Platinum Homes Specifications

BRICKLAYER

MATERIALS

All Bricks shall be from an approved manufacturer and shall conform to one of the following:

NZS 366

Clay Building Bricks

NZS 3102

Concrete Masonry Units

Materials for mortar shall conform to the following:

NZS 3103

Specification for Sands for Mortars and Internal and External

Renderings

NZS 3122

Specification for Portland Cement

NZS 3117

Specification for Pigments for Portland Cement and Portland

Cement Products. Natural stone shall be approved for use as noted

in NZS 4210

WORKMANSHIP

All brickwork shall be carried out by qualified and experienced tradesman.

Work shall conform to the following:

NZS 3604

Code of Practice for Light Timber Frame Buildings not Requiring

Specific Design

NZS 4210

Code of Practice for Masonry Construction: Materials and

Workmanship. Masonry manufacturer's codes of practice.

DAMP COURSE

Set damp course layer at foundation with a slope outwards to deflect any moisture to the weep holes in the veneer.

MORTAR

Mortar shall be mixed in the following proportions:

Clean Sand

- 5 parts by volume

Portland Cement

- 1 part by volume

Plasticiser

- according to the manufacturer's Instructions.

Mineral Oxide Pigment shall not exceed 3% by weight of cement.

Materials shall be mixed in a batch mixer for not less than 5 minutes. Mortar shall be used within 90 minutes of mixing. Compressive strength shall be not less than 12.5 MPa at 28 days.

1.5 **LAYING**

Lay bricks in stretcher bond, true to line, level and plumb, and in accordance with good trade practice. Bricks shall be laid 40mm clear of the timber frame. No corner shall be raised by more than one metre above the wall line. Corners shall be racked back. Lay bricks on an even bed of mortar, with ends buttered. Care shall be taken to keep the cavity between the veneer and the frame free from mortar droppings. Leave weep holes at every third joint at the base of the veneer and leave a ventilation gap at the top.

1.6 TIES

Brickwork shall be tied to the timber frame with heavy-duty wall ties, face fixed to framing. Spacing shall not exceed 600mm horizontally and 400mm vertically.

POINTING

Unless indicated otherwise in the Variation Schedule, all joints shall be tooled to a hard concave finish.

LINTELS

Where bricks are laid above openings, they shall be supported on galvanised steel angles as noted in NZS 3604 or other approved system and according to manufacturer's specification. e may be charged if plens are unavailable.

ROOFER

J.1 MATERIALS

All materials shall be from a recognised manufacturer and shall conform to their respective NZ Standards. All materials used shall be compatible one with the other.

J.2 WORKMANSHIP

All roofing work shall be carried out by accredited fixing agents or if the manufacturer does not require this, by qualified roofing tradesmen. The work shall be carried out strictly in accordance with the manufacturer's recommendations. On completion, the roof shall be left free from defects and watertight.

J.3 ROOFING UNDERLAY

Under all metal roofing, lay breather type building paper or roof underlay as required by the roofing manufacturer. Where building paper is used, fix 50mm mesh, 1.0mm thick galvanised wire netting to support paper.

J.4 CORRUGATED GALVANIZED STEEL

Fix in accordance with the manufacturer's recommendations, using galvanised spring head or lead headed nails. Finish at ridges and hips with lead edges, 0.6mm galvanised steel ridging or purpose made flashings. Finish at barges and verges with 0.6mm galvanised steel barge flashing, and into walls with 0.6mm galvanised steel apron flashing.

J.5 STRIP METAL ROOFING

Fix roofing with approved clips at every purlin in accordance with the manufacturer's recommendations. Finish at ridges, hips, barges, verges, etc. with the manufacturer's purpose made flashings.

J.6 OTHER LONGRUN ROOFING

Fix roofing with approved clips, screws, nails in accordance with the manufacturer's recommendations.

Finish at ridges, hips, barges, verges, etc. with the manufacturer's purpose made flashings.

J.7 PREFINISHED ROOFING

In addition to the above requirements, prefinished roofing shall be handled to avoid marking. All marks shall be made good and if required by the manufacturers, shall be replaced. All exposed fittings, flashings and finishings shall be prefinished to match the roofing.

J.8 COATED METAL TILES

Fix battens and tiles in accordance with the manufacturer's instructions. Finish at ridges, hips, barges, verges, etc. with the manufacturer's purpose made flashings.

J.9 CONCRETE TILES

Fix battens and tiles in accordance with the manufacturer's instructions.

Finish at ridges, hips, barges with capping moulds. Paper optional unless specified by Building Code due to coastal or other requirements.

J.10 FLASHINGS

Fix 0.6mm galvanized steel valleys and associated flashings. Fix flashings to all roof penetrations, etc.

J.11 SPOUTING & DOWNPIPES

Fix spouting and down pipes as shown on plan. Spouting shall be installed in accordance with the manufacturer's recommendations, with falls to down pipes (80mm unless otherwise secified). Install sufficient down pipes to ensure the efficient removal of water from the spouting.

J.12 GUARANTEE Provide the owner with a year minimum, written guarantee or as determined by the manufacturer, in respect of materials and workmanship.

Platinum Homes Specifications

PLUMBER

K.1 MATERIALS

All materials shall conform to their respective N.Z. Building Code and its approved documents.AS/NZS 3500

K.2 WORKMANSHIP

All works hall be carried out under the supervision of a qualified and experienced tradesman and shall be executed in accordance with the best trade practices.

All work shall conform to the N Z Building Code and its approved documents.

K.3 INSTALLATIONS

All installations shall be carried out strictly in accordance with the manufacturer's recommendations and with the N Z Building Code and its approved documents.

K.4 WATER SUPPLY

Run 20mm pipe from connection to building.

K.5 WATER PIPING

All piping shall be Fusiotherm or 1.0mm thick copper joined by an approved brazing or welding system.

Run 15mm branches to all fittings as required. Where a hot water storage cylinder is installed, this shall be fed by a 20mm branch.

K.6 HOT WATER SUPPLY

Supply and install hot water cylinder as per plan, complete with element and thermostat, and complete with isolating stopcock and tempering valve.

Support cylinder and supply tank as detailed in the NZ Building Code with suitable seismic restraint.

K.7 WASTE PIPING

All waste pipes to be an approved UPVC piping.

Run waste pipes from all fittings to gully traps in accordance with the NZBC G13 or AS/NZS 3500 and its approved documents. Install back vents to fittings as required.

K 8 SOIL PIPES

Run soil pipe from toilet pans and connect to sewer drain.

Run soil pipes from fittings as noted on the drawings and connect to sewer drain.

K.9 FITTINGS

Supply and install all fittings as shown on the drawings, complete with all taps, traps etc.

K.10 TAPS

Install matching taps as required.

Install outlets and taps for an automatic washing machine.



Platinum Homes Specifications

DRAINLAYER

L.1 MATERIALS

All materials shall be the best of their respective kinds and shall conform to their respective N Z Standards. Only materials permitted by the NZ Building Code and its approved documents shall be issued.

L.2 WORKMANSHIP

All work shall be carried out under the supervision of a qualified and experienced tradesman and shall be executed in accordance with the best trade practices.

L.3 INSTALLATIONS

All installations shall be carried out strictly In accordance with the manufacturer's recommendations and in accordance with the NZ Building Code and its approved documents.

L.4 EXCAVATIONS

Excavate for all drains to required depths and gradients. On completion of laying drains, carefully backfill around pipes.

L.5 SANITARY SEWER

Lay 100mm diameter drain from building to disposal as per plan. Pipes shall be approved UPVC. Install all necessary cleaning eyes, inspection pipes, etc.

Cover to the drains shall be in accordance with the NZ Building Code Handbook.

L.6 FITTINGS

Supply and connect all necessary gully traps, terminal vents, etc. and complete in accordance with the N Z Building Code and its approved documents.

L.7 SEPTIC TANK

Supply and install as per plan and suppliers recommendations.

Run overflow as required.

Septic tank shall conform to NZS 4610 and the required documents.



WILTON JOUBERT LTD.

Consulting Engineers

PRODUCER STATEMENT - DESIGN

ISSUED BY:	Andrew Wilton (Suitably qualified Design Professional)	
To:	Tuscany Construction Lt (Owner/Developer/Contractor)	d.
To be supplied to:	City/District Council (Territorial Authority)	
At:	Lot 2, 39A Egmont St, F	awera
	(Address)	
WILTON JOUBERT I	TD. has been engaged by	Tuscany Construction Ltd. (Owner/Developer/Contractor)
raftfloor SHOULI SEWERS LOCAL SPECIFI THIS PR	slab, structural beams D THE LOCATION OF THE S OR SERVICES REQUIRE TERRITORIAL AUTHORIT C DESIGN WILL BE NECE CODUCER STATEMENT. of the requirements	BUILDING AND/OR IT'S PROXIMITY TO THAT FOUNDATIONS BE MODIFIED TO MEET TY REQUIREMENTS, THEN FURTHER SSARY AND SHALL NOT BE COVERED BY
of Clause(s) B1	of the New Zealand Bu	ilding Code
	The design has been prepared in	accordance with B1/VM1, VM4 (verification method(s) acceptable solution(s)
(respectively) of the ap	proved documents issued by the	e Department of Building and Housing and the work is
Described on drawings and the specification an		t St, Hawera which the building is proposed to be constructed.
As a design professiona	I I BELIEVE ON REASONAE	LE GROUNDS that subject to:
0505	of the following design assump	

(ii) All proprietary products meeting the performance specification requirements, the drawings, specifications, and other documents according to which the building is proposed to be constructed comply with the relevant

(Signature suitably qualified Design Professional)

Soil movement no greater than Class M as defined in AS2870

BE, MIPENZ, CPEng, IntPE, Dip MS

provisions of the Building Act 2004.

(Professional Qualifications)
PO Box 11381 Ellerslie, Auckland
(Address)



No inspection will be made without these documents. A further inspection

WILTON JOUBERT LTD., P.O. Box 11-381, Ellerslie, Auckland, New Zealand if pians are unavailable. t: (09) 579 1114, f: (09) 579 7778

Date: 11 June 2007

WILTON JOUBERT LTD.

Consulting Engineers

PRODUCER STATEMENT - DESIGN

ISSU	ED	BY:

Andrew Wilton

(Suitably qualified Design Professional)

To:

Tuscany Construction Ltd.

(Owner/Developer/Contractor)

To be supplied to:

City/District Council

(Territorial Authority)

At:

Lot 2, 39A Egmont St, Hawera

(Address)

WILTON JOUBERT LTD. has been engaged by

Tuscany Construction Ltd.

(Owner/Developer/Contractor)

to provide

ENGINEERING DESIGN AND STRUCTURAL DRAUGHTING SERVICES FOR

raftfloor slab, structural beams

SHOULD THE LOCATION OF THE BUILDING AND/OR IT'S PROXIMITY TO
SEWERS OR SERVICES REQUIRE THAT FOUNDATIONS BE MODIFIED TO MEET
LOCAL TERRITORIAL AUTHORITY REQUIREMENTS, THEN FURTHER

SPECIFIC DESIGN WILL BE NECESSARY AND SHALL NOT BE COVERED BY THIS PRODUCER STATEMENT.

in respect of the requirements

(Extent of Engagement)

of Clause(s) B1

of the New Zealand Building Code

All Part only as specified

of the building work. The design has been prepared in accordance with

B1/VM1, VM4
(verification method(s) acceptable solution(s)

(respectively) of the approved documents issued by the Department of Building and Housing and the work is

Described on drawings titled Lot 2, 39A Egmont St, Hawera and the specification and other documents according to which the building is proposed to be constructed.

As a design professional I BELIEVE ON REASONABLE GROUNDS that subject to:

(i) The site verification of the following design assumptions:

Minimum dependable bearing capacity of 100kPa over all platforms

Soil movement no greater than Class M as defined in AS2870

(ii) All proprietary products meeting the performance specification requirements, the drawings, specifications, and other documents according to which the building is proposed to be constructed comply with the relevant provisions of the Building Act 2004.

(Signature suitably qualified Design Professional

BE, MIPENZ, CPEng, IntPE, Dip MS

(Professional Qualifications)

PO Box 11381 Ellerslie, Auckland
(Address)

Date: 11 June 2007

APPROVED PLANS



Residential Concrete Specialists
P.O. Box 11-381 Ellerslie Auckland
Phone 021-494-586 Fax 09-579-7778

RIBRAFT FLOOR SYSTEM

RIBRAFT DESIGN - EDGE BEAM <job no.="">: <job title="">: <description>:</description></job></job>	39A Egmont St	Date: Designer: Checker:	6/7/2007 ASC ARW
Loadings	WALLS		

Loodings	T MALLO			-		THE RESERVE OF THE PARTY OF THE
Loadings	WALLS	-				
	Lower:	Timber	•			
			•		(North/Sou	ith Taupo)
				=	38	kg/m²
				=	0.37	kPa
	1	Height		=		m
		Load/m		=		kN/m
Upper Wall must be Timber	Upper:	Timber	•			•
			•		(North/Sou	th Taupo)
				=	38	kg/m²
				=	0.37	kPa
	1	Height		=		m
		Load/m		=		kN/m
	Other (Spec	cify Wall We	ight)			
4		Weight		=	185	kg/m²
S				=	1.81	kPa
		Height		=	2.42	m
	1	Load		=:	4	kN/m
1	SUSPE	NDED MID)-FLC	OR	Tributary L	ength L/2
		Unit/Flo	or	=	Timber	~
1 1	1	Topping		=	N/A	▼
				=	0.38	kPa
Table Destand Florida		Live Loadi		=	1.50	kPa
Tributary Roof and Floor Lengths	Super	Imposed De	ad	=		kPa
	1	Γributary Sp	an	=		m
	1	Dead Load		=		kN/m
		Live Load	/m	=		kN/m
	ROOF		Tri	butary L	.ength = S	
		Heavy	▼ I	=	0.84	kPa
		Live Loadii	ng		0.25	kPa
			- 573		4.00	
	1	ributary Spa Dead Load		=	4.00 3.36	m kN/m
		Live Load		==	1.00	kN/m
		LIVO LUAU			1.00	DEFER
Loadings Summary	Walls			4.39	kN/m	Dead
	Mid Floors				kN/m	Dead

oadings Summary	Walls	4.39	kN/m	Dead
	Mid Floors		kN/m	Dead
	1		kN/m	Live
	Slab*	1.37	kN/m	Dead+SDL
		1.20	kN/m	Live
	Roof	3.36	kN/m	Dead
		1.00	kN/m	Live^
	Extra Dead		kN/m	Dead
	Extra Live		kN/m	Live
	* Includes E	dge Beam Weight, SI	DL = 0.5 kPa LI	_ = 1.5 kPa
	^ Roof LL n	ot added if midfloor ex	ists else LL = 0	.25 kPa

ALOSEE MESTAGNESS

THESE MUST BE AVAILABLE FOR VIEWING AT ALL TIMES

No inspection will be made without these documents. A further inspection fee may be charged if plans are unevaluable.



Residential Concrete Specialists
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RIBRAFT FLOOR SYSTEM

Jo	b Details: 39A Egmont St			10280	7/6/07
Working Loads	Dead		9.13	kN/m	
	Live		2.20	kN/m	
Ultimate Loads (1.2G + 1.5Q)	Dead		10.95	kN/m	
	Live		3.30	kN/m	
Total Ultimate Load on Edge		w =	14.25	kN/m	

Vall Construction Type	Brick Veneer min beam wi	idth 300mm	,	•
dge Beam Details	Beam Width =	300	mm	
	Pod Depth =	220	mm	OK!
	Top slab Depth =	85	mm	
	Self Weight =	2.20	kN/m	
	Construction Type =	Standard Raft	-	

Standard Raft Construction Bearing Capacity Over Platform: 100 kPa Bearing Check = OK! Working Pressure = 33 kPa Steel Requirements - As Per Firth Std Drg's

Top Steel: 1HD12 Btm Steel: 2HD12

Continuous Piers

(N.B. All Edge Beam Load in to 'strip' footing) Assume 100 kPa Bearing Under Edge Beam, unless a Soil Report indicates otherwise.



