment shall not be operated directly upon the geogrid reinforcement. A minimum fill thickness of 150 mm is required prior to operation of tracked vehicles over the geogrid. Tracked vehicle turning should be kept to a minimum to prevent tracks from displacing the fill and damaging the geogrid.
- (vi) Rubber tired equipment may pass over geogrid reinforcement at slow speeds, less than 15 KPH. Sudden braking and sharp turning shall be avoided.
- (vii) At the end of each day's operation, the Contractor shall slope the last lift of reinforced backfill away from the wall units to direct runoff away from wall face. The Contractor shall not allow surface runoff from adjacent areas to enter the wall construction site.

1.9 Quality Control Records

The Contractor shall keep records of at least the following information during the entire construction stage.

- (a) Foundation excavation levels
- (b) All laboratory and field quality control tests results.

Planning Processing Check Sheet

Retaining Walls over culvert
Road Reserve near
86 Emerald Glen Road

Building Consent Number: BC 100764

Property Information				
Property Address	86 Emerald Glen Road, Paraparamu			
Legal Description	Lot 5 DP 33688			
Planning Zone	Rural			
Building Project	Culvert, Retaining Walls			
	Yes	No	Action reqd	Actioned
Certificate of Title		✓		
Easements		✓		
Section 221		✓	Include copy	
District Plan Items to Check				
	Yes	No	Action Reqd	Actioned
Designations	✓		Airport Conical Surface / Hillcountry	
Heritage Register		✓	Noise Corridor	
Flood Hazards		✓		
Min Floor Level		✓		
Other Hazards		✓		
EQ Faults		✓	80m from Ohariu Fault trace	
Services		✓		
Eco-site/Native Veg				
	Complies?		Action Reqd	Actioned
Yards (+water bodies)			Neighbours Approval??	
HIRB	NA			
Max Height				
Site Coverage				

BC Number: BC 100764

Permeable Surface		NA		
Earthworks				
Fences/Retaining Wall				
Parking				
Water Tank / Siting				
Airport Noise 45dbI				
Other Provisions				
	Yes	No	Action reqd	Actioned
Does the building straddle a Title/cross-lease boundary?		✓		
Existing Resource Consent?	✓		RM100140	
Resource Consent Required?		✓	Apply Section 37 and advise Plan Vetting Team. <i>Already granted.</i>	
DIF Required?		✓	Calculate Fees:	
Section 72 or 77 Required?		✓	Advise Plan Vetting Team and send letters out.	
Section 221 Consent Notice		✓	Advise Plan Vetting Team	

Additional Comments

* ALL WORK TO BE IN ACCORDANCE WITH RM 100 140 .

Decision: (delete item that does not apply)

- Application complies with District Plan requirements
- Application does not comply with District Plan – BC to be issued subject to s37

Officer's Name & Signature: *J. Burke*

Date: 6-10-10

Richard Coles

From: Curtis Lee [Curtis.Lee@opus.co.nz]
Sent: Thursday, 9 September 2010 15:00
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Cc: 'Wayne Holcroft'; 'Andrew Quinn'
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Team Leader Transportation Engineering Design
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<http://www.opus.co.nz>

Level 7 Majestic Centre, 100 Willis St, PO Box 12 003, Wellington, New Zealand

106September 2010



Kapiti Coast District Council
Private Bag 601
Paraparaumu 5254

Request for Tax Invoice/Receipt

Attached is our cheque for \$1,350.00

For: Building Consent



Would you please arrange to send a Tax Invoice /Receipt, including your GST registration number for tax purposes, to me at the address printed below.

Yours faithfully

Colin Widdis
Accounting Office Manager

Items applicable to the retaining walls over emerald glen culvert

Item	Description	Unit	Quantity	Rate	Amount
1 Retaining walls					
	Reinforced Earth Walls - northern culvert				
1.1	Green Terramesh (approx 152.4m2 facing). Excludes Imported Fill, which is scheduled separately.	sq.m	152	100	\$15,240
1.2	Imported Fill	cu.m	1,420	25	\$35,500
2 Drainage					
2.1	Culverts				
	Single Box 2.0x1.5	m	17	5,000	\$85,250
	E/O for fish passage beams (2.0m x 0.2m x 0.15m)	ea	11	200	\$2,200
	Wingwall	ea	1	20,000	\$20,000
	Rip Rap Protection	m3	25	100	\$2,500
2.3	Kerb & channel	m	120	50	\$6,000
2.2	Sumps				
	Double Sump	ea	2	3,050	\$6,100
2.4	225 pipe	m	17	150	\$2,550
2.5	Manholes - 1050mm dia (Up to 2m deep)	ea	1	2,000	\$2,000
2.6	300 pipe	m	16	250	\$4,000
2.7	Wingwall (for 300 pipe)	ea	1	2,000	\$2,000
3 Traffic Services					
3.1	Safety Barriers				
	TL-3 W-Beam	m	150	100	\$15,000
	Post and Rail	m	35	50	\$1,750
Total					\$200,090