Bearing Capacity under Piers:

Bearing Check =

Pressure =

Steel Requirements - As Per Firth Std Drg's

Top Steel: Btm Steel:

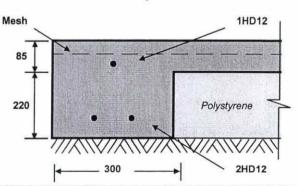
The prescribed Edge Beam design is:

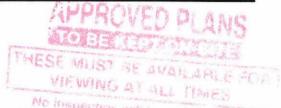
Location (ie: which wall): Edge beam

Standard Raft Contruction

kPa

300 mm wide edge beam with 1HD12 in top & 2HD12 in bottom





kPa

No inspection will be made infinite these documents, a further inspection in many be charged it many to the charged in stone and the charged in th



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RIBRAFT FLOOR SYSTEM

RIBRAFT DESIGN - INTERNAL LOADBEARING	RIB		
<job no.="">:</job>	10280	Date:	6/7/2007
<job title="">:</job>	39A Egmont St	Designer:	ASC
<description>:</description>	internal thickening	Checker:	ARW

<description:< th=""><th>internal thi</th><th></th><th></th><th></th><th>Checker:</th><th>ARV</th></description:<>	internal thi				Checker:	ARV
Loadings	WALLS					
	Lower:	HOTBloc 🔻		Solid	•	
		Light		(North/S	South Taup	0)
			=	400	kg/m²	•
	1	graveous privis	=	3.92	kPa	
	1	Height	=		m	
		Load/m	=		kN/m	
	Upper:	HOTBloc ▼		Solid	<u> • </u>	
		Light ▼		(North/S	South Taup	0)
			=	400	kg/m²	
	1		=	3.92	kPa	
	1	Height	=		m	
		Load/m	=		kN/m	
	Other (Spec	cify Wall Weight)				
		Weight	=	30	kg/m ²	
			=	0.29	kPa	
L		Height	=	2.42	m	
S		Load	=	111	kN/m	
	SUSPE	NDED MID-FLOC	R	Tributar	y Length =	(L ₁ +L ₂)/2
		Unit/Floor	=	Timber	-	
<u>L1</u> ← <u>L2</u> →		Topping	=	N/A	-	
			=	0.38	kPa	
		Live Loading	=	1.50	kPa	
	Su	per Imposed Dead	=		kPa	
	1	Tributary Span	=		m	
Tributary Roof and Floor Lengths		Dead Load/m	=		kN/m	
	i	Live Load/m	=		kN/m	
	ROOF	Tributary Length =	s:	(Check if	f Rafter or T	russ)
	1	Heavy 🔻	=	0.84	kPa	
		Live Loading		0.25	kPa	
		Tributary Span	=	4.30	m	
		Dead Load/m	=	3.61	kN/m	
		Live Load/m	=	1.08	kN/m	
Loadings Summary	Walls		0.71	kN/m	Dead	
	Mid Floors			kN/m	Dead	
			See Laboratory of the Control of the	kN/m	Live	
	Slab*		2.60	kN/m	Dead+SDL	
	Doof		1.80	kN/m	Live	
	Roof		3.61 1.08	kN/m kN/m	Dead	
	Extra Dead		1.08	kN/m	Live^ Dead	
	Extra Live			kN/m	Live	
		Includes Internal Rib We	ight, SDL			
		Roof LL not added if mid				MAIC

NO BEKEPT ON-SITE

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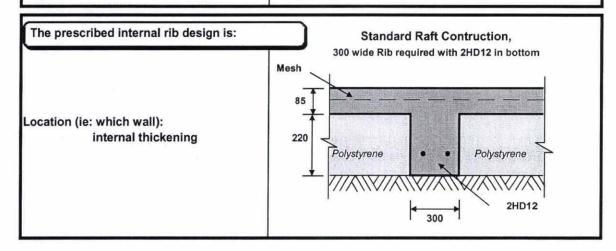
RIBRAFT FLOOR SYSTEM

Job Details: 39A Egmont St		Job Number:	10280	7/6/07
Dead		6.92	kN/m	
Live		2.88	kN/m	
Dead		8.31	kN/m	
Live		4.31	kN/m	
w	=	12.62	kN/m	
	Dead Live Dead Live	Dead Live Dead	Dead 6.92 Live 2.88 Dead 8.31 Live 4.31	Dead 6.92 kN/m Live 2.88 kN/m Dead 8.31 kN/m Live 4.31 kN/m

Internal Rib Details	Rib Width =	300	mn	n	
	Pod Depth =	220	mn	n	OKI
	Top slab Depth =	85	mn	n	
	Self Weight =	1.58	kN	/m	
	Construction Type	Standard R.	aft	-	

tandard Raft Construction	Bearing Capacity:	100	kPa	
	Bearing Check =	OK!		
	Working Pressure =	27	kPa	
	Steel Requirements - As Per Fir	th Std D	rg's	
	Btm Steel: 2HD12			

Continuous Piers (N.B. All Interal Load in to 'strip' footing) Assume 100 kPa Bearing Under Loadbearing Rib, unless a Soil Report indicates otherwise. Bearing Capacity: kPa Bearing Check = Pressure = kPa Steel Requirements - As Per Firth Std Drg's Btm Steel:







Beam Loadings and Deflections

Residential Concrete Specialists

Job Number:

10280

Date:

6/7/2007

Job Title: Beam Description: 39A Egmont St L1: Bed 3 lintel

Designer:

ASC

Beam Loadings: Primary Bea	am		Beam Span =	2.1		
roof span:	2.2	m	floor A span:		m	
roof weight:	0.84	kPa	floor A weight:		kPa	
roof live load:	0.25	kPa	floor A live load:		kPa	
wall A height:		m	floor B span:		m	
wall A mass:		kg/m²	floor B weight:		kPa	
wall B height:		m	floor B live load:		kPa	
wall B mass:		kg/m²	self weight:	0.1026	kN/m	

Beam Loadings: Seconda	ary Bea	n	Beam Span = 4.2	
roof span:	2.2	m	floor A span:	m
roof weight:	0.84	kPa	floor A weight:	kN/m
roof live load:	0.25	kPa	floor A live load:	kPa
wall A height:		m	floor B span:	m
wall A mass:		kg/m²	floor B weight:	kN/m
wall 8 height:		m	floor B live load:	kPa
wall B mass:		kg/m ²	self weight:	kN/m

Beam Loadings: Additional	Point Load
G:	kN
Q:	kN

1.9506 UDL G: kN/m kN/m Point Load, G:

3.8808 kN

0.55

1.155 kN

Consider Load Combination:

1.2 G and

kN

1.5 Q

UDL: Point Load:

Additional Point Load:

3.16572 kN/m

6.38946 kΝ

0

at

at

1.8 m from support 1 m from support 1

kNm

kΝ

kN

kΝ

mm

mm

mm

OK

Beam end sup	port con	dition:	simply	supported	~		
			<u> </u>		, -	⁴ M*=	2.8
Try:	2/190x4	5				V* =	8.8
				_		support 1 reaction:	4.2
Beam Type:	timber	•		▼		support 2 reaction:	8.8
for t	imber &	b =	90	mm		k2 =	2
flitched	beams	d =	190	mm		max. long term deflection:	4.4
		1 =	51442500	mm⁴		max. short term deflection:	2.4
for othe	er beams	=		mm⁴	i	allowable defn (functionality), L/400 =	5.3
		E =	8000	MPa			

Beam Design

$$\phi = 0.8$$
 $k1 = 0.8$
 $lay/b = 10$

3.8 MPa shear stress = MPa bending stress = 14

541500

mm3

οк 4.9 kNm 27.7 kΝ QΚ



Beam Loadings and Deflections

Residential Concrete Specialists

Job Number:

10280

Date:

6/7/2007

Job Title:

39A Egmont St

Designer:

Beam Description:

L2: Bathroom lintel

ASC

Beam Loadings: Primary Beam

Beam Span = m

floor A span:

m

roof span: roof weight: roof live load:

0.84 0.25

3.1

floor A weight: floor A live load:

kPa kPa

wall A height: wall A mass: kPa m kg/m²

kPa

floor B span: floor B weight: floor B live load:

Beam Span =

m kPa

wall B height: wall B mass:

m kg/m⁴

self weight:

0.1026

kPa kN/m

Beam Loadings: Secondary Beam

roof span: roof weight:

1.4 m 0.84 0.25

floor A span: floor A weight: floor A live load: m kN/m kPa

roof live load: wall A height: wall A mass:

kPa kPa m kg/m² m

floor B span: floor B weight: floor B live load:

self weight:

m kN/m kPa

kN/m

Beam Loadings: Additional Point Load

wall B height:

wall B mass:

Q:

kN kN

kg/m²

Loadings Summary

UDL G: 2.7066 0.775 Q:

kN/m kN/m Point Load, G:

Q:

kN

kN

Consider Load Combination:

1.2 G and 4.41042 kN/m

1.5 Q

UDL: Point Load:

4.16283 kN

•

at

2.5284

0.7525

1.4 m from support 1

Additional Point Load:

kN

at

m from support 1

Beam end support condition:

2/190x45

simply supported

 $M^* =$ 1.9 7.2

V* = support 1 reaction:

kN 4.0 kN

kNm

kN

Beam Type: timber

E =

support 2 reaction:

for timber & b = flitched beams

90 mm 190 mm

8000

k2 = max. long term deflection: 2

7.2

51442500 mm⁴ max. short term deflection:

1.7 mm

for other beams I =

mm⁴ MPa

V

allowable defn (functionality), L/400 =

0.9 mm 4.0 mm OK

0.8 0.8 k1 =

shear stress = bending stress = 3.8 MPa 14

lay/b =

10

MPa

k8 =

Z =

mm3

Beam Design

Try:

4.9 27.7

kNm kN

OK OK 541500

VIEWING AT ALL TIMES

No inspection will be made without these documents. A further inspection may be charged if plans are unavailable.



Beam Loadings and Deflections

Residential Concrete Specialists

Job Number:

10280

Date:

6/7/2007

Job Title:

39A Egmont St

Designer: ASC

Beam Description:

L3: Master bedroom lintel

Beam Loadings: Primary Bea	am		Beam Span =	1.9	N. Committee of the Com
roof span:	1.3	m	floor A span:		m
roof weight:	0.84	kPa	floor A weight:		kPa
roof live load:	0.25	kPa	floor A live load:		kPa
wall A height:		m	floor B span:		m
wall A mass:		kg/m ²	floor B weight:		kPa
wall B height:		m	floor B live load:		kPa
wall B mass:		kg/m²	self weight:	0.1026	kN/m

Beam Loadings: Second	ary Bea	m	Beam Span = 4.	4
roof span:	1.4	m	floor A span:	m
roof weight:	0.84	kPa	floor A weight:	kN/m
roof live load:	0.25	kPa	floor A live load:	kPa
wall A height:		m	floor B span:	m
wall A mass:		kg/m ²	floor B weight:	kN/m
wall B height:		m	floor B live load:	kPa
wall B mass:		kg/m ²	self weight:	kN/m

Beam Loadings: Additiona	l Point Load	
G:	kN	
Q:	kN	

			C	mary
L	.oau	mas	Sum	ımarv

UDL G: 1.1946 kN/m Point Load, G: 2.5872

0.325 kN/m Q:

Q: 0.77 kN

Consider Load Combination:

1.2 G and

1.5 Q

kN

1.92102 UDL:

kN/m 4.25964 kN

0.5 m from support 1

Point Load: at Additional Point Load: 0 kN at m from support 1

kNm kN kN kN
kN kN
kN
mm
mm
mm
mm
DOINE
of the party

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Beam Loadings and Deflections

Residential Concrete Specialists

Job Number:

10280

Date:

6/7/2007

Job Title:

39A Egmont St

Designer:

ASC

Beam Description:

L4: Lounge lintel

Beam	Loadings:	Primary	Beam	
				1

-			
roof span:	1.6	m	
roof weight:	0.84	kPa	
roof live load:	0.25	kPa	
wall A height:		m	
wall A mass:		kg/m²	
wall B height:		m	
wall B mass:		kg/m²	
wall A mass: wall B height:		kg/m² m	

Beam Span =	2.8	
floor A span:		m
floor A weight:		kPa
floor A live load:		kPa
floor B span:		m
floor B weight:		kPa
floor B live load:		kPa
self weight:	0.1566	kN/m

gs. Second	ary Dear	
roof span:	1.8	m
roof weight:	0.84	kPa
of live load:	0.25	kPa
all A height:		m
vall A mass:		kg/m ²
all B height:		m
vall B mass:		kg/m ²

Beam Span =	5.4		
floor A span:		m	
floor A weight:		kN/m	
floor A live load:		kPa	
floor B span:		m	
floor B weight:		kN/m	
floor B live load:		kPa	
self weight:		kN/m	

Beam Loadings: Additional Point Load

wall B mass:

J.	MIA
2:	kN

Loadings Summary

UDL	G:	1.5006	kN/m
	0.	0.4	kNI/m

Point Load, G: 4.0824 kN

1.215 kN

Consider Load Combination:

1.2 G and

1.5 Q

Point Load:

2.40072 kN/m kN

UDL: 6.72138

at

1.8 m from support 1

Additional Point Load:

•

at

m from support 1

Beam end support condition: simply supported

2/290x45	
timber	

M* =	6.3	kNm
V* =	7.7	kN
support 1 reaction:	5.8	kN

Beam Type: timber

Try:

k2 = 2 4.4

support 2 reaction:

7.7 kN

for timber & b = flitched beams 290

mm I = 182917500 mm⁴

max. long term deflection:

mm mm

for other beams | = E= 8000

max. short term deflection: allowable defn (functionality), L/400 = 2.3 7.0 mm OK

Beam Design

$$\phi = 0.8$$
 $k1 = 0.8$
 $lay/b = 10$

mm⁴

MPa

shear stress = bending stress = MPa

k8 =

OK

OK

14 MPa

3.8

φ V =

kNm 42.3 kN

1261500

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Beam Loadings and Deflections

Residential Concrete Specialists

Job Number:

10280

Date:

6/7/2007

Job Title:

39A Egmont St

Designer:

ASC

Beam Description:

L5: Garage door lintel

Beam Span =

floor A span: m

roof span: roof weight:

Beam Loadings: Primary Beam

0.8 m kPa 0.84

floor A weight: floor A live load: kPa kPa

roof live load: wall A height:

0.25 kPa m kg/m²

floor B span: floor B weight:

Beam Span =

m kPa

wall A mass: wall B height: wall B mass:

m kg/m⁴

m

floor B live load: self weight:

kPa 0.23 kN/m

Beam Loadings: Secondary Beam

roof span: roof weight: roof live load:

2.2 0.84 0.25

floor A span: kPa floor A weight: kPa floor A live load: m kN/m kPa

wall A height: wall A mass: wall B height: m kg/m² m kg/m²

floor B span: floor B weight: floor B live load:

self weight:

m kN/m kPa

kN/m

wall B mass: Beam Loadings: Additional Point Load

kN G: 6

Q: 2 kN

kN/m

kN/m

kN

Loadings Summary UDL G: 0.902

Q: 0.2 Point Load, G:

5.7288 1.705 kN

Consider Load Combination:

1.2 G and kN/m

kN

kN

1.5 Q

UDL: Point Load:

1.3824 9.43206

Q:

2.6 m from support 1

Additional Point Load:

10.2

at

4 m from support 1

Beam end support condition:

simply supported •

M* = V* =

19.6 kNm 16.7

kN

Try:

steel

200PFC

support 1 reaction:

9.7 kN

Beam Type:

support 2 reaction:

16.7 kN

for timber & d = mm mm

•

max. long term deflection:

flitched beams

1 = 0 mm⁴

max. short term deflection:

9.1 mm 9.9

for other beams | =

mm⁴ 1.91E+07 E = 200000 MPa allowable defn (functionality), L/400 =

mm 12.3 mm

OK

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these accuments. A further inspection se may be charged it plans are unavailable.

MemDes Calculations @ 10:01 on 7/06/2007 by ASC

Project : Egmont St Descriptn : L5

Section: 200PFC Grade 300+

Check of Section Category Requirements

NOTE: Designer to ensure that Clause 12.8.3.3 is complied with ***

**** MAJOR axis bending ****

Major Axis Bending

Design Action M*_x = 20.0 kNm

$$\alpha_{\rm m} = 1.22$$

$$\alpha_{\rm s} = 0.28$$

$$\alpha_{\rm m}$$
 $\alpha_{\rm s}$ < 1.0, => Segment NOT Fully Restrained

$$M_{bx} = 1.22 * 0.28 * 66.3 = 23.0$$

Major axis capacity Ratio =
$$M_X^* / \phi M_{bx}$$

Shear Calculations (Unstiffened Web)

Design Action $V_{X}^{*} = 17.0 \text{ kN}$

Nominal Shear Yield capacity V_w = 230.4 kN

$$\alpha_v = 5.35 >= 1.0 => \text{ full web shear capacity}$$

$$V_u = V_w = 230.4 \text{ kN}$$

Shear capacity ratio =
$$V_{X}^{*}/\phi V_{U}$$

$$= 0.08,$$

**** U.L.S. Capacity Check Passed, Load Cap. Ratio = 0.97 ---- OK ----

APACVED PLANS

ALST STREAVAILABLE SE

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MemDes v 2.1: Calculations for Wilton Joubert Ltd

Project: Egmont St

Descriptn: L5

at 10:01:28 AM on 6/7/2007, Page 1

by ASC



Residential Concrete Specialists

PO Box 11381 Ellerslie, Auckland. Ph: 09 579 1114, Fax: 09 579 7778

Job Title:

39A Egmont St

Client: Owner: Platinum Homes Platinum Homes

Job Address:

39A Egmont St

Page: 1

Job #: 10280

Date: 7th June

By: ASC

Chkd: ARW

Calculation of Shear Pile Requirements

Use AS1170 design philosphy of preventing damaging movement up to an SLS event, and thereafter accepting increasing levels of damage as the event increases.

Assumed Class C soil:

City/town:

New Plymouth

(t) = Ch(t) Z R N(T,D)

Ch(t)

2.36

Rs (25 yr

0.18

0.25

1

Timber frame building ductility u = 2 (ULS only) Assume T1 = 0.4

Ru 500yr

ku = 1 + (u-1)T1/0.7

N(T,D)C(t) SLS

1 SLS 0.11

ku = 1.57

C(t) ULS 0.42

Cd(T1) = C(t) Sp/ku

For base sliding Sp =

SLS Cd(T1) =

0.11

Note for Class D soil SLS Cd(t1) = 0.14

ULS Cd(T1) =

0.27

ULS Cd(t1) = 0.34

Testing by Dr McManus University of Canterbury (Geotechnical Society Symposium, 2003) found the friction co-efficient of slab on polythene on sand had peak friction co-efficients of 0.42 for an unloaded slab and 0.53 for a loaded slab. Residual friction angles were 0.38 and 0.49 respectively.

ese values are some what lower than that published by other sources, normally 0.7 peak and 0.5 residual.

Even using the lower values there will be no relative movement between the slab and the ground at SLS level event on either Class C or D soil



GIB[®] Wall Bracing Calculation Sheet A

single storey

V85A

Job Details

GIB[®] EzyBrace™

GIB® Bracing Systems, 2006

Name Street and Number TAR 15 J & M ROBERTS 39A EGMONT STREET

Lot and DP Number

LOT 2 DP 371240

City/Town/District
Designer and date

HAWERA ADAM SPITZ

29/05/2007

Company Name

PLATINUM DESIGN LTD

Building Specification

Building Specification			
Location of Storey	single	•	
Floor Loading	2 kPa	-	
Foundation Type	slab	-	
Building Height to Apex (m)	5	•	
Roof Height above Eaves (m)	2	-	
Stud Height (m)	2.4	•	
Cladding Weight (top or single)	heavy	•	
Cladding Weight (lower)	light	•	not applicable (s
Cladding Weight (subfloor)	light	•	not applicable (sla
Roof Weight	heavy	•	
Roof Pitch (degrees)	0-25	~	
Room in Roof Space	no	-	
Building Length (m)	20.5		
Building Width (m)	12.46		
Gross Building Plan Area (m2)	165.49		

not applicable (single storey building) not applicable (slab)

Building Location

Wind Zone	High		Earthquake Zone		
Region	R1	~	Α		
Terrain	Inland	-			
Exposure	Exposed	-			
Topography	Moderate	•			

Bracing Units required for Wind

per m	subfloor	walls	
W along	n/a	64 BUs/m	1
W across	n/a	54 BUs/m	
			,
Totals	subfloor	walls	
W along	n/a	797 BUs	
W across	n/a	1107 BUs	

Bracing Units required for Earthquake

subfloor	walls	
n/a	7.9 BUs/m2	~
April	PROVED PL	ANS
subfloor	walls	
n/a	1307 BUs 41_4	BLE FOR
n/a	1307 BUs 1	IES
	n/a subfloor n/a	n/a 7.9 BUs/m2 POVED Pusubfloor walls n/a 1307 BUs 11.0

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Alc inspection will be made without the documents. A further inspection be charged if plans are unavailable.

GIB® Ezy	all Bracing Brace ™					single sto		cing Syster	ns, 2006
Along									
	racing Line	Bracing E	lements p					Wind	Earthq.
1	2	3	4	5	7	8	6	9W	10EQ
Line Label	Minimum BUs Req/Ach	Bracing Element No.	Supplier	Bracing Type		Element Height H (m)	Angle to Bracing line (degrees)	BUs Achieved	BUs Achieved
Α	203	1	PLY	SP1	1.2	2.4		120	120
		2	PLY	SP2	0.6	2.4		51	51
line totals		3	PLY	SP1	1.2	2.4		120	120
W	291	4						120	120
EQ	291	5							
В	203	1	GIB®	GS2	1.8	2.4		144	126
The same	1 192.0	2	GIB®	GS2	2.4	2.4		216	192
line totals		3	GIB®	GS2	2.4	2.4		216	192
W	644	4	PLY	SP2	0.8	2.4		68	68
EQ	578	5	150-17	1	0.0	A. 1		- 00	50
С	177	1	PLY	SP4	0.45	2.4		34	38
		2	PLY	SP2	0.7	2.4		60	60
line totals		3	GIB®	GS2	1.8	2.4	45	102	89
W	195	4							30
EQ	187	5							
D	118	1	PLY	SP2	0.78	2.4		66	66
		2	GIB®	GS2	2.1	2.4		168	147
ine totals	YEAR E R MISH	3	737						
N	234	4	Spiritualia			77.1	77-11	3 3 1 7	
Q .	213	5	Promp		7.1.7			707.0	1 - 130
	57	1	PLY	SP1	1.2	2.4		120	120
		2							
ine totals		3							
V	120	4							
Q	120	5				A Mary Transport			
	enter	1							January Marcola Control
	78.77.57	2			-1-4				
ne totals	The state of	3							
V		4	1172						
Q		5				**			
G	enter	1							
		2							
ine totals		3							
V		4							的是一种
Q		5			E III AS HAS THE				AND THE STATE OF
1	enter	1				A STATE OF THE STA			
		2							
ne totals	TETU	3	Topical of		1 12 1		1	we 1808	1 1 1 1 1 1 1 1
v	THE ENVI	4					- Sep. 1	AP A C	VYL
Q		5					-447	135.15	1111
					THE RESERVE THE PARTY OF THE PA		ESE W	Wind	Earthq.
Totals Ach	ieved				W 100 COX 20 CO		VIEw	1484	1389
The state of the s							THE PERSON NAMED IN COLUMN	ОК	ОК
otals Rec	quired (from S	Sheet A)					St Other	797	1307

GIB[®] Bracing Systems, 2006

For full construction details see literature GIB® Bracing Systems, 2006

200		Minimum Length	BUs	BUs
Supplier	System	(m)	W/m	EQ/m
	none			
GIB [®]	GS1a	1.8	65	55
		2.4	75	65
GIB [®]	GS2	1.2	70	60
		1.8	80	70
		2.4	90	80
GIB [®]	BL1	0.4	120	115
		0.6	125	115
GIB [®]	BL1a	1.8	130	115
GIB [®]	BLP	0.6	145	135
		0.9	145	145
GIB®	BLG	0.6	145	130
		1.2	150	130
PLY	SP1	0.9	100	100
PLY	SP2	0.6	85	85
PLY	SP4	0.45	75	85
Custom	Custom			
Custom	Custom			- 4
Custom	Custom			

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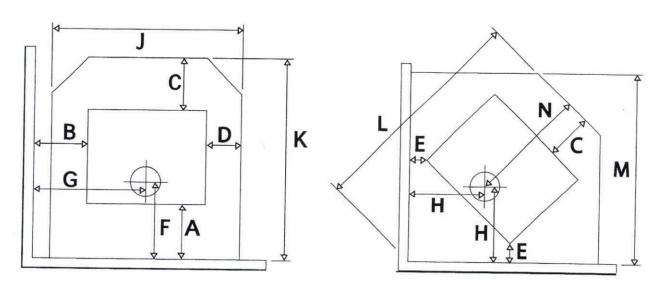
GID LZYI	Brace ™						GIB® Brad	cing Syster	ns, 2006
Across									
	acing Line	Bracing El	ements p					Wind	Earthq.
1 Line Label	2 Minimum BUs Req/Ach	3 Bracing Element No.	4 Supplier	5 Bracing Type		8 Element Height H (m)	Angle to Bracing line (degrees)	9W BUs Achieved	BUs Achieved
M	70	1	PLY	SP1	1.2	2.4		120	120
		2	PLY	SP1	1.2	2.4		120	120
line totals		3	I L I	OF 1	1,2	2.4		120	120
W	240	4						To the second	
EQ	240	5							
N	124	1	GIB®	GS2	1.8	2.4	10	144	126
	The second second	2	GIB®	GS2	1.5	2.4		105	90
line totals	74 79.3	3	PLY	SP2	0.6	2.4		51	51
W totals	300	4		01 2	0.0	2.7	Table 1	- 51	01
EQ .	267	5		218/8/5					
O	70	1	GIB®	GS2	2.7	2.4		243	216
		2	GIB®	GS2	1.8	2.4		144	126
line totals		3	PLY	SP1	1.18	2.4		118	118
W	505	4		0, 1	1.10	<u></u>		110	110
EQ	460	5							
P	124	1	GIB®	GS2	1.9	2.4	(Captonios N.2968)	152	133
		2							
line totals		3	11 M 12 M 12						1
W	150	4	Torrest the		Bullyne		THE WAY	- Dayse	Thorse or a
EQ.	152 133	5	A Charles	-			70.00		
Q	80	1	PLY	SP4	0.49	2.4	In the contract of	37	42
		2	GIB®	GS2	1.5	2.4		105	90
line totals		3	PLY	SP4	0.58	2.4		44	49
W	268	4	PLY	SP4	0.5	2.4		38	43
EQ	274	5	PLY	SP4	0.6	2.4		45	51
R	100	1	PLY	SP1	1.2	2.4	1) 12 2 (62 (62))	120	120
	100	2	More, and					120	1.20
line totals	LOT CONTY	3	erester i pen		777				
W totals	120	4	-						
EQ	120	5	-						
	enter	1							
		2							
ine totals		3							
N totals		4		2000					
EQ		5		A LO TORI DELL'AND DE		Programme and the second			
CONTRACTOR OF THE PARTY OF	enter	1			NE REMAIN PER			777	DIAN
		2					7077	Proposes	710000
ine totals	7.17	3	-			"NE			
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EQ		5				Water Spine	(HEW)(4)	HATAL	TIMES
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Totals Achi	eved			1001300000		1129	docume	1585	1494) e
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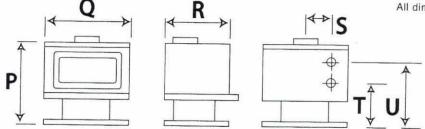
SS200 FS 2006

MODEL: CLEAN AIR

FREESTANDING SPECIFICATIONS PAGE 1 OF 2



Dimension 'C' is taken from door seal lip (fire box) to the front of the floor protector. All dimensions are in millimetres. All dimensions are set as a minimum guide only.



SS200 Clearance requirements with double 1200mm double skin flue shield

A	В	С	D	E	F	G	Н	J	K	L	M	Ν	Р	Q	R	S	Т	U
150	250	300	200	150	300	550	435	1000	880	1125	1000	575	625	600	430	145.5	280	460

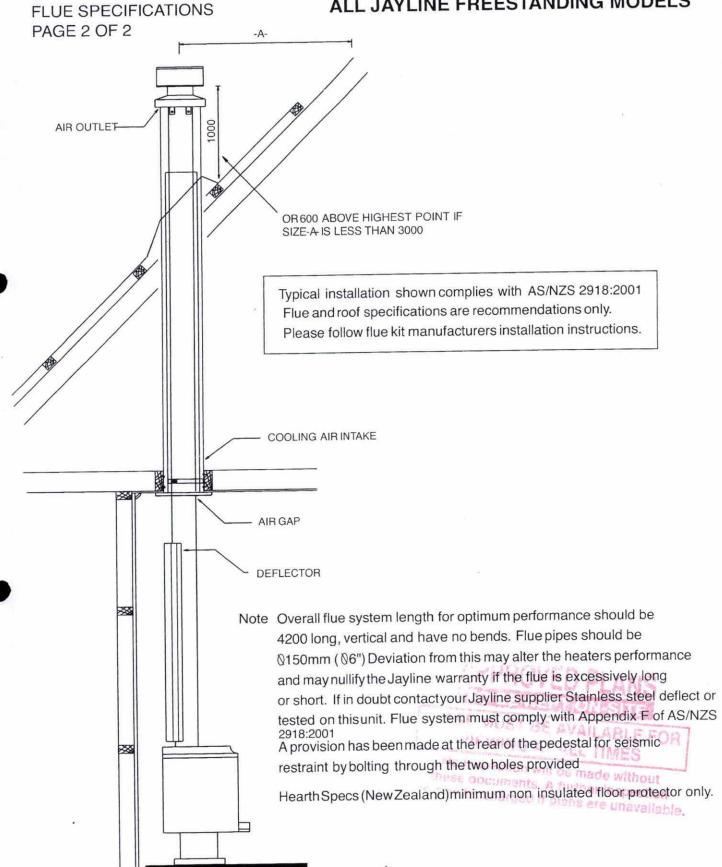
WITHOUT FLUE SHIELD A = 350, B = 300, E = 200

Industry Rated Reak Output (kW)	Water Heating Option	Floor Protector	Emissions Efficiency
14	No	Non Insulated	.95g/kge documer65.8% luther harnest
		30.	the charged if plans are unavailable.

IF INSTALLED IN ACCORDANCE WITH THESE SPECIFICATIONS THE HEATER COMPLIES WITH AS/NZS 2918:2001 (A.R.S Report No. 06/1314) Authorisation #N0023



ALL JAYLINE FREESTANDING MODELS



IF INSTALLED IN ACCORDANCE WITH THESE SPECIFICATIONS THE HEATER COMPLIES WITH AS/NZS 2918:2001 including appendix F

RETAIL LINKS LTD RESERVE THE RIGHT TO CHANGE SPECIFICATIONS OR DESIGNS WITHOUT PRIOR NOTICE.

PlaceMakers New Plymouth

Producer Statement: Page 1

Job: Q1324T

Description:

Client: Platinum Homes

Phone

New Residence J / M Roberts 39a Egmont Street

Hawera

MiTek New Zealand Ltd.

Phone:

Printed: 08:14:59:23 Aug 2007

PRODUCER STATEMENT MiTek 20/20™ TRUSS DESIGN PROGRAM

Certification of MiTek 20/20™ Truss Design Program

Building Consent No.: MiTek 20/20 - Engineering 4.4 Gamma 1.5 (build 1597-51)

The MiTek 20/20™ truss design program has been developed by MiTek New Zealand Ltd for the design of GANG-NAIL timber roof, floor and attic trusses in New Zealand. The truss designs computed by this program are prepared using sound and widely accepted engineering principles, and in accordance with NZS 4203, NZS 3603 and NZS 3604 as verification methods and acceptable solutions of the approved documents of the New Zealand Building Code to satisfy the requirements of the Building Regulations 1992: Clause B1 - Structure. This computer design for the proposed building complies with the relevant provisions of the NZ Building Code. This is subject to all proprietary products meeting their performance specification requirements, the provision of adequate bracing, fixings and the correct input of design data carried out by suitably trained personnel.

Summary of MiTek 20/20™ Truss Design Data and Output

The MiTek 20/20™ computer design output for this job titled and located at the site identified on the top of this page is based on the following parameters entered into the program. The owner must ensure that the following job details below are current and relevant to the project before fabrication and erection of the GANG-NAIL

Job Details

Roof

Roof Truss Timber Group: Material-

Dead Load:

MSG8 H1.2

Pitch: Celling

Material:

25.000 deg

Std Overhang:

700 mm

Gib Board 12mm

Wind Area:

0.600 kPa Restraints: 400 mm centres Qur = 0.250 kPa Live Load:

Qc = 1.000 kN

Monier Concrete Tiles

Dead Load: 0.200 kPa Restraints: 600 mm centres Pressure Coeff:

High (44.0 m/s) Cpe = varies; Cpi = -0.30, 0.20

These trusses must be fabricated and erected in accordance with the GANG-NAIL manual. Proper erection bracing must be installed to hold the components true and plumb and in a safe condition until permanent bracing is fixed. All permanent bracing and fixing must be installed before any loads are applied. The specifications for timber shall be as shown on the output. The timber shall be standard gauged and treated to the requirements of NZS 3640:2003. Unless otherwise noted, this design assumes that the steel fixings and timber connectors are situated in a closed environment, as defined by NZS3604:1999 Section 4.

Truss List

* = detail only, ? = input only, Two = failed design, Unmarked trusses = designed successfully, LB = lateral bracing required Legend:

	Truss	Qty	Span (mm)	Pitch (deg)	Spacing (mm)	Truss	Qty	Span (mm)	Pitch (deg)	Spacing (mm)	Truss	Qty	Span (mm)	Pitch (deg)	Spacing (mm)
	*HB1	2	3903	18.249	900	J6B	1	2145	25.000	900	*R5	1	1813	25.000	
	*HB2	2	4705	18.249	900	J6C	1	2145	25.000	900	*R5A	1	1813	25.000	900
	*HB3	2	5233	18.249	900	J7	1	1245	25.000	900	T1	1	4255	25.000	900
	*HB4	1	1273	18.249	900	J7A	1	1245	25.000	900	T10	1	7735	25.000	
	*HB5	2	6364	18.249		J7B	1	1245	25.000	900	T10A	3	7735	25.000	745
	*HB6	1	2429	18.249	900	J7C	1	1245	25.000	900	T11	1	2945	25.000	728
	J1	1	2105	25.000	900	J8	1	2945	25.000	900	T12	1	4255	25.000	927
	J1A	1	2105	25.000	900	J8A	1	2945	25.000	900	T12A	1	2945	25.000	927
	J1B	1	2105	25.000	900	J8B	1	2670	25.000	900	T13	1	3555	25.000	900
	J2	1	1205	25.000	900	J8C	1	2945	25.000	900	T14	1	1403	25.000	900
	J2A	1	1205	25.000	900	J8D	1	2945	25.000	900	T2	1	5389	25.000	900
	B	1	1205	25.000	900	J8E	1	2670	25.000	900	T2A	1	5389	25.000	900
Ĭ		1	1205	25.000	900	J8F	1	2945	25.000	900	Т3	1	6135	25.000	900
	JO	1	2672	25.000	900	J9	1	2045	25.000	900	T4	1	7735	25.000	900
	J3A	1	2672	25.000	900	J9A	1	2045	25.000	900	T5	1	7735	25.000	900
	J3B	1	2672	25.000	900	J9B	1	2045	25.000	900	T5A	1	7735	25.000	900
	J4	1	1772	25.000	900	J9C	1	2045	25.000	900	T6	3	6135	25.000	728
	J4A	1	1772	25.000	900	*R1	1	973	25.000	900	T7	1	6135	25.000	728
	J4B	1	1772	25.000	900	*R1A	1	973	25.000	900	T8	1	3845	25.000	748
	J4C	1	1772	25.000	900	*R2	1	1540	25.000	900	T8A	1	3845	25.000	748
	J5	1	3045	25.000	900	*R2A	1	1540	25.000	900	T8B	1	3845	25.000	748
	J5A	1	3045	25.000	900	*R3	4	968	25.000	900	T9	1	5389	25.000	900
	J5B	1	3045	25.000	900	*R3A	4	968	25.000	900	TG1	1	7735	25.000	900
	J6	1	2145	25.000	900	*R4	1	1013	25.000	900	V1	1	3593	25.000	898
	J6A	1	2145	25.000	900	*R4A	1	1013		900	, V2	1	1796	25.000	898
									ALIEN PORCH	TOPOTRETO	1 1 64 6 6 1 1 1	35 4 4	3 6 3		000

Total quantity: 89

ESE WILST RE SVAILABLE FOR MEWING AT OLL PIMES

se inspection will be made without west documents. A further inspection " av be charged if plans are unavailable.

PlaceMakers New Plymouth

Producer Statement : Page 1

Job:

Q1324T

Platinum Homes Client

Phone:

New Residence J / M Roberts 39a Egmont Street

Printed: 08:14:59 23 Aug 2007

Description: Building Consent No.: MTek 20/20 - Engineering 4.4 Gammen.5 (build 1597-51)

MiTek New Zealand Ltd.

The computer design input has been carried out by:

Date: 23/08/2007

Name of Computer Operator:

Kevin Gyde

Qualifications and Title:

Manufacturing Manager

Company:

PlaceMakers New Plymouth

Verification / Accep	tance of	Input	Data:
----------------------	----------	-------	-------

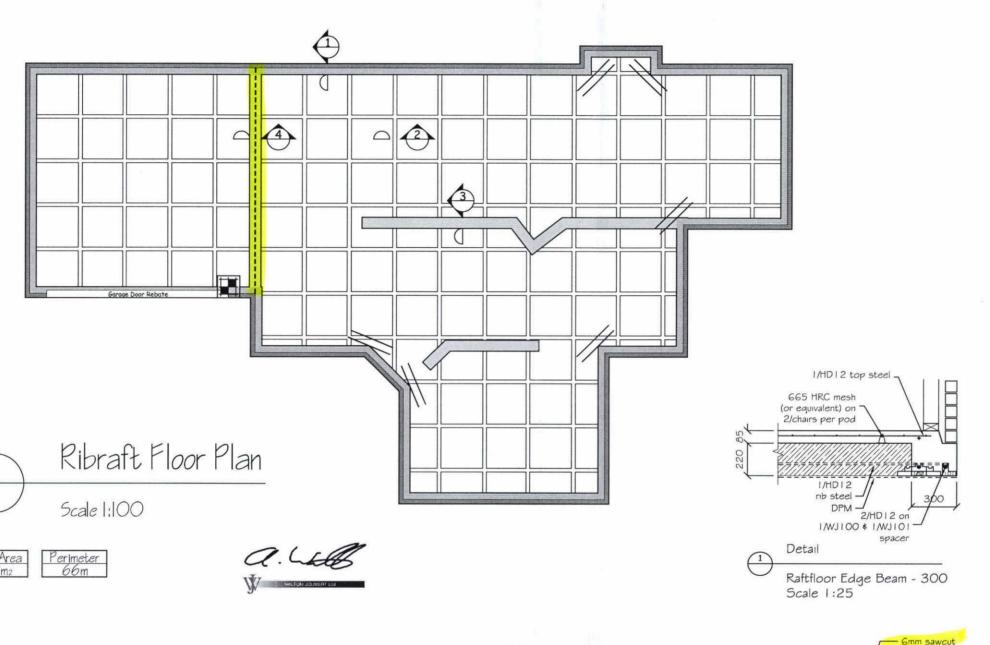
I have checked the input data against the construction drawings and specifications and verify that they are correct and suitable for this job.

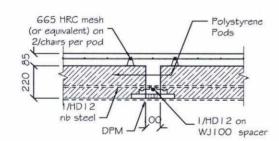
Date:

Company:

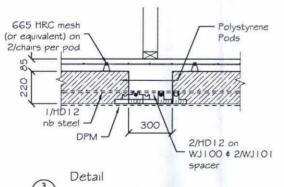
MEWING AT ALL TIMES Mc inspection will be made without these documents. A further inspection re may be charged it plans are unavallable.

Quantities of	f Spacers
(Quantities are	approximate
and to be used as	a quide only)
ltem	Qty.
WJ 100 Centre Spacer	171
WJ 101 Clip-on Spacer	119









Raftfloor Internal Rib - 300 Scale 1:25

NOTES:

Under no circumstances shall polystyrene spacers be used.

All Concrete strength shall be 20MPa minimum unless otherwise noted.

Where underfloor heating is installed, floor topping shall be increased to 110mm.

Do not scale from Drawings. Refer Architectural Drawings for overall dimensions.

Polystyrene pods shall be I I OOmm or 1200mm square.

Legend:



Re Entrant corner steel 2/HD12 x 1200mm at 200 crs

6mm sawcut 100mm deep



600x600x305 Pad with 3/HD12 each way

OI	add sawcut	27-8-07
Revision	Description	Date

Structural Engineers:

WILTON JOUBERT Ltd P.O. BOX 11-381 ELLERSLIE AUCKLAND

> Ph (09) 579 1114 Fax (09) 579 7778

100mm deep

Raftfloor Internal Rib -300

with Sawcut

Scale 1:25

- Polystyrene

IMJ100 \$ 2MJ101 ASE docume

665 HRC mesh

(or equivalent) on

2/chairs per pod

rib steel .

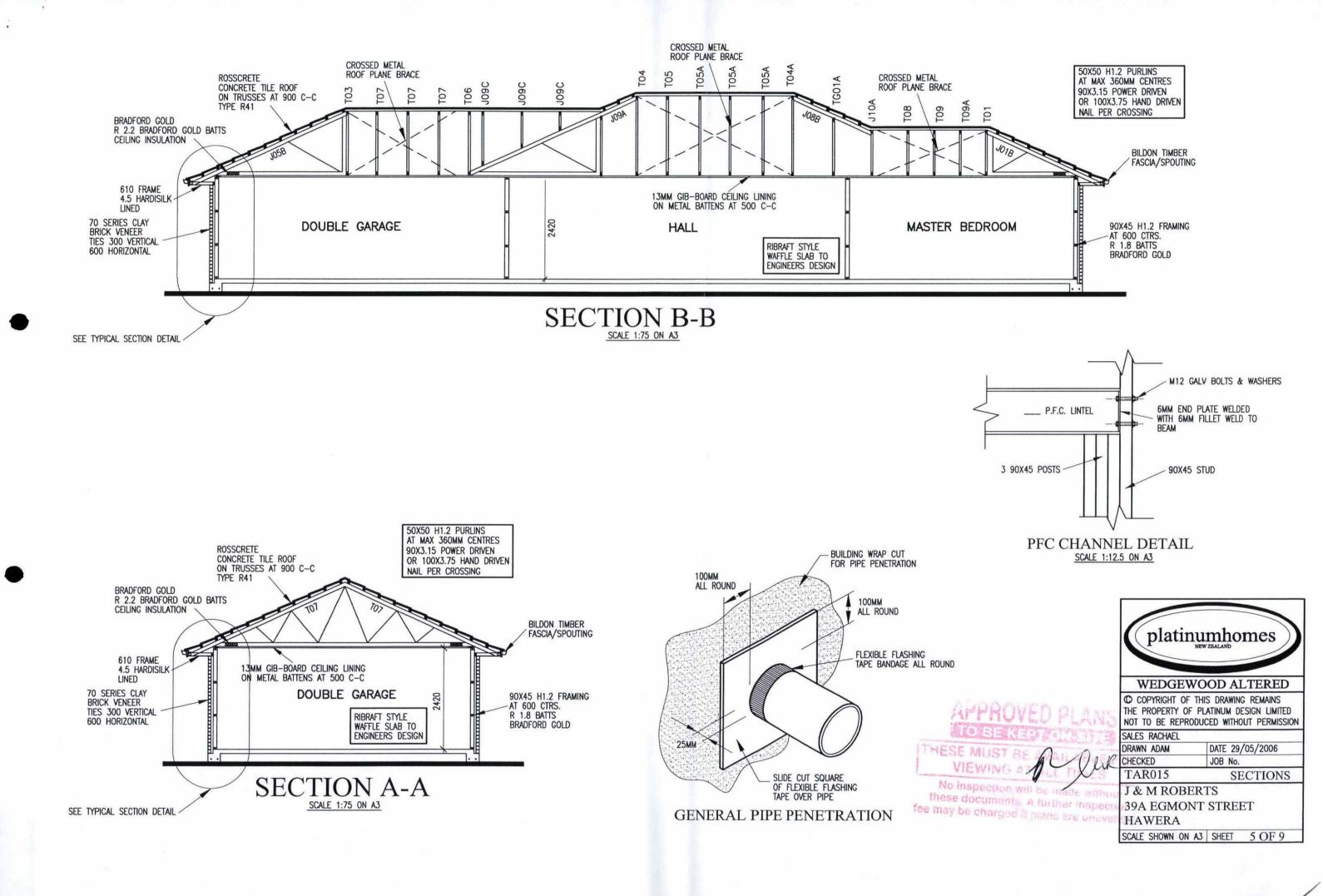
Proposed Residence Lot 2: DP 371240 39A Egmont Street

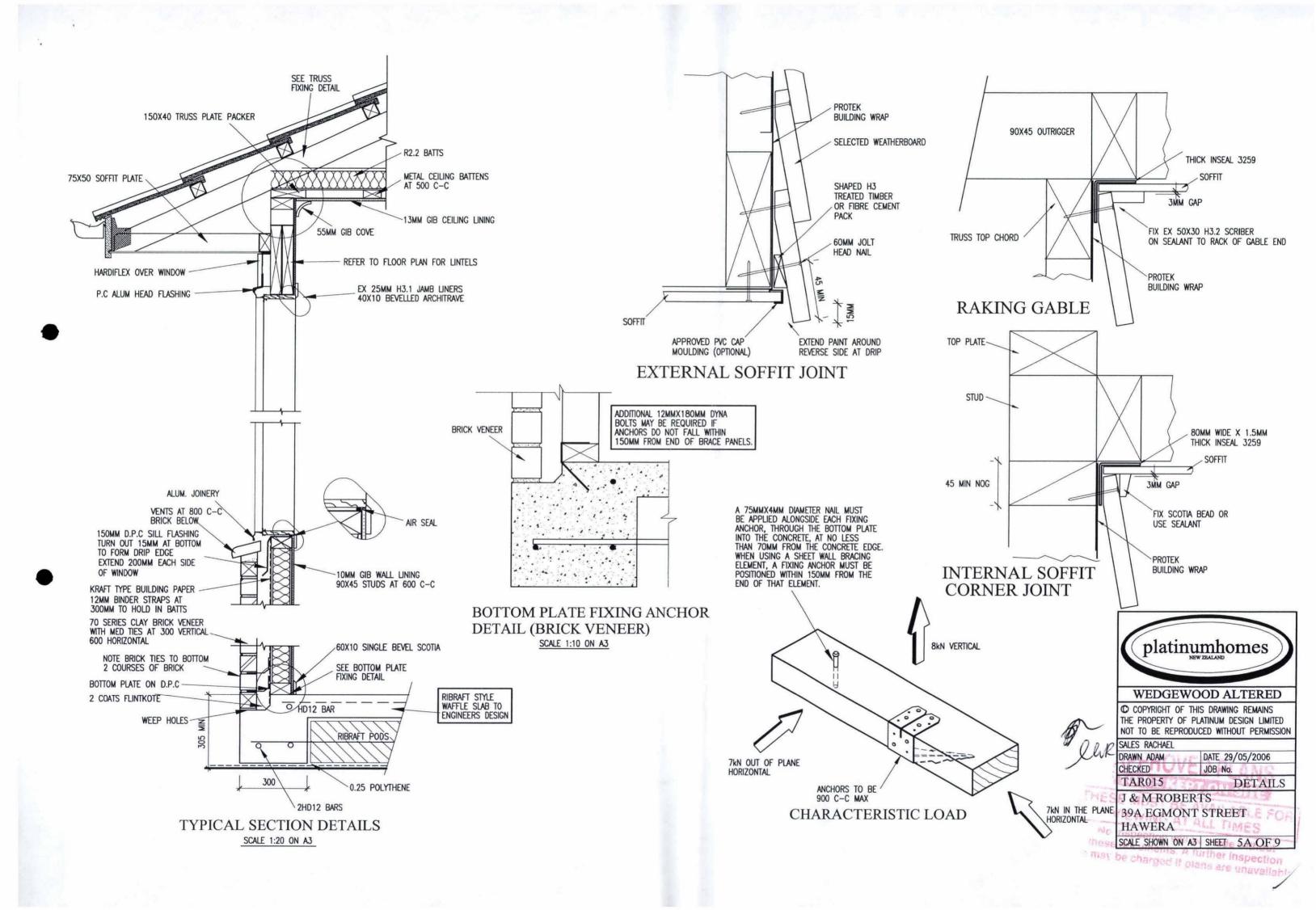
Hawera

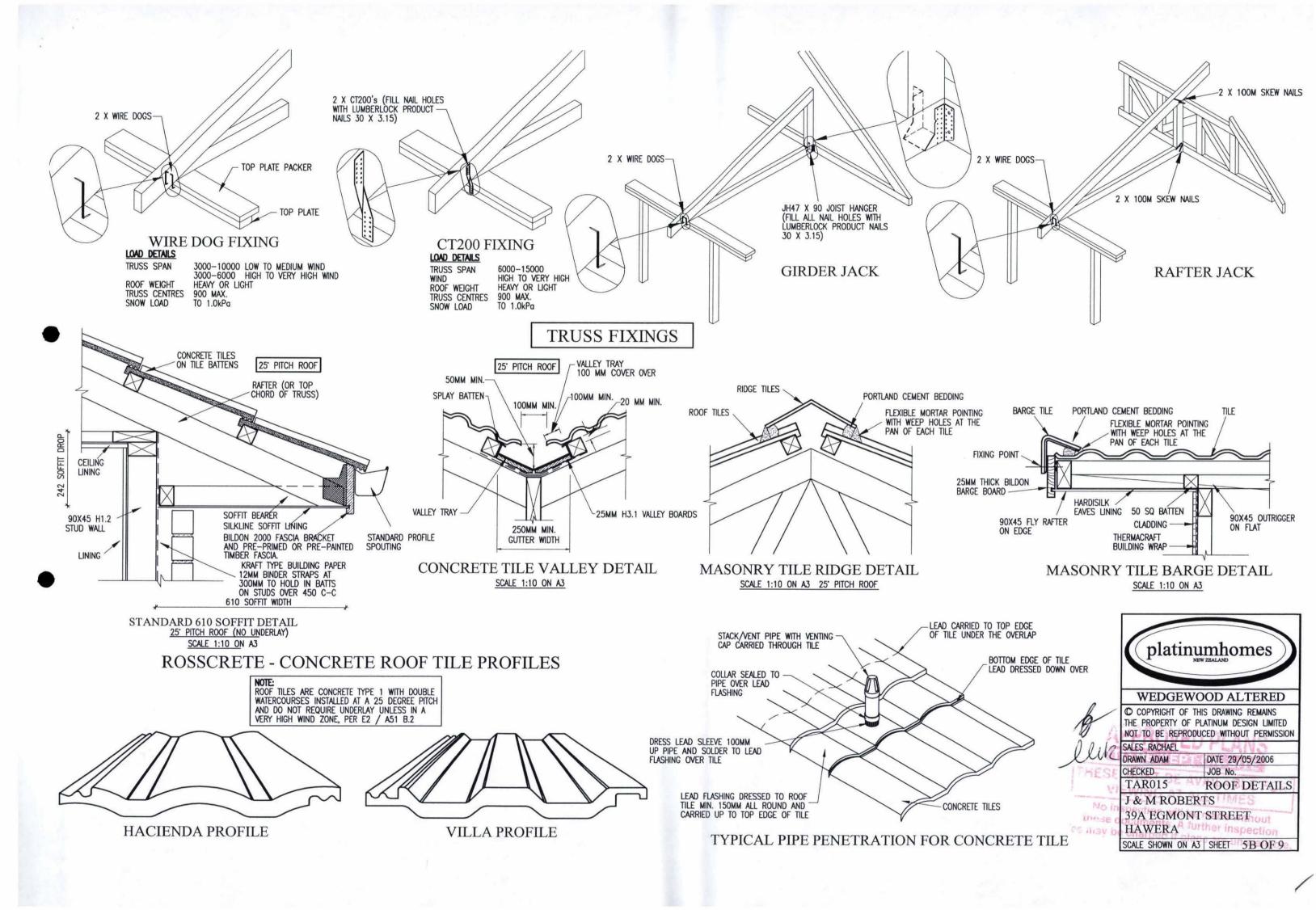


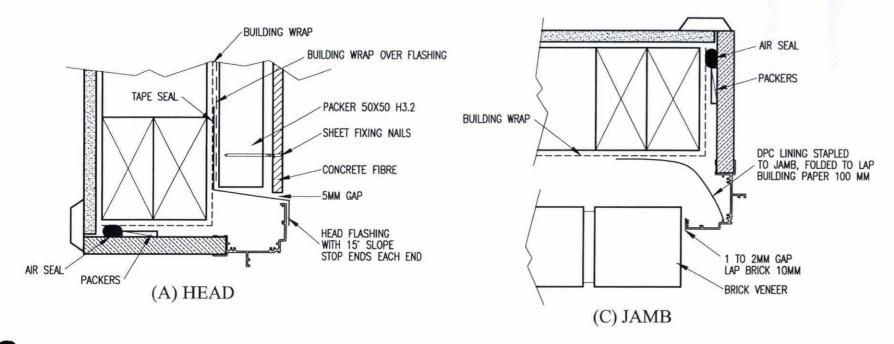
Job # 10280 revision 01

51









NOTE: JAMB FLASHINGS

TO OVERLAP SILL FLASHING

=======

(B) SILL

PACKERS -

AIR SEAL

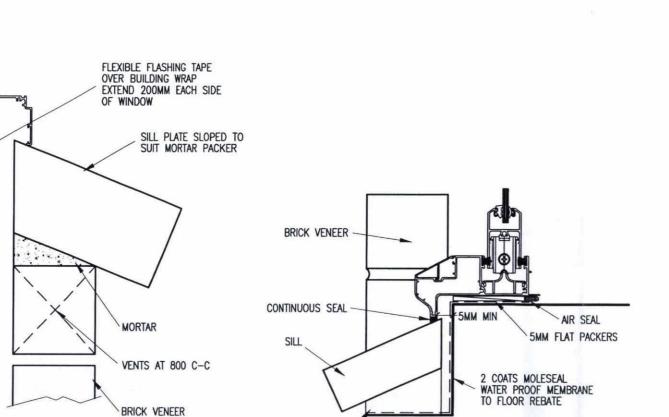
(1) WINDOW PROFILE TO BE SELECTED TO ACHIEVE COVER SHOWN IN DETAILS

ARCHITRAVES ARE SHOWN FOR CONSISTENCY ONLY, DETAIL MAY BE USED WITH REBATED LINER

MAKE ALLOWANCE BETWEEN PACKER AT SILLS FOR SUPPORT BRACKETS FOR LARGE WINDOWS. SUCH BRAKCETS REQUIRE SPECIFIC DESIGN, AND SHALL BE SUPPLIED BY THE WINDOW MANUFACTURER

BUILDING WRAP

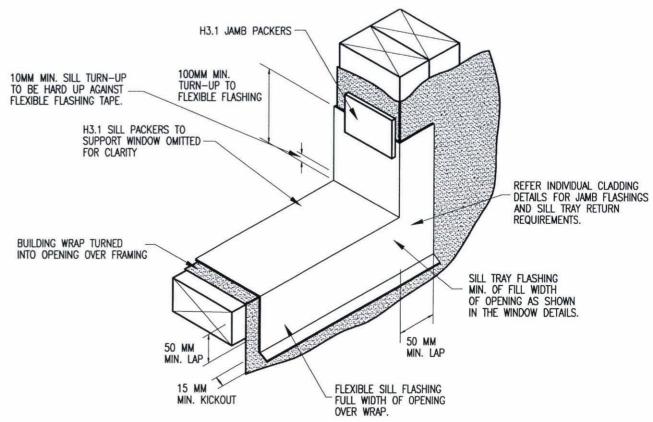
(4) WINDOW JAMBS TO BE H3.1 TAN



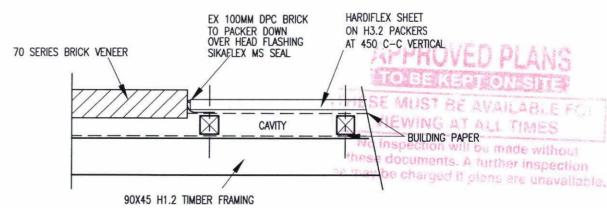
WEEP HOLES THRU

MORTAR PACK

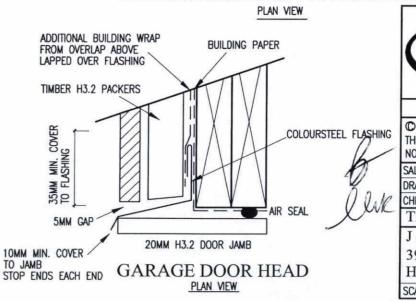
SLIDING DOOR SILL



WINDOW OPENING SILL DETAIL (BRICK) SCALE 1:5 ON A3



HARDIFLEX/BRICK VENEER DETAIL





WEDGEWOOD ALTERED

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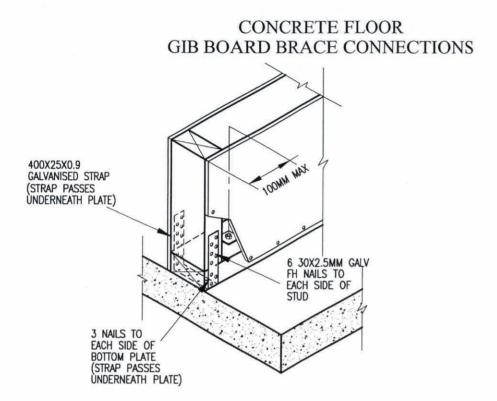
DATE 29/05/2006 JOB No. CHECKED TAR015 WINDOW DETAILS

J & M ROBERTS

39A EGMONT STREET

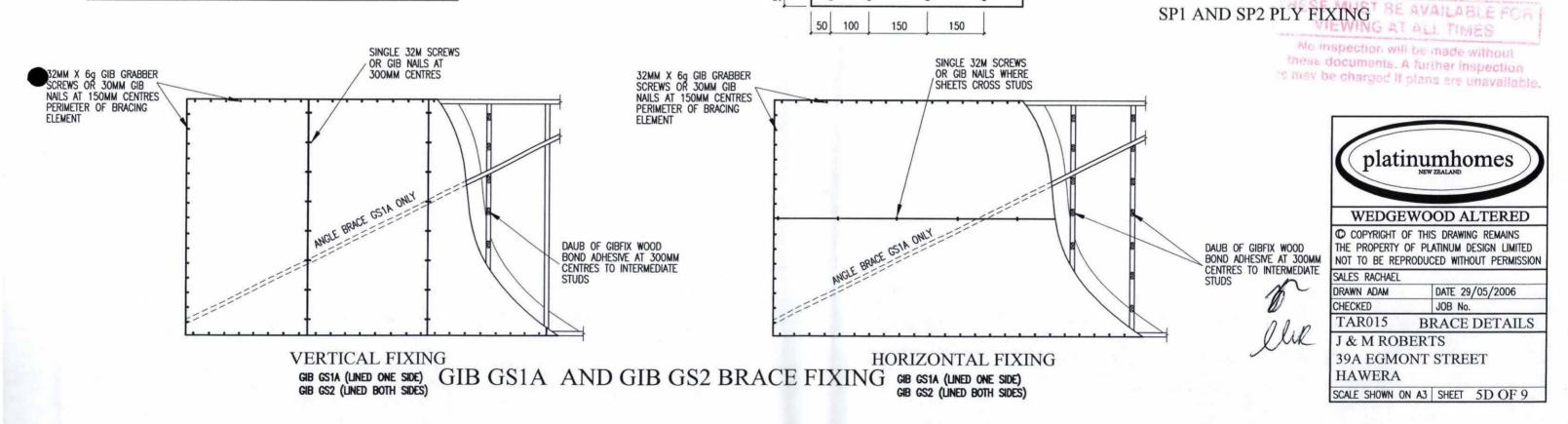
HAWERA

SCALE SHOWN ON A3 SHEET 5C OF 9



FASTENING THE GIBRALTAR BOARD BRACING ELEMENT TO CONCRETE SLABS

FASTENING OF THE BOTTOM PLATE OF A GIB WALL BRACING ELEMENT TO CONCRETE FLOORS MUST BE IN ACCORDANCE WITH NZS3604 FOR EXTERNAL WALLS, WHICH INCLUDES A 12MM BOLT (COMPLETE WITH A 50X50X3MM SQUARE WASHER) OR A PROPRIETARY FIXING WITH EQUIVALENT PERFORMANCE WITH 100MM FROM BOTH ENDS OF THE WALL BRACING ELEMENTS ON INTERNAL BRACING LINES, THE BOTTOM PLATE OF GIB BRACING ELEMENTS MAY BE FIXED USING 3.8MM SHOT FIRED FASTENERS FITTED WITH 16MM DISCS, SPACED AT 150MM AND 300MM FROM THE END STUDS AND THEREAFTER AT 600MM CENTRES.



PLYW00D

CONCRETE SLAB

150

150MM

MAXIMUM

CONCRETE SLAB ON GROUND

150MM

MAXIMUM

CONCRETE SLAB

30X2.5MM S/STEEL CLOUTS AT 150MM

TYPE 1 6KN FIXING STRAP

WRAPPED AROUND PLATE
AND FIXED WITH 12-30X2.5MM
S/STEEL CLOUTS; 6 INTO

OUTSIDE FACE OF STUD AND

6 INTO UNDERSIDE OF

12MM BOLT WITH 50X50X3MM WASHER, EMBEDDED 75MM INTO CONCRETE WITH CRANKED END

BOTTOM PLATE

OF SHEET

30X2.5MM S/STEEL CLOUTS AT 300MM CENTRES TO ALL INTERMEDIATE FRAMING

CENTRES TO PERIMETER

PLYWOOD

DPC

6KN PLY BOTTOM PLATE FIXINGS

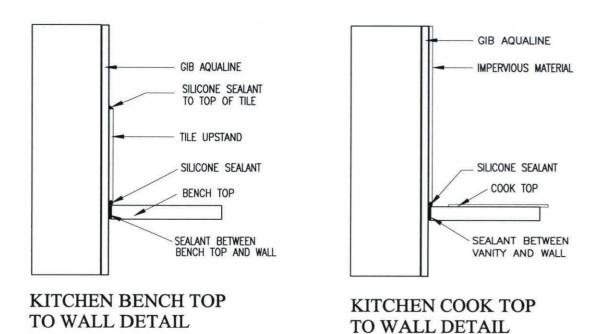
THIS PATTERN APPLIES TO CEILING AND ALL WALL BRACING EXCEPT BR9

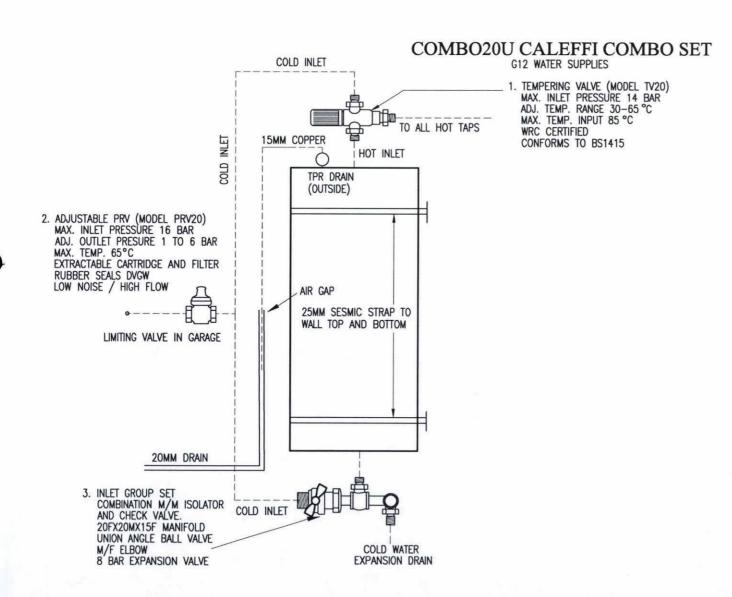
12MM FROM TAPERED

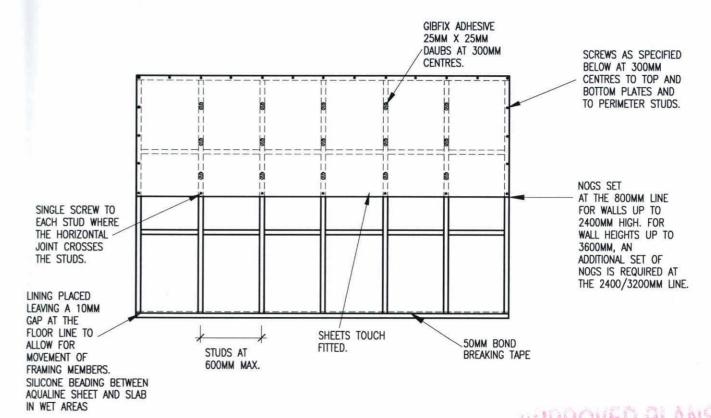
18MM FROM CUT EDGE

PAPER BOUND EDGE

CORNER FLASHING DETAILS







GIB AQUALINE FIXING DETAILS - HORIZONTAL FIXING

THESE MUST BE AVAILABLE FOR VIEWING AT ALL TIMES

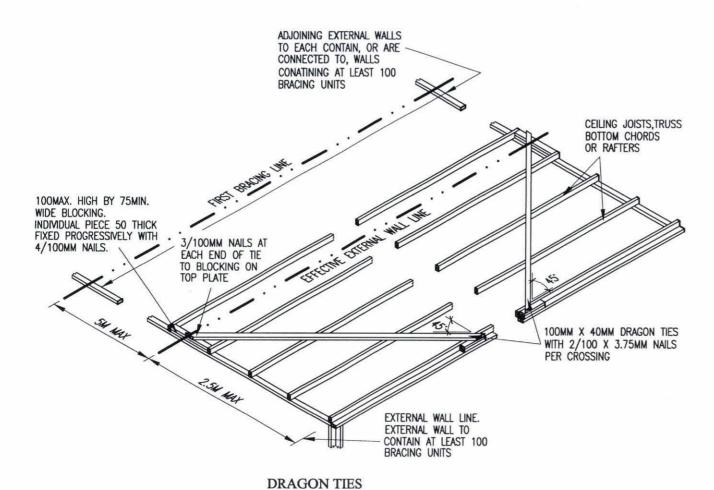
No inspection will be made without these documents. A further inspection es may be charged if plans are unavailable.

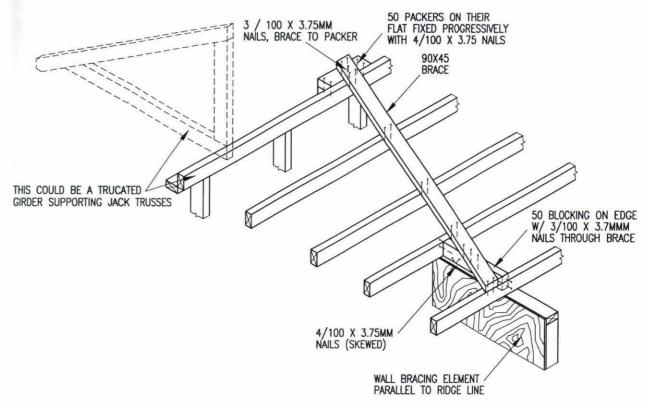
BUILDING WRAP FOLDED SHEET-STEEL BUILDING WRAP OVER FLASHING BRICK CLADDING TAPE SEAL WEEP HOLE 30X30X3 GALV.STEEL ANGLE TO 4 SIDES GALV.STEEL ANGLE 100MM FOOT BOTH WAYS 5MM RIVETS TO SECURE ANGLE TO SILICON BEAD **METERBOX** FLEXI-TAPE SEAL 4 SIDES METERBOX TIMBER FRAMING

DETAILS OF DCB-15 METER BOX FLASHING IN BRICK VENEER

BUILDERS TO FIX FLASHING TO THE METERBOX WITH ADHESIVE AT THE APPROPRIATE PLACE TO SUIT THE CLADDING.







CEILING PLANE BRACE

(CEILING BRACES CONNECTING HIP-END TOP PLATES TO WALL BRACING ELEMENTS)

APPHUVED PLANS

VIEWING AT ALL TIMES

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DRAWN ADAM DATE 29/05/2006
CHECKED JOB No.

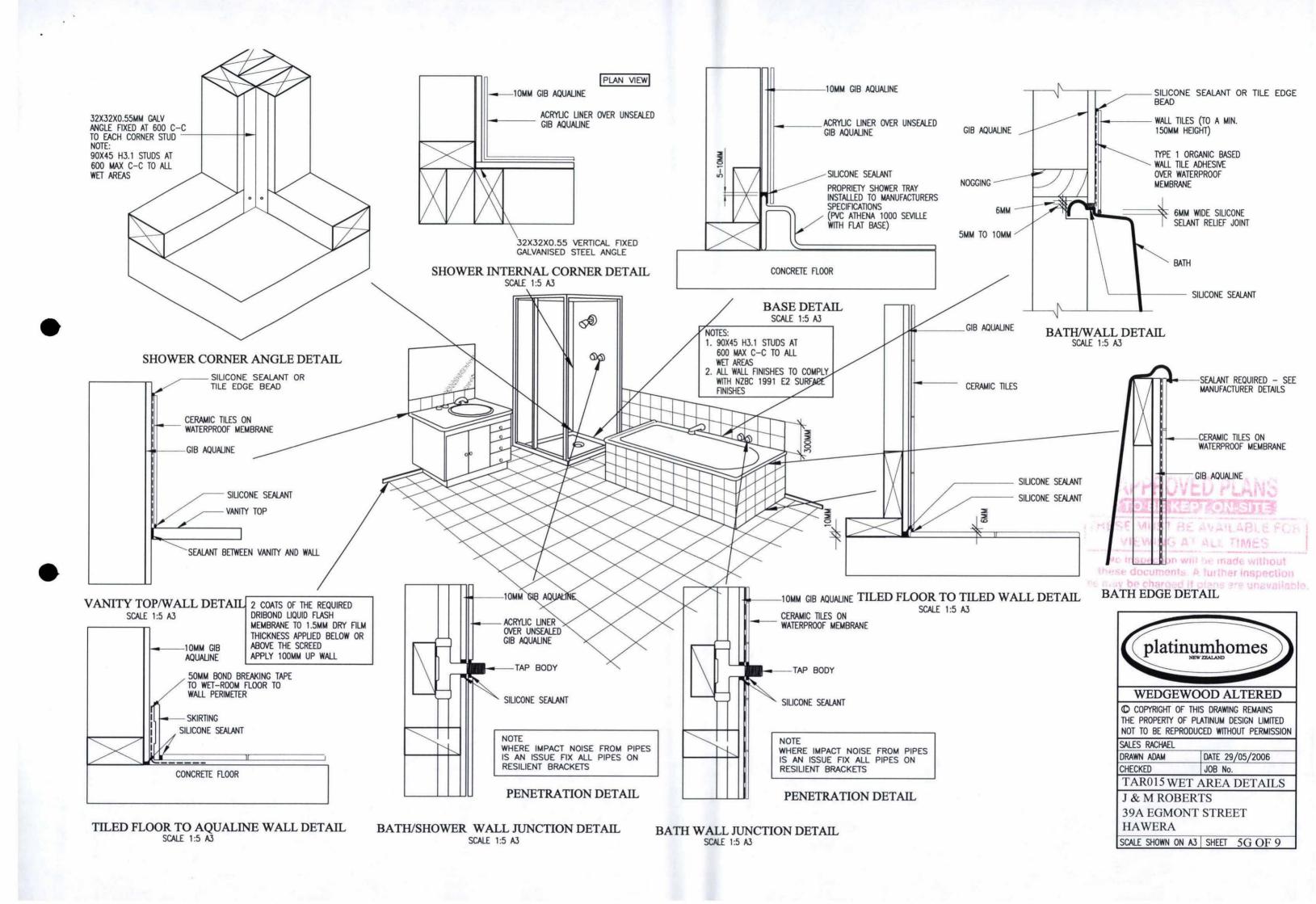
TAR015 ROOF BRACING

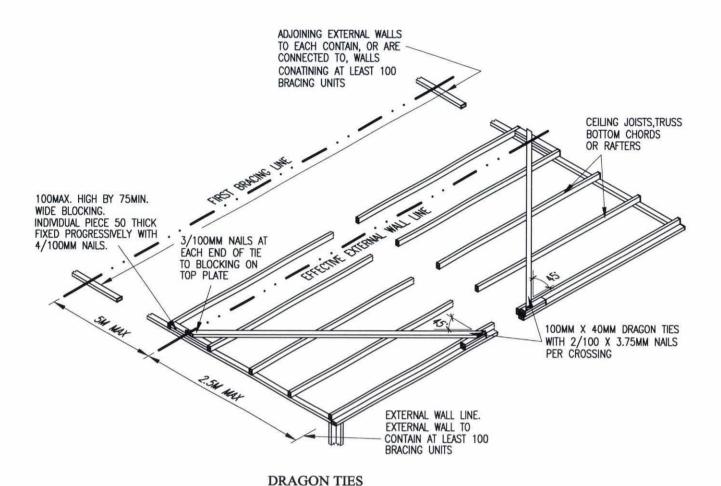
J & M ROBERTS

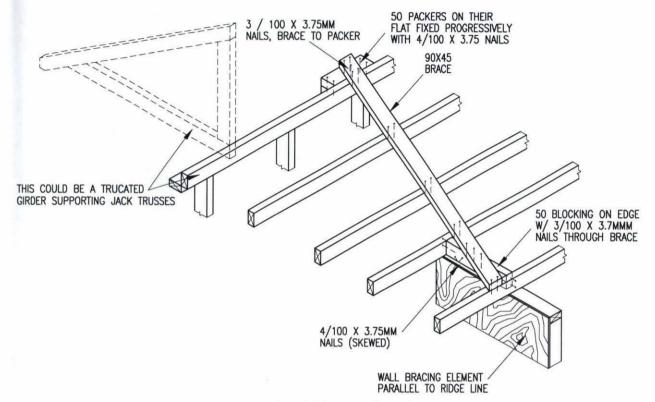
39A EGMONT STREET HAWERA

SCALE SHOWN ON A3 SHEET 5F OF 9









CEILING PLANE BRACE

(CEILING BRACES CONNECTING HIP-END TOP PLATES TO WALL BRACING ELEMENTS)

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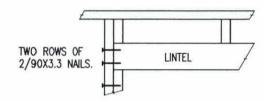
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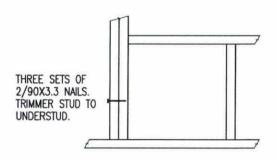
39A EGMONT STREET

HAWERA

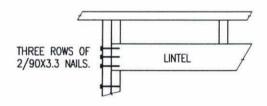
SCALE SHOWN ON A3 SHEET 5F OF 9

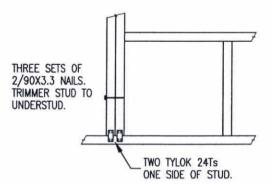
LINTEL FIXINGS



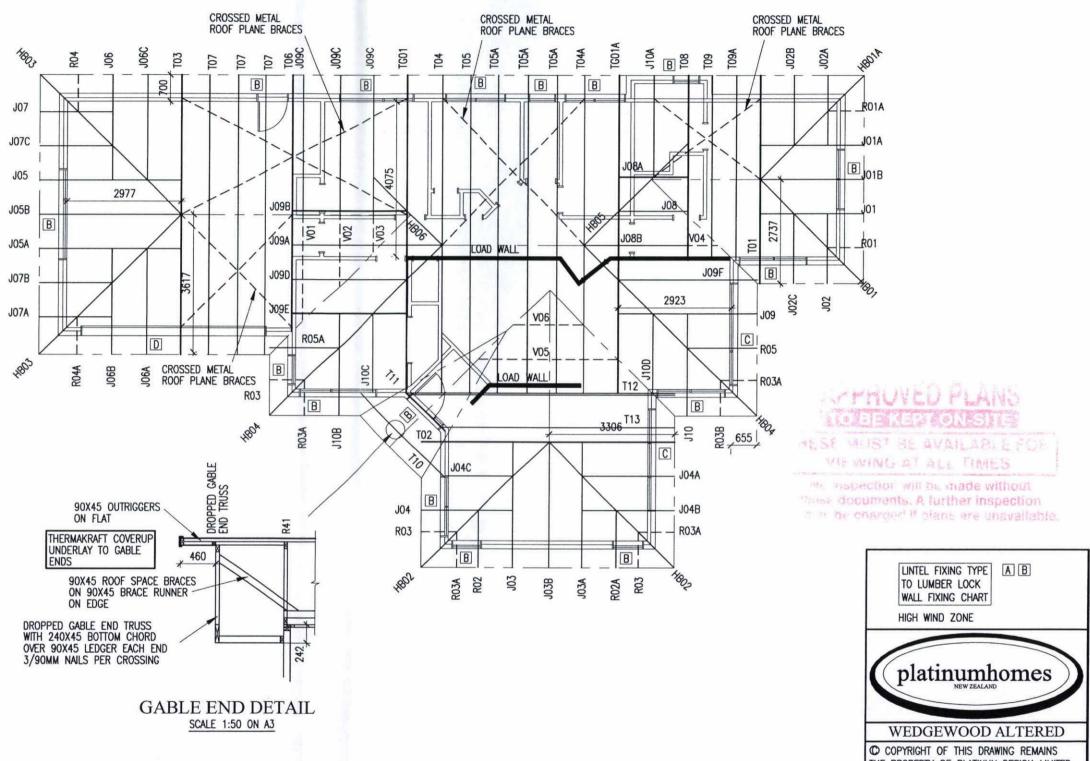


TYPE A (1.4 kN)





TYPE B (4.0 kN)



llu

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DRAWN ADAM CHECKED

CHECKED JOB No.

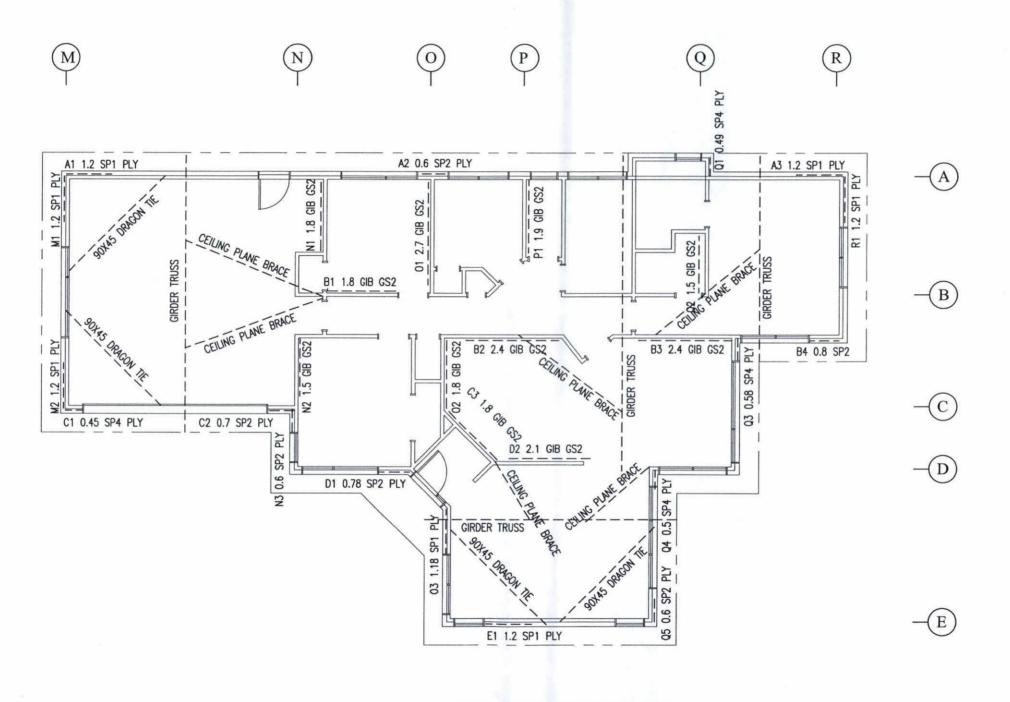
TAR015 ROOF PLAN

DATE 29/05/2007

J & M ROBERTS 39A EGMONT STREET

HAWERA

SCALE 1:100 ON A3 | SHEET 6 OF 9



REFER CEILING PLANE BRACES AND DRAGON TIES SHEET 5F

THE WING AT ALL TIMES

Ms inspection will be made without these documents. A further inspection



WEDGEWOOD ALTERED

ROOF AREA HOUSE 200.80 SQ METRES FLOOR AREA HOUSE 168.17 SQ METRES 20.50 METRES BUILDING LENGTH TOTAL WIDTH 12.465 METRES WIND ZONE HIGH CORROSION ZONE EARTHQUAKE ZONE

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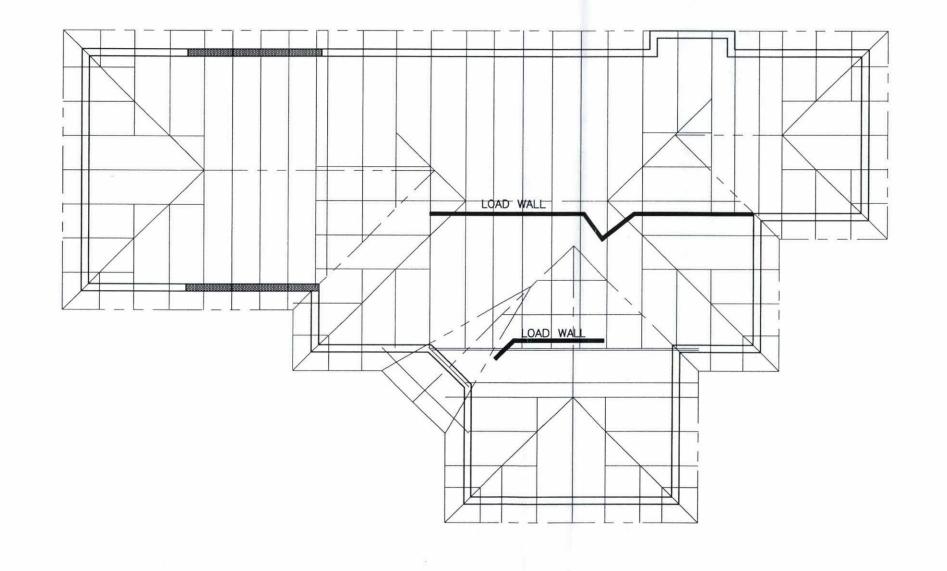
SALES RACHAEL

DATE 29/05/2007 DRAWN ADAM JOB No. CHECKED

TAR015 **BRACING PLAN**

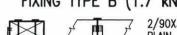
J & M ROBERTS 39A EGMONT STREET

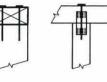
HAWERA SCALE 1:100 ON A3 SHEET 7 OF 9





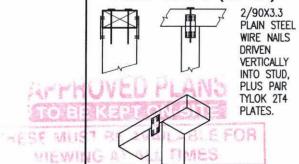
FIXING TYPE B (1.7 kN)





2/90X3.3
PLAIN STEEL
WIRE NAILS
DRIVEN
VERTICALLY
INTO STUD,
PLUS SINGLE
TYLOK 2T4
PLATE.

FIXING TYPE C (2.7 kN)



these documents. A further inspection as may be charged it plans are unavailable.

TOP PLATE FIXINGS

TOP PLATE FIXINGS TYPE '_'
TO LUMBERLOK WALL FIXING CHART

FIXING TYPE B

FIXING TYPE C

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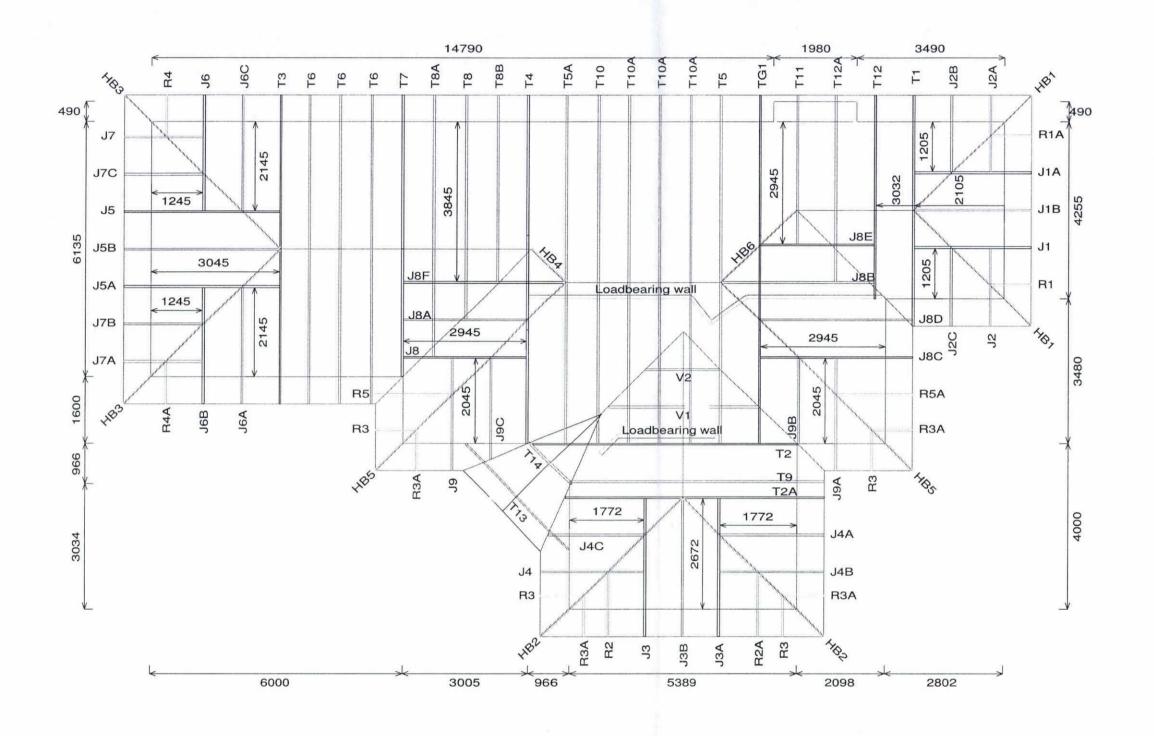
TAR015 TOP PLATE FIXINGS

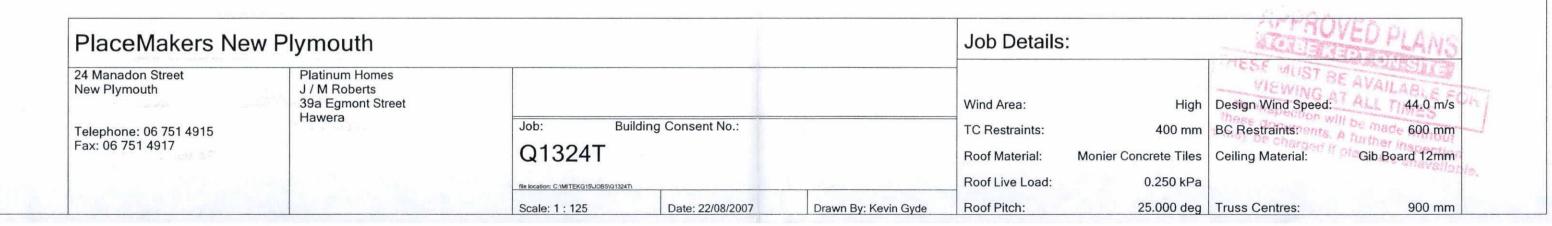
J & M ROBERTS 39A EGMONT STREET

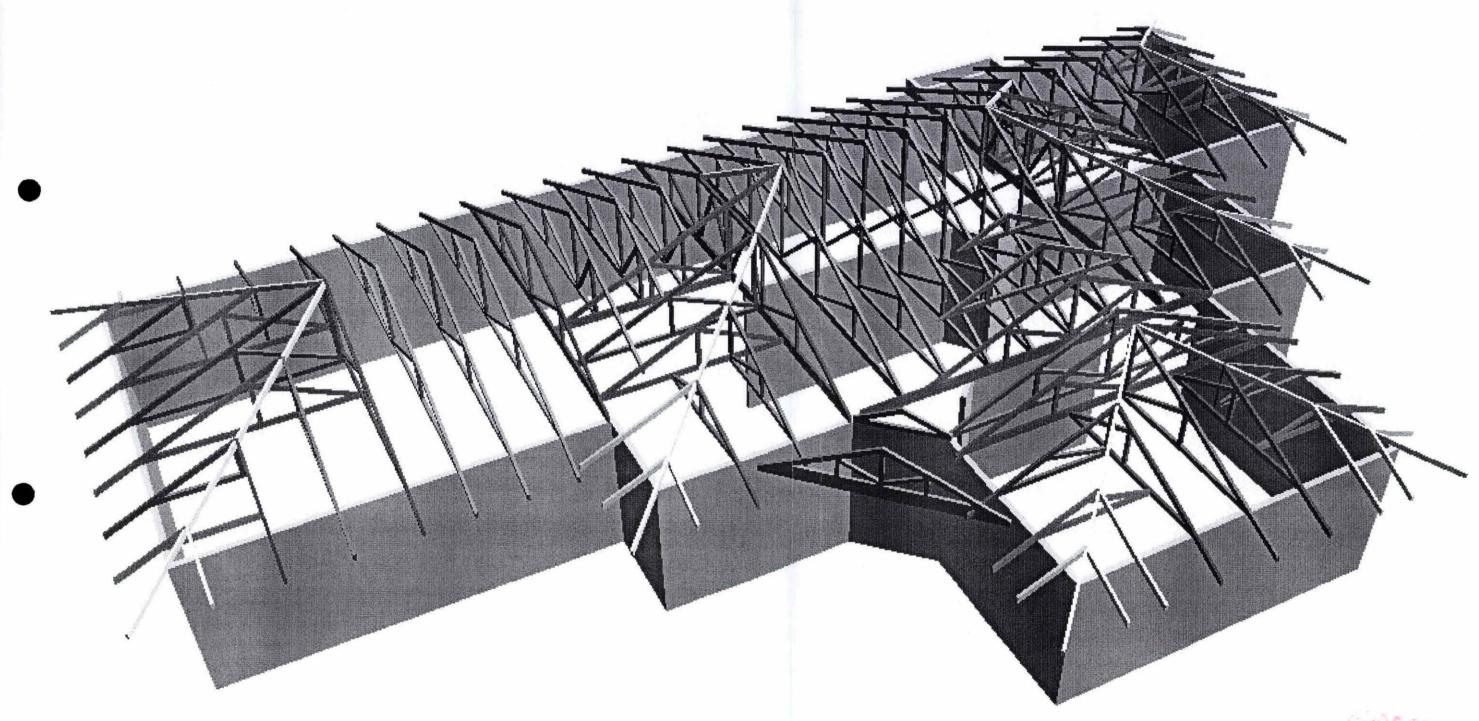
HAWERA

SCALE 1:100 ON A3 | SHEET | 8 OF 9

Dew







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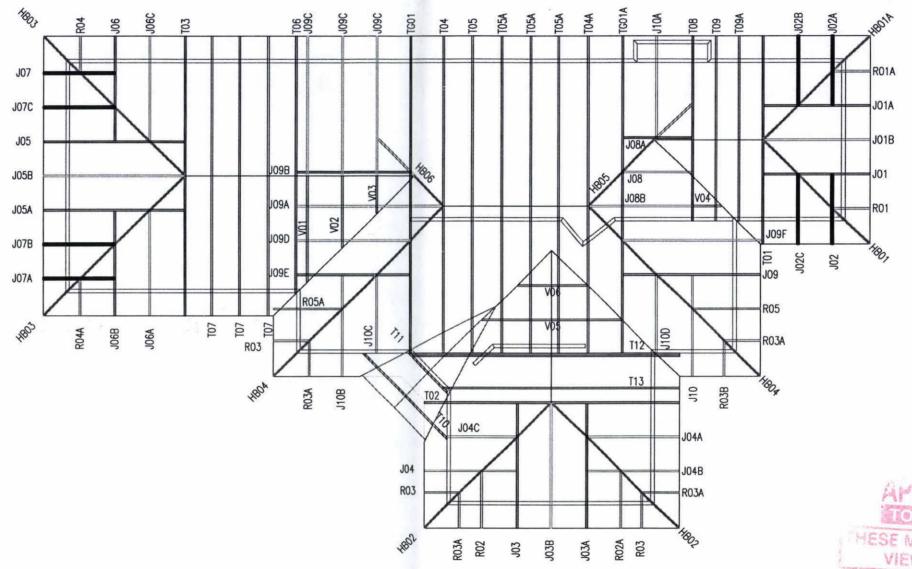
FIXING LEGEND

 $_{\mathsf{X}}$ 45×90JH _H 45×120JH _J 95x165JH $_{\mathsf{M}}$ 1 Pr Multigrips _# 1 Pr CT200 _C CT400 _0 CPC80 Load bearing walls =LB

All trusses to be fixed to building top plates with Minimum 1-pair) of wiredogs each end unless otherwise specified as per this layout plan.

NOTES Dimensions shown are to the face side of the truss, nearest too the point of the dimension line origin

On site remedial work should not be carried out prior to Katikati Building Supplies being advised and arried out by them, or such work has been autherised. Katikati Building Supplies will not accept any charges for remedial work unless autherised by them.



VIEWING AT

No inspection will be made without these documents. A turther inspection fee may be charged it plant are unavailable

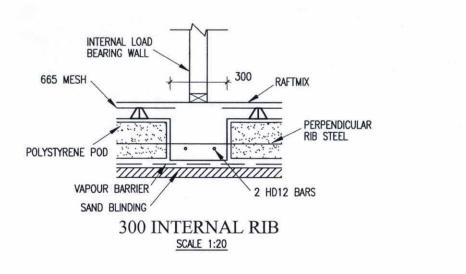
J & M ROBERTS 39A EGMONT STREET **HAWERA**

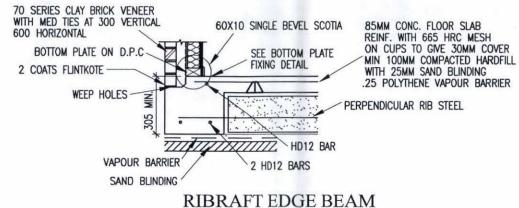
PO Box - 243, KATIKATI, Phone: 07-549-0689, Fax: 07-549-0702

Job No: TAR015

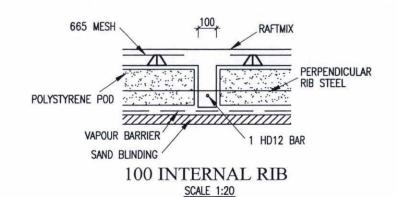
Drawn By: DAVE

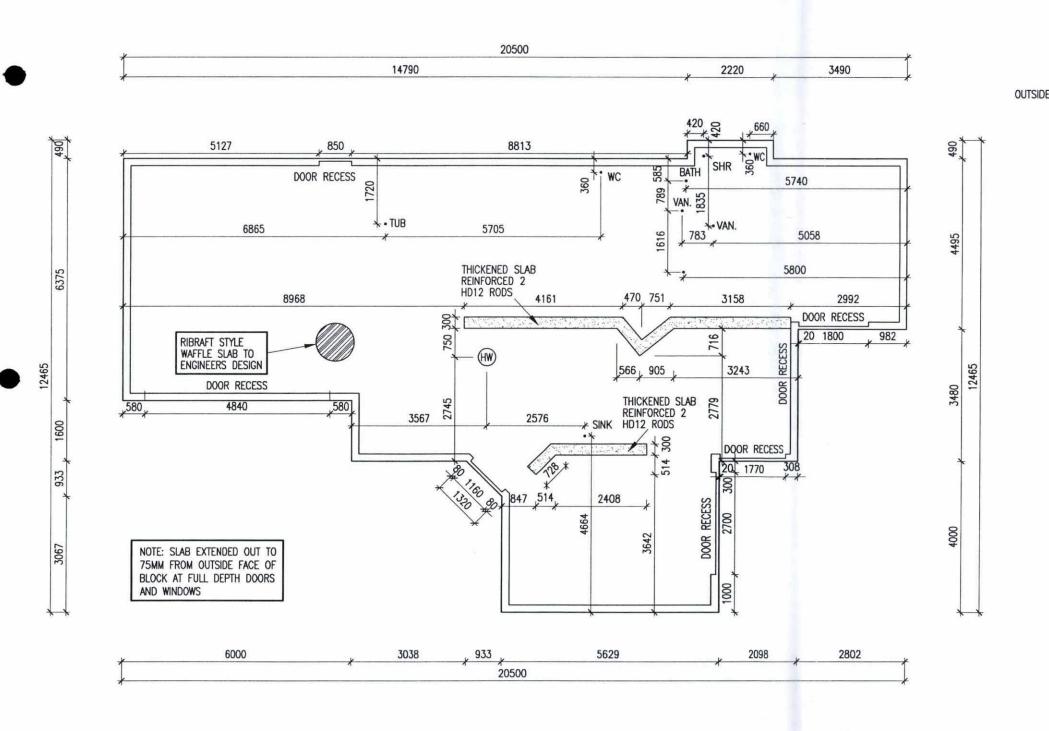
Date: 30/05/07

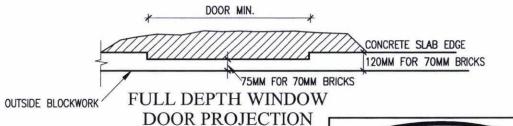




SCALE 1:20







SCALE 1:25 ON A3

these documents. A further inspection

e may be charged if plans are unavail

WEDGEWOOD ALTERED

platinumhomes

NOTES

- . ALL FOUNDATIONS SHALL BEAR INTO FIRM UNDISTURBED SUBSOILS WITH MIN. BEARING CAPACITY OF 100 KPA. NOTIFY ENGINEER IF SOFT SUB SOILS ARE ENCOUNTERED.
- 2. CONCRETE STRENGTH SHALL BE 17.5 MPA AT 28 DAYS. SOLID FILL ALL BLOCKWORK REINF. CORES AND BOND BEAMS 20MPA 10MM AGGREGATE
- 3. LAP ALL REINFORCING 40 BAR DIAMETERS AT ALL INTERSECTIONS AND WHEREVER ELSE NECESSARY.
- . ALL BLOCKWORK SHALL COMPLY WITH THE REQUIREMENTS OF N.Z.S. 4210:1989 MASONRY CONSTRUCTION, MATERIALS AND WORKMANSHIP
- BECAUSE OF THE POSSIBILITY OF CRACKS DEVELOPING IN CONCRETE FLOOR DURING CURING PROCESS PLATINUM HOMES (NZ)LTD CANNOT GUARANTEE AGAINST TILES CRACKING

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SALES RACHAEL DRAWN ADAM DATE 29/05/2007 CHECKED JOB No. TAR015 **FOUNDATION**

J & M ROBERTS 39A EGMONT STREET

HAWERA

SCALE 1:100 ON A3 SHEET 4 OF 9



RML070079

13 August 2007

Jason and Marea Roberts 237B South Road Hawera

Dear Jason and Marea

Resource Consent Application – Erect a new Dwelling, 39A Egmont Street, Hawera

Your application was considered by the Planning Manager under delegated authority on 10 August 2007 and she has resolved as follows:

<u>THAT</u> consent be granted to Jason and Marea Roberts to erect a new dwelling 1.5 metres from the south-eastern boundary and 1.5 metres from the north-eastern boundary of the property at 39A Egmont Street, Hawera (land legally described as Lot 2 DP 371240), pursuant to Sections 104 and 108 of the Resource Management Act 1991.

For the following reasons:

- 1. Written approval has been received from all affected parties.
- 2. The proposal would not detract from the amenity of the surrounding area.
- 3. The proposal would not give rise to any adverse environmental effects that are more then minor.

Subject to the following condition:

 That the dwelling is positioned in accordance with the site plan submitted to the South Taranaki District Council and signed by the affected parties.

It is noted that your Building Consent (BC070650) has not yet been issued. Once your building consent is issued you may commence building work.

Council draws your attention to the rights of objection to which this Consent is subject, as set out in Section 357 of the Resource Management Act 1991.

Enclosed is an invoice for the costs associated with processing this Consent.

If you have any queries relating to the above decision, please do not hesitate to contact me.

Yours sincerely

Carl Diamond
Environmental Monitoring Officer

Tax Invoice/Statement: 4292

GST Reg. No: 52-769-477 Invoice Date: 13 August 2007

J A Roberts, M A Roberts

237 B South Road

Hawera 4610



Address:

Private Bag 902

Phone:

Hawera 06 278 0555 0800 111 323

Free Phone: Facsimile:

06 278 8757

Email: Website: contact@stdc.govt.nz

www.stdc.co.nz

Application Number:

RML070079

Description:

Yard Dispensation to erect new dwelling

Owner:

J A Roberts, M A Roberts

Location Address:

39 A Egmont Street Hawera 4610

Charge Description	Charge	GST	Amount
Non Notified Decision Fee Resource Consent Administration Charge Resource Consent Processing Fee (Field) Planning Assessment	\$40.00 \$13.78 \$21.11 \$152.89	\$5.00 \$1.72 \$2.64 \$19.11	\$45.00 \$15.50 \$23.75 \$172.00
Total	\$227.78	\$28.47	\$256.25

Remittance Advice

Please post to:

South Taranaki District Council Private Bag 902 Hawera

Application Number: RML070079 Applicant: J A Roberts, M A Roberts

Amount Due: \$256.25



RESOURCE CONSENT APPLICATION

Application Number:	RM L 070079	Non-Notified	Notified	Building Consent
Applicant: J \$	m Roberts	1 11		
	A Egmont Stree	t, Hawe	ra.	Manya
Described: 123	30,118,01	(10	01203	
Proposal:				
Application Received	3, 8, 2007	Receipt No	<u>).:</u>	
Deposit Paid	1	\$		
Application Type (Please	e complete) Yard Dispersion new du	ensation	to e	red
	rew du	re lling		

NOTIFICATION/NON-NOTIFICATION
Type of Resource Consent: Land Use
Activity: Yord Repensation to erect new chaetling
Activity: Total population to out her ownering
Activity Status: Duchonary
ACTIVITY STORIES: VOCERGA OF
Reasons for Notification or Non-Notification
ENVIRONMENTAL EFFECTS ARE MINOR
ENVIRONMENTAL EFFECTS ARE
WITHIN PERMITTED BASELINE
AFFECTED NEIGHBOURS HAVE GIVEN CONSENT
Senior Planner Bluffff
Approved/Not Approved
1 m/2
Stephen
Authorised Officer

Report by:	Applicant:
Address of Property:	
Zone:	Activity Status:
Statutory and District Plan Requirements:	
Evaluation:	
Environmental Effects are minor	
Environmental Effects are within Permitted	Baseline
Affected Parties have given their written app	proval and any effects on them cannot be taken into account
Recommendation:	
For the Following Reasons:	
1)	
2)	
Subject to the Following Conditions:	
1)	
2)	
Planner:	Senior Planner:
APPROVED/NOT APPROVED	
Date	Authorised Officer

Time/Charges Associated with Resource Consent

	Planning Assessment	(\$86)
Date	Time Spent	Notes
3/00/07	13	Fither into leg
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	4	
		,
		-
	Site Visit (\$95)	
Date	Time Spent	Notes
7/08/07	4	
1242		
		Maria de mare esta se anticidade de la constanta de la constan
	Planning Report (\$8	86)
Date	Time Spent	Notes
7/08/07	1.00 - 1.30 =	Report.
	14	. ,
	Decision Correspondence	ce (\$86)
Date	Time Spent	Notes
	玄	
	Engineering Assessmer	nt (\$92)
Date	Time Spent	Notes
NYS (1745) 6071		

RESOURCE CONSENT APPLICATION CHARGE OUT SHEET

Date	Activity	Officer	Charge Out	Hours	Amount
3/8	Admin I file Set up Non Notified Fee	SU		25	15-50
3/8	Non Notified Fee	M			45-00
101.0	0 1			2	11 30
801	Planing Assement	CD	36-	3	64.50
18/01	Planing Assessment Ste Voit Planing Report Revior Correspondence	(D	95-	14	23.75
18/0/	1 KINING REPORT				64.50
190/	RUNON COMPONDENCE	CD	86-	之	43.00
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			777		
			7		
4-2-2-					



Land Use Consent Report

RML070079

To

Alison Gibson, Planning Manager

From

Carl Diamond, Environmental Monitoring Officer

Date

9 August 2007

Subject

Yard Dispensation - Erect a New Dwelling, 39A Egmont Street, Hawera

Application Number:

RML070079

Applicant:

Jason and Marea Roberts

Location of Activity:

39A Egmont Street, Hawera

Legal Description:

Lot 2 DP 371240

Zone:

Residential

Type of Activity:

Discretionary

DISTRICT PLAN REQUIREMENTS

Section 4 - Residential Zone

Discretionary Activities

4.01.4(a)

Any activity other than an activity listed in sections 4.01.5 or 4.01.6 below which cannot meet the performance standards set out or referred to in sections 4.02 and 4.03 below which and has not been granted a resource consent as a controlled activity.

Performance Standards - Bulk and Location

- 4.02.1(2) Rear sites not directly adjacent to a public road:
 - (a) A building (other than an accessory building), shall be located no closer than:
 - (ii) 3 metres to any site boundary.

APPLICATION

The applicants, Jason and Marea Roberts, are proposing to build a new dwelling on the property at 39A Egmont Street, Hawera (land legally described as Lot 2 DP 371240). The property is zoned residential.

The applicants' property is a rear section on Egmont Street. The proposed dwelling would be unable to meet the District Plan's bulk and location requirements and as such is required to be assessed as a Discretionary Activity.

EVALUATION

The District Plan requires that a building on a rear site within a Residential Zone shall be located no closer than 3 metres from any site boundary.

The proposed new dwelling would be located 1.5 metres from the property's south-eastern boundary and 1.5 metres from the property's north-eastern boundary. Therefore, the dwelling would be unable to meet the 3 metre setback required. The proposed new dwelling would comply with all other applicable bulk and location requirements.

The District Plan's provisions are in place to ensure that ongoing development does not detract from the amenity values of the surrounding area. The dwelling would be a new dwelling similar to other dwellings recently constructed within Hawera. It is considered that the proposed dwelling would not detract from the amenity of the surrounding residential area and the proposal would not give rise to any environmental affects that are more than minor.

Written approval has been received from all parties considered to be affected by the proposal.

Conclusion

The applicants' propose to erect a new dwelling on the property located at 39A Egmont Street, Hawera.

It is considered that the proposal would not detract from the amenity of the surrounding area and that the proposal would not give rise to any environmental effects that are more than minor.

RECOMMENDATION

THAT consent be granted to Jason and Marea Roberts to erect a new dwelling 1.5 metres from the south-eastern boundary and 1.5 metres from the north-eastern boundary of the property at 39A Egmont Street, Hawera (land legally described as Lot 2 DP 371240), pursuant to Sections 104 and 108 of the Resource Management Act 1991.

For the following reasons:

- 1. Written approval has been received from all affected parties.
- 2. The proposal would not detract from the amenity of the surrounding area.
- 3. The proposal would not give rise to any adverse environmental effects that are more then minor.

Subject to the following condition:

1. That the dwelling is positioned in accordance with the site plan submitted to the South Taranaki District Council and signed by the affected parties.

RML070079 - Yard Dispensation, Jason and Marea Roberts, 39A Egmont Street, Hawera

Carl Diamond

Environmental Monitoring Officer

[Seen by] Blair Sutherland **Senior Planner**

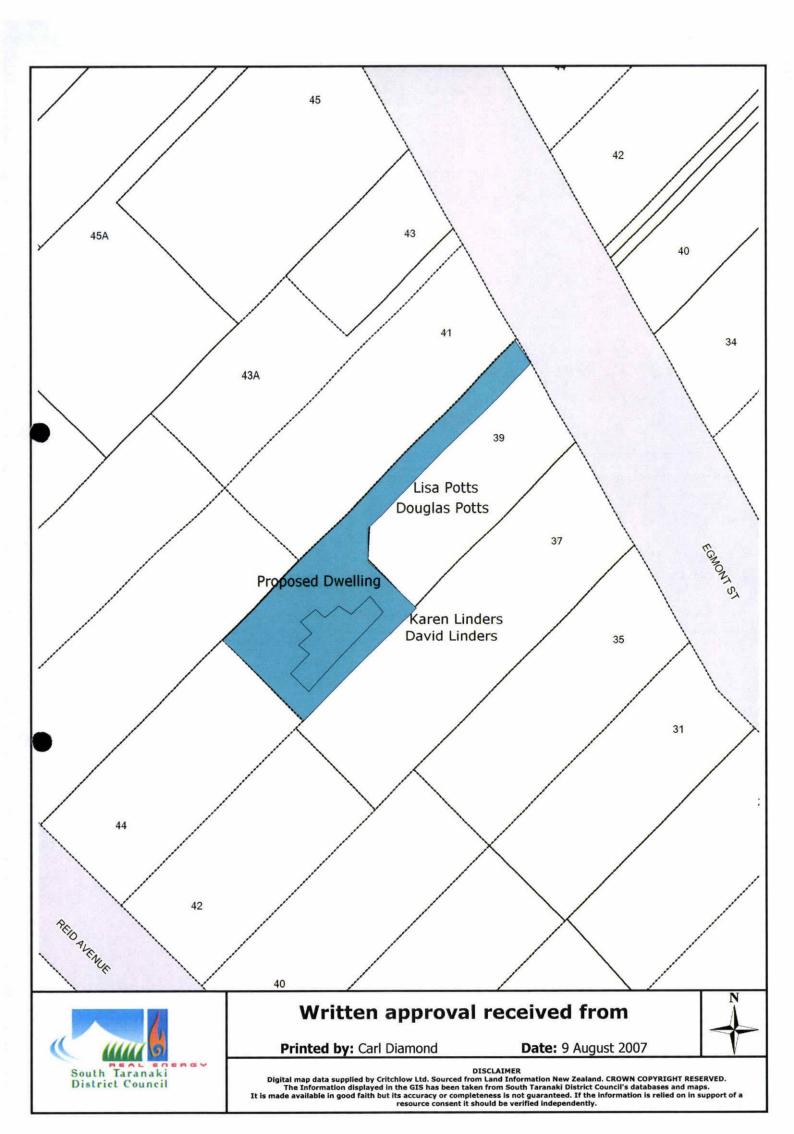
Blestlef

[Approved/Not Approved]

Alison Gibson

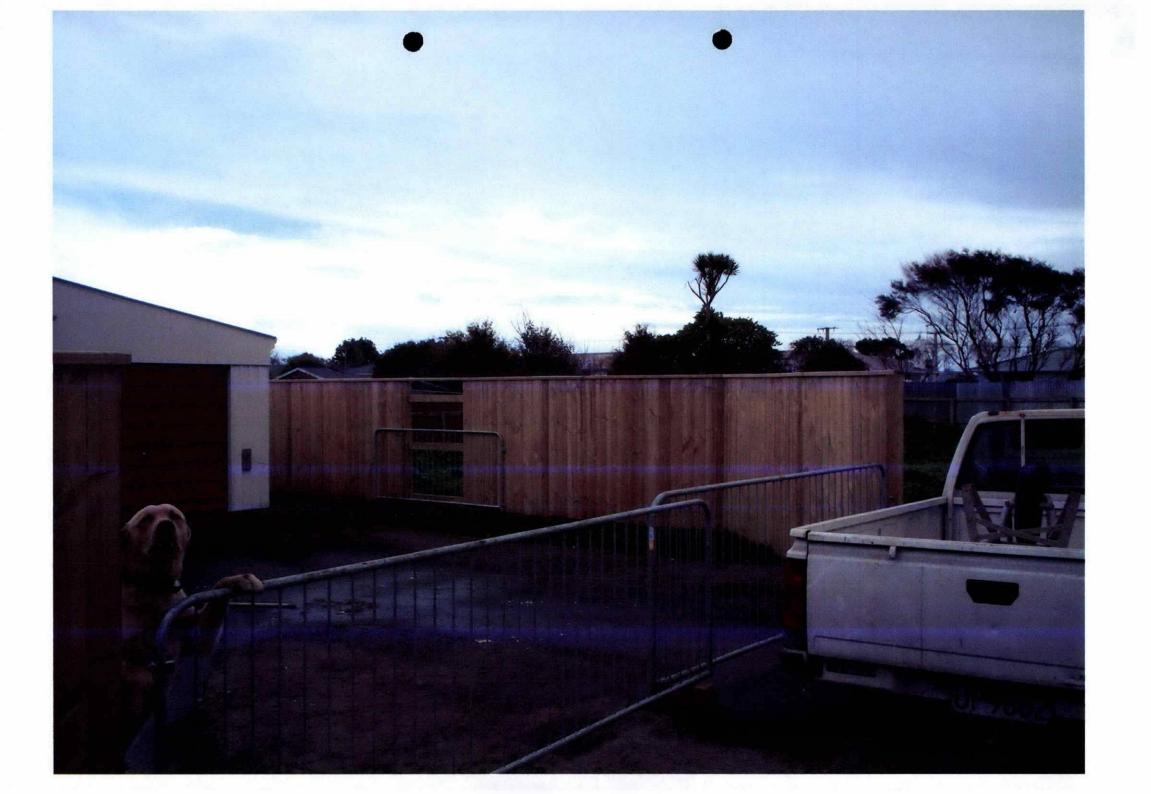
Planning Manager

Date: 10/8/07....









3 August 2007

Jason and Marea Roberts 237B South Road Hawera

Dear Jason and Marea

Resource Consent Application – Erect New Dwelling, 39A Egmont Street, Hawera

I write regarding the above application for resource consent which you have submitted to erect a new dwelling on the property at 39A Egmont Street, Hawera.

It is clear from your application that you have made considerable effort to obtain the appropriate signatures to continue processing your resource consent. However, at this point we have not received signed approval forms and site plans from Karen Linders (co-owner of 37 Egmont Street, Hawera)

Your application will be placed on hold until we receive this information from you.

If you have any queries please do not hesitate to contact me.

Yours sincerely

Carl Diamond
Environmental Monitoring Officer



COMPUTER FREEHOLD REGISTER **UNDER LAND TRANSFER ACT 1952**



Search Copy

ldentifier

Date Issued

288517 Land Registration District Taranaki

29 August 2006

Prior References

TNA2/1368

Estate

Fee Simple

674 square metres more or less

Legal Description Lot 2 Deposited Plan 371240

Proprietors

Jason Alfred Roberts as to a 12 share

Marea Aroha Roberts as to a 1/2 share

Interests

Subject to a right of way over part marked A on DP 371240 created by Easement Instrument 7006326.2 - 29.8.2006 at

7361253.3 Mortgage to GE Custodians - 25.5.2007 at 3:07 pm

Approval from Person Affected by an Application for a Resource Consent



1, Karen Linders	being the owner and occupier			
Please note that Council requires the approval of <u>all</u> legal owners and <u>all</u> occupiers of the property.	being the owner being the occupier			
of the property at 37 Egmont	Street, Hawera.			
hereby give my approval for the application				
39 A EgMONT ST, F	Hawery			
The proposal is to				
Frect a new dwelling	Cet 39A Egmont St, Howera.			
and does not meet the following Propos				
1. Dwelling is 1.5 metres from left boundary. 2.				
3.				
The proposal has been explained to me and I understand its effects on me. (See reverse of form for possible effects)				
I have signed a copy of the site plan and any other relevant plans.				
I have signed a copy of the photos. (Where the application is to relocate a building)				
I also understand that if I give my approval, the Council cannot take into account any effect that the proposal may have on me, when it considers the application.				
Signed	Date <u>08 08 07 </u>			

If you do not fully understand the proposal or do not agree with the proposal <u>do not</u> sign this form.

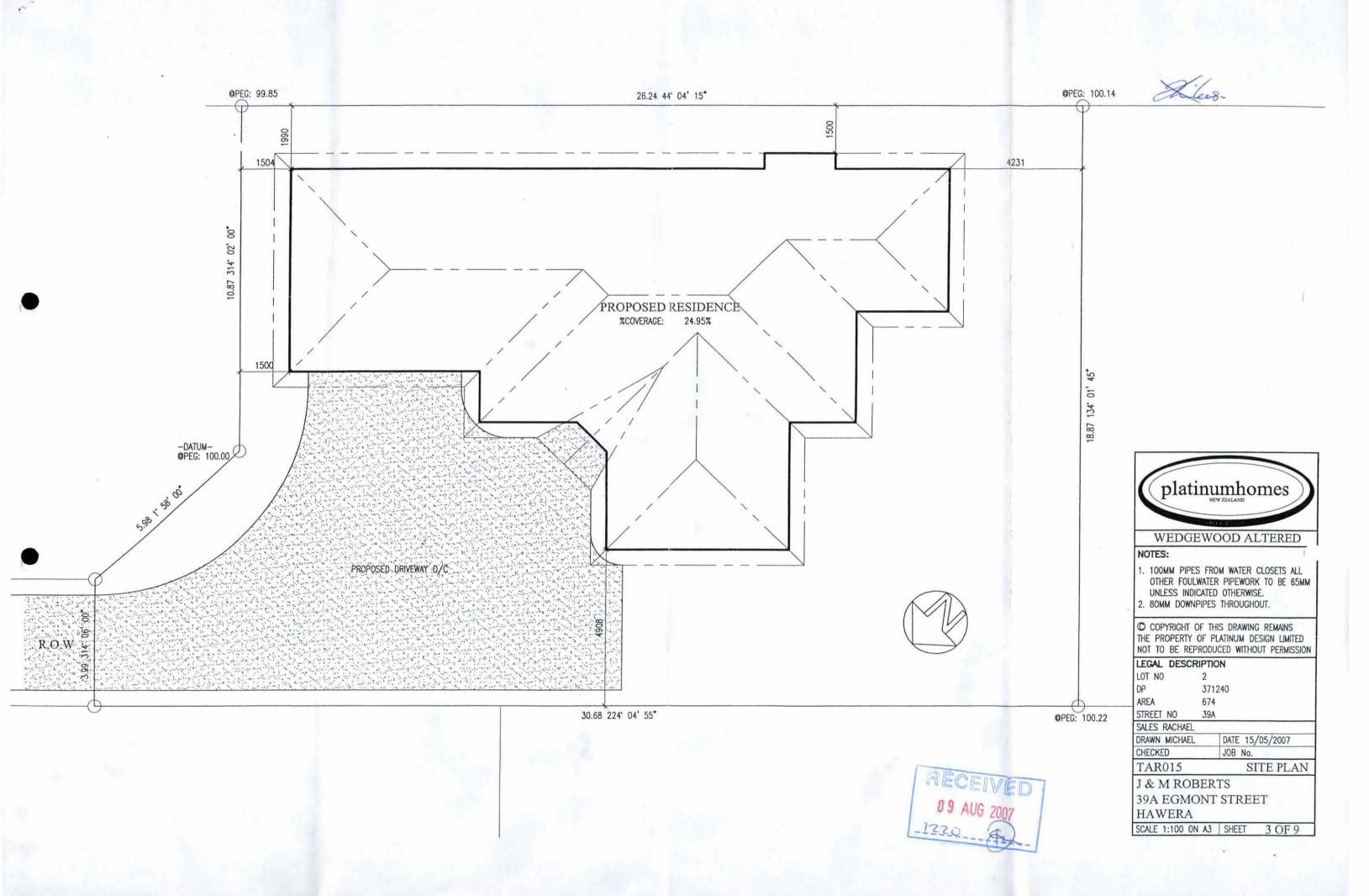
Please note you may withdraw your consent at any time prior to the Council making a decision on the application.

If you have any questions contact a Planner at the South Taranaki District Council on VED 278 0555 or 0800 111 323.

1220 AUG 2007

The following is a list of effects that you may wish to consider when deciding whether or not this application will affect you and the use of your land.

- 1. The visual impact of the proposal in terms of its scale, location on the site, distance from the boundary, external design and appearance and relationship to the site boundary.
- 2. The likelihood of the proposal generating effects on your land and the use of that land in terms of the following:
 - (a) Traffic Generation
 - (b) Noise
 - (c) Odour
 - (d) Vibration
 - (e) Air Pollution
 - (f) Shading, loss of daylight and sunlight
 - (g) Loss of Privacy
- 3. The impact of the proposal on the neighbourhood in general and the things that contribute to the pleasantness, aesthetics and cultural and recreational attributes of the neighbourhood.



Approval from Person Affected by an Application for a Resource Consent



1. Lisa & Douglas Pots	being the owner and occupier			
Please note that Council requires the	being the owner			
approval of <u>all</u> legal owners and <u>all</u> occupiers of the property.	being the occupier			
of the property at S9 Emmo	ut St. Hawera.			
hereby give my approval for the application				
399 Egmont St	, Hawara			
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Buil New duelle	ng 1.5m Join poit			
and does not meet the following Propose	d District Plan requirements			
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3.				
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I have signed a copy of the site plan and any other relevant plans.				
I have signed a copy of the photos. (Where the application is to relocate a building)				
I also understand that if I give my approval, the Council cannot take into account any effect that the proposal may have on me, when it considers the application.				
Signed Yuralohi	Afell Date 19/06/07			

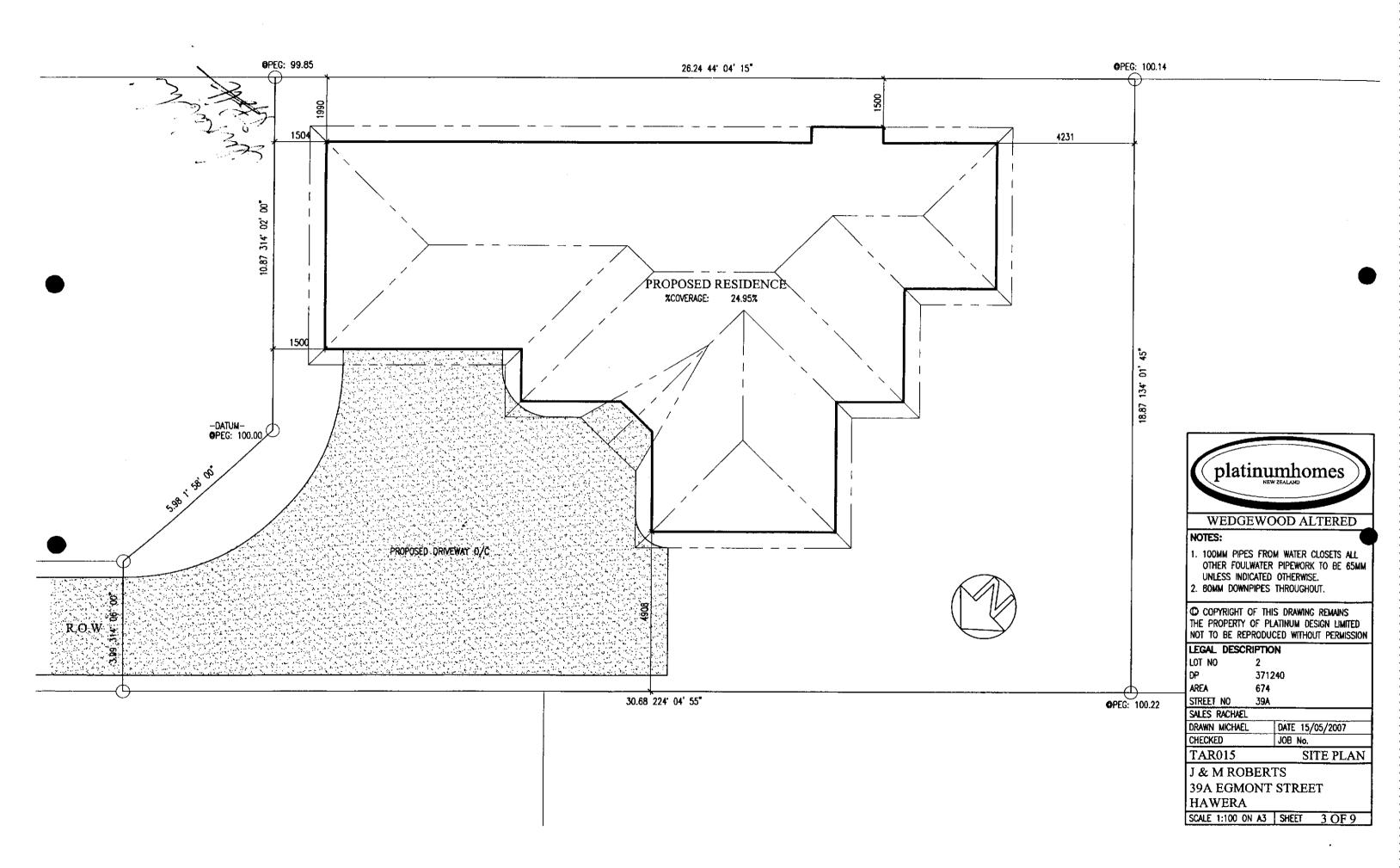
If you do not fully understand the proposal or do not agree with the proposal <u>do not</u> sign this form.

Please note you may withdraw your consent at any time prior to the Council making a decision on the application.

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Approval from Person Affected by an Application for a Resource Consent



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Please note that Council requires the approval of all legal owners and all	
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any arcacinatina o oposal may naveour me whantited	
Signed	Date 14/06/07.

If you do not fully understand the proposal or do not agree with the proposal <u>do not</u> sign this form.

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